9 "SHOT PIN" NOTES FOR HILTI X-U

S1.01 NOT TO SCALE

1. THESE NOTES GOVERN ALL CONDITIONS CALLED OUT ON THE PLANS AS "SHOT PINS" UNLESS SPECIFICALLY NOTED OTHERWISE.
2. ALL SHOT PINS SHALL BE AS MANUFACTURED BY HILTI, INCORP. REFERENCE SHALL BE MADE TO THE LATEST EDITION OF THE HILTI PRODUCT TECHNICAL GUIDE AND THE ICC-ES ESR-2269 REPORT FOR ADDITIONAL INFORMATION.
3. SHOT PINS DRIVEN INTO STEEL BASE MATERIAL SHALL BE X-U TYPE WITH P8 WASHERS. LENGTH OF PIN SHALL BE AS REQUIRED TO PENETRATE THROUGH THE STEEL BASE MATERIAL. MINIMUM EDGE DISTANCE TO ANY CONNECTED PART SHALL BE 1/2" AND MINIMUM FASTENER SPACING SHALL BE 3". ENTIRE POINTED END OF PIN MUST PENETRATE THROUGH STEEL LESS THAN 1/2" THICK OR PENETRATE A MINIMUM OF 1/2" INTO STEEL 1/2" THICK OR GREATER. PINS IN STEEL SUBJECT TO WITHDRAW LOADS ARE REQUIRED TO HAVE KNURLED SHANK.
4. SHOT PINS DRIVEN INTO CONCRETE BASE MATERIAL SHALL BE X-U TYPE WITH P8 WASHERS. LENGTH OF PIN SHALL BE AS REQUIRED TO PENETRATE 1 1/2" INTO THE CONCRETE BASE MATERIAL. MINIMUM EDGE DISTANCE TO ANY CONCRETE MATERIAL SHALL BE 3" AND MINIMUM FASTENER SPACING SHALL BE 4".
5. WHERE STEEL WASHERS ARE INDICATED ON THE DRAWINGS, PINS SHALL BE X-U WITH PREMOUNTED STEEL WASHERS WITH A MINIMUM DIAMETER OF 56mm (1 7/16").

5 LEGENDS AND SYMBOLS

S1.01 NOT TO SCALE

-STEEL
-MASONRY
-AGGREGATE
-WOOD BLOCK
-CONTINUOUS WOOD MEMBER
-NATIVE SOIL
-ENGINEERED FILL
-GROUT
-CONCRETE

DETAIL NUMBER / SHEET NUMBER — X / XX — -DETAIL
SECTION NUMBER / SHEET NUMBER — X / XX — -SECTION
ELEVATION NUMBER / SHEET NUMBER — X / XX — -ELEVATION

10 HILTI HIT-HY 200 EPOXY NOTES IN CONC.

S1.01 NOT TO SCALE

1. EPOXY SHALL BE HILTI HIT-HY 200 ADHESIVE AS MANUFACTURED BY HILTI, INC., 7250 DALLAS PARKWAY, SUITE 1000, PLANO, TEXAS 75024. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND I.C.C. REPORT NO. ESR-2322.
2. TEST VALUES AND INSTALLATION REQUIREMENTS SHALL BE AS FOLLOWS:

BAR SIZE	BOLT SIZE	MIN. EMBEDMENT	TEST LOAD (POUNDS)
#4	1/2"	4"	1140
#5	5/8"	5"	1540
#6	3/4"	7"	2190

3. PLACEMENT GUIDELINES FOR ABOVE VALUES IN ITEM 2 REQUIRE THE FOLLOWING CONDITIONS.
   - TABLE VALUES ARE BASED ON f'_c = 3000 PSI
   - 6" MIN. EDGE DISTANCE & 12" MIN. BOLT/BAR SPACING
   - HOLES DRILLED WITH A HAMMER DRILL AND CARBIDE BIT
   - BIT DIAMETER EQUALS (BAR DIAMETER) + 1/8"
   - HOLES SHALL BE DRY OR WATER-SATURATED, BUT NOT WATER FILLED
   - MAX. LONG TERM TEMPS OF 110° F OR LESS & MAX. SHORT TERM OF 162° F OR LESS
   - ANY SEISMIC DESIGN CATEGORY PER 2016 C.B.C.
   - A.S.T.M. A615 GRADE 60 REBAR
   - A.S.T.M. A193 GRADE B7 THREADED RODS
   - A.C.I. "CRACKED" CONCRETE CONDITION IS SUFFICIENT
   - DEPTH/THICKNESS OF CONCRETE BEING ATTACHED TO MUST BE AT LEAST 1 1/2" GREATER THAN THE EMBEDMENT DEPTH.
4. WHEN INSTALLING EPOXIED REBAR/BOLTS IN EXISTING CONCRETE, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS. MAINTAIN A MINIMUM CLEARANCE OF ONE-INCH BETWEEN THE EXISTING REINFORCEMENT AND THE EPOXIED REBAR/BOLT.
5. ANY REBAR/BOLTS SHOWN ON THE APPROVED PLANS AS BEING EPOXIED, REQUIRES SPECIAL INSPECTION IN ACCORDANCE WITH SECTION 4.4 IN THE I.C.C. REPORT. SPECIAL INSPECTION SHALL BE BY AN APPROVED TESTING AND INSPECTION AGENCY. ANY ITEMS THAT REQUIRE EPOXY BUT ARE NOT SPECIFICALLY SHOWN AS BEING EPOXIED ON THE APPROVED PLANS MUST BE APPROVED BY THE STRUCTURAL ENGINEER AND D.S.A./O.S.H.P.D. PRIOR TO INSTALLATION.

1 GENERAL NOTES

S1.01 NOT TO SCALE

1. ANY SUBSTITUTIONS FOR STRUCTURAL ITEMS ON APPROVED PLANS SHALL BE REVIEWED BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO USE. REVIEW WILL BE BILLED ON A TIME AND MATERIALS BASIS TO CONTRACTOR WITH NO GUARANTEE THE SUBSTITUTION WILL BE ALLOWED.
2. DETAILS AND NOTES ON TYPICAL SHEETS SHALL APPLY U.N.O. DETAILS OF CONSTRUCTION NOT FULLY SHOWN SHALL BE OF THE SAME NATURE AS SHOWN FOR SIMILAR CONDITIONS. U.N.O.
3. DO NOT SCALE STRUCTURAL DRAWINGS. IF DIMENSIONS IN DRAWINGS ARE NOT CLEAR, OR DISCREPANCIES EXIST ON THE DRAWINGS OR SPECIFICATIONS, CONTACT THE ENGINEER.
4. SEE MECHANICAL, ELECTRICAL, AND/OR ARCHITECTURAL DRAWINGS FOR LOCATION AND SIZE OF PIPES, CONDUITS, FLOOR DRAINS, VENTS, DUCTS, DRAIN LEADERS AND OTHER SIMILAR OPENINGS NOT INDICATED ON THE STRUCTURAL DRAWINGS.
5. SEE MECHANICAL, ELECTRICAL AND/OR ARCHITECTURAL DRAWINGS FOR EMBEDMENT OF BOLTS, ANCHORS AND OTHER MISCELLANEOUS EMBEDDED ITEMS NOT SHOWN ON STRUCTURAL DRAWINGS.
6. THE DESIGN, FABRICATION AND CONSTRUCTION SHALL COMPLY WITH ACCEPTED LOCAL GOVERNING CODES OF THE PARTICULAR AREA.
7. THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. UNLESS OTHERWISE SHOWN, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL SOLELY AND COMPLETELY BE RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.
8. THE DUTY OF THE ENGINEER TO CONDUCT CONSTRUCTION REVIEW OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES IN, ON, OR NEAR THE CONSTRUCTION SITE.
9. CONSTRUCTION SHALL BE DISTINGUISHED FROM CONTINUOUS AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ENGINEER WHETHER PERFORMED PRIOR TO, DURING, OR AFTER COMPLETION OF CONSTRUCTION ARE PERFORMED SOLELY FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DRAWINGS AND SPECIFICATIONS, BUT THEY DO NOT GUARANTEE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.
10. ALL WORK SHALL CONFORM TO THE LATEST APPLICABLE CONSTRUCTION SAFETY REQUIREMENTS OF O.S.H.A. AND ANY OTHER GOVERNMENTAL ENTITY HAVING JURISDICTION.
11. SHOP DRAWINGS:
    - SHOP DRAWINGS SHALL BE SUBMITTED PER PROJECT SPECIFICATIONS.
    - PRIOR TO SUBMISSION, THE CONTRACTOR SHALL REVIEW ALL SUBMITTALS FOR CONFORMANCE WITH THE APPROVED DOCUMENTS AND SHALL STAMP SUBMITTALS AS BEING REVIEWED FOR CONFORMANCE TO APPROVED CONTRACT DOCUMENTS.
    - THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE DESIGN TEAM THAT HE AND THE SUBCONTRACTOR UNDERSTAND THE DESIGN CONCEPT BY INDICATING WHICH MATERIAL HE INTENDS TO FURNISH AND INSTALL, AND BY DETAILING THE FABRICATION AND INSTALLATION METHODS HE INTENDS TO USE.
    - DESIGN DOCUMENTS ARE NOT SHOP DRAWINGS AND SHALL NOT BE SUBMITTED AS SUCH.
12. ALL NOTES SPECIFIED ON PLANS AND IN DETAILS WITH THE WORD "TYPICAL" FOLLOWED BY **BOLD AND UNDERLINED** TEXT REFER TO THE TYPICAL PROJECT DETAILS ON S1 SERIES SHEETS. TYPICAL DETAILS ARE NOT SPECIFICALLY REFERENCED ON PLANS AND SPECIFIC DETAIL U.N.O.

2 DESIGN CRITERIA

S1.01 NOT TO SCALE

CODE
2016 CALIFORNIA BUILDING CODE, TITLE 24

RISK CATEGORY	TYPE III
FLOOD HAZARD AREA	ZONE X
ROOF LOAD	
DEAD LOADS	23 PSF (BLDG A)
DEAD LOADS	21 PSF (BLDG B)
LIVE LOAD	20 PSF
WALL LOADS	
EXTERIOR WALL DEAD LOADS	19 PSF
INTERIOR WALL DEAD LOADS	11 PSF
SEISMIC LOADING CRITERIA	
ANALYSIS PROCEDURE USED	ELF
SEISMIC IMPORTANCE FACTOR	1.25
MAPPED SPECTRAL ACCELERATION, MCE:	
a. Ss	1.114
b. S1	.0411
SPECTRAL RESPONSE COEFFICIENT:	
a. Fa	1.054
b. Fv	1.588
MAXIMUM CONSIDERED EARTHQUAKE RESPONSE ACCELERATIONS:	
a. Sms	1.175
b. Sm1	0.654
DESIGN SPECTRAL RESPONSE ACCELERATIONS:	
a. Sds	0.784
b. Sd1	0.436
SEISMIC DESIGN CATEGORY	D
RESPONSE MODIFICATION COEFFICIENT, R (WOOD SHEARWALLS)	6.5
SEISMIC RESPONSE COEFFICIENT/BASE SHEAR	0.151 *W
SEISMIC SYSTEM OVERSTRENGTH FACTOR	2.5
SEISMIC DEFLECTION AMPLIFICATION FACTOR	4
SEISMIC FORCE REDUNDANCY FACTOR	1.0
WIND LOADING CRITERIA:	
BASIC WIND SPEED	115 MPH
WIND DIRECTIONALITY FACTOR	0.85
EXPOSURE CATEGORY	C
TOPOGRAPHIC FACTOR	1.0
ENCLOSURE CLASSIFICATION	ENCLOSED
DESIGN WALL WIND LOADS, Pnet	30 PSF
DESIGN PARAPET WIND LOADS, Pnet	72 PSF
DESIGN UPLIFT WIND LOADS, Pnet	-30 PSF
SOILS CRITERIA:	
ALLOWABLE BEARING PRESSURE (DL+LL)	2500 PSF
PASSIVE EARTH PRESSURE	350 PSF
COEFFICIENT OF FRICTION	0.4
SITE CLASSIFICATION	D
TESTING LAB	KRAZAN & ASSOC.
SOILS REPORT NUMBER	022-11101
SOILS REPORT DATE	2017-11-01

11 HILTI KWIK BOLT TZ IN CONC.

S1.01 NOT TO SCALE

1. EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ AS MANUFACTURED BY HILTI, INC., 7250 DALLAS PARKWAY, SUITE 1000, PLANO, TEXAS 75024. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND I.C.C. REPORT NO. ESR-1917.
2. TEST VALUES AND INSTALLATION REQUIREMENTS SHALL BE AS FOLLOWS:

BOLT SIZE	MIN. EMBED	CONT. THKS.	TENSION TEST	TORQUE TEST
3/8"	2"	4"	1100#	25 #-FT
1/2"	3 1/2"	6"	2000#	40 #-FT
5/8"	4"	6"	2300#	60 #-FT
3/4"	4 3/4"	8"	3100#	110 #-FT

3. PLACEMENT GUIDELINES FOR ABOVE VALUES IN ITEM 2 REQUIRE THE FOLLOWING CONDITIONS:
   - TABLE VALUES ARE BASED ON f'_c = 3000 PSI
   - HOLES DRILLED WITH A HAMMER DRILL AND CARBIDE BIT COMPLYING W/ ANSI B212.15-1994
   - BIT DIAMETER EQUALS THE SIZE OF THE ANCHOR BEING INSTALLED
   - HOLE DEPTH MUST EXCEED MIN. EMBED. BY ONE BOLT DIAMETER
   - ANY SEISMIC DESIGN CATEGORY PER 2016 C.B.C.
   - TENSION LOAD VALUES SHALL BE MULTIPLIED BY 0.6 FOR LIGHTWEIGHT CONCRETE
   - A.C.I. "CRACKED" CONCRETE CONDITION IS SUFFICIENT
   - FOR CARBON OR STAINLESS STEEL BOLTS
4. WHEN INSTALLING EXPANSION ANCHORS IN EXISTING CONCRETE, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS. MAINTAIN A MINIMUM CLEARANCE OF ONE-INCH BETWEEN THE EXISTING REINFORCEMENT AND THE EXPANSION ANCHOR.
5. ANY BOLTS SHOWN ON THE APPROVED PLANS AS EXPANSION ANCHORS, REQUIRES SPECIAL INSPECTION IN ACCORDANCE WITH SECTION 4.4 IN THE I.C.C. REPORT. SPECIAL INSPECTION SHALL BE BY AN APPROVED TESTING AND INSPECTION AGENCY. ANY ITEMS THAT REQUIRE EXPANSION ANCHORS BUT ARE NOT SPECIFICALLY SHOWN ON THE APPROVED PLANS MUST BE APPROVED BY THE STRUCTURAL ENGINEER AND D.S.A./O.S.H.P.D. PRIOR TO INSTALLATION.

7 GENERAL PROJECT NOTES

S1.01 NOT TO SCALE

A SPECIAL INSPECTOR EMPLOYED BY THE OWNER IN ACCORDANCE WITH THE REQUIREMENTS OF THE CALIFORNIA CODE OF REGULATIONS, TITLE 24 AND SECTION 1704A.2 OF THE 2016 C.B.C. SHALL BE REQUIRED TO INSPECT THE PORTIONS OF THE PROJECT LISTED BELOW. THE SPECIAL INSPECTOR'S DUTIES ARE SPECIFICALLY DEFINED BY TITLE 24. THE SPECIAL INSPECTOR SHALL KEEP RECORDS OF INSPECTIONS AS REQUIRED IN SECTION 1704A.2.4 OF THE 2016 C.B.C. AND SUBMIT THEIR REPORTS DIRECTLY TO D.S.A./O.S.H.P.D.

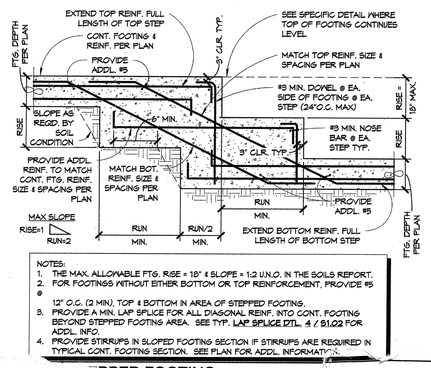
REQUIRED AREAS OF INSPECTION	2016 C.B.C. REFERENCES
· STEEL	SECTIONS 1705A.2.1 & 1705A.12.1 & TABLE 1705A.2.3
· CONCRETE	SECTION 1705A.3 & TABLE 1705A.3
· MASONRY	SECTIONS 1705A.4
-OCCUPANCY CATEGORY I, II OR III	TMS 402/ACI 530/ASCE 5
-OCCUPANCY CATEGORY IV	TMS 602/ACI 530.1/ASCE 6
· WOOD	SECTIONS 1705A.5 & 1705A.11.1/1705A12.2
· SOILS	SECTION 1705A.6 & TABLE 1705A.6
· PIER FOUNDATIONS	SECTION 1705A.9 & TABLE 1705A.9
· POST-INSTALLED ANCHORS	SECTION 1910A.5 & I.C.C. E.S.R. REPORTS
· EPOXY ADHESIVES	SECTION 1910A.5 & I.C.C. E.S.R. REPORTS

NOTE: FOR SPECIFIC REQUIREMENTS REGARDING SPECIAL INSPECTION FOR D.S.A. PROJECTS, SEE THE TESTING AND INSPECTION FORM FOR THIS PROJECT.

4 ABBREVIATIONS

S1.01 NOT TO SCALE

A.B.	ANCHOR BOLT
ABV.	ABOVE
A.C.I.	AMERICAN CONCRETE INSTITUTE
ADDL.	ADDITIONAL
A.E.S.A.	ARCHITECTURAL EXPOSED STEEL
A.F.P.A.	AMERICAN FOREST AND PAPER ASSOCIATION
A.I.S.C.	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
A.I.T.C.	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION
ALT.	ALTERNATE
A.P.A.	AMERICAN PLYWOOD ASSOCIATION
ARCH.	ARCHITECT(URAL)
A.E.S.S.	ARCHITECTURAL EXPOSED STRUCTURAL STEEL
A.S.C.E.	AMERICAN SOCIETY OF CIVIL ENGINEERS
A.S.T.M.	AMERICAN SOCIETY FOR TESTING & MATERIALS
A.W.S.	AMERICAN WELDING SOCIETY
BLDG.	BUILDING
BLK., BLKG.,	BLOCK, BLOCKING
BM.	BEAM
B.O.	BOTTOM OF
BOT.	BOTTOM
C	CHANNEL
C.B.C.	CALIFORNIA BUILDING CODE
CDX	C-D EXPOSURE 1
C.J.	CONSTRUCTION JOINT
CL.	CENTERLINE
CLG.	CEILING
CLR.	CLEAR
C.M.U.	CONCRETE MASONRY UNIT
COL.	COLUMN
CONC.	CONCRETE
CONN.	CONNECTION
CONT.	CONTINUOUS
CTSK.	COUNTERSINK
d	PENNY NAILS
DBL.	DOUBLE
DEMO.	DEMOLISH
DET., DTL.	DETAIL
DF	DOUGLAS FIR
DIA., Ø	DIAMETER
DIAG.	DIAGONAL
DIM.	DIMENSION
DL.	DEAD LOAD
DO	DITTO / DO OVER
DP	DEEP
D.S.A.	DEPARTMENT OF STATE ARCHITECT
DWG	DRAWING
(E)	EXISTING
EA.	EACH
ELEV.	ELEVATION
E.N.	EDGE NAILING
ENGR.	ENGINEER
EQ.	EQUAL
EQUIP.	EQUIPMENT
EXP.	EXPANSION
E.W.	EACH WAY
FDN.	FOUNDATION
F.E.M.A.	FEDERAL EMERGENCY MANAGEMENT AGENCY
F.F.	FINISH FLOOR
FIN.	FINISH
FLR.	FLOOR
F.N.	FIELD NAILING
FRMG.	FRAMING
FT., FTG.	FOOT, FOOTING
F.V.	FIELD VERIFY
GA.	GAUGE
GALV.	GALVANIZED
GLB	GLUE LAMINATED BEAM
H.D.	HOLDOWN
HDR.	HEADER
HGR.	HANGER
HORIZ.	HORIZONTAL
HSS	HOLLOW STRUCTURAL SECTION
HT.	HEIGHT
I.B.C.	INTERNATIONAL BUILDING CODE
I.C.C.	INTERNATIONAL CODE COUNCIL
IN	INCH
I.D.	INSIDE DIAMETER
INT.	INTERIOR
I.R.	INTERPRETATION OF REGULATIONS
JST.	JOIST
K	KIPS
KSI	KIPS PER SQUARE INCH
L	ANGLE
LBS, #	POUNDS (XXX LBS, XXX#)
LL	LIVE LOAD
LLV(LLH)	LONG LEG VERTICAL (HORIZ.)
LOC.	LOCATION
LSL	LAMINATED STRAND LUMBER
LT. WT.	LIGHT WEIGHT
LVL	LAMINATED VENEER LUMBER
MAS.	MASONRY
MAX.	MAXIMUM
M.B.	MACHINE BOLT
MFR	MANUFACTURER
MIN.	MINIMUM
M.S.R.	MACHINE STRESS RATED
MTL.	METAL
(N)	NEW
NO. #	NUMBER, (NO. XX, #XX)
N.T.S.	NOT TO SCALE
O/, o/	OVER
O.C.	ON CENTER
O.D.	OUTSIDE DIAMETER
O.H.	OPPOSITE HAND
OPNG.	OPENING
OP.	OPPOSITE
OSB	ORIENTED STRAND BOARD
O.S.H.P.D.	OFFICE OF STATEWIDE HEALTH PLANNING AND DEVELOPMENT
PL.	PLATE
P.T.	PRESSURE TREATED
PLYWD.	PLYWOOD
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
REINF.	REINFORCING
REQD.	REQUIRED
RM.	ROOM
SCHED.	SCHEDULE
SHTG.	SHEATHING
SHT.	SHEET
SIM.	SIMILAR
SMS	SHEET METAL SCREWS
SPEC.	SPECIFICATION
SQ.	SQUARE
STGR.	STAGGER
STD.	STANDARD
STIFF.	STIFFENER
STL.	STEEL
STRUCT.	STRUCTURAL
SYM.	SYMMETRICAL
T&B	TOP AND BOTTOM
T&G	TONGUE AND GROOVE
THK.	THICK
TL.	TOTAL LOAD
T.O.	TOP OF
TYP.	TYPICAL
U.N.O., U.O.N.	UNLESS NOTED OTHERWISE
VERT.	VERTICAL
W/, W/O	WITH, WITH OUT
WD	WOOD
W.F.	WIDE FLANGE
W.C.L.I.B.	WEST COAST LUMBER INSPECTION BUREAU
WT.	WEIGHT
W.W.F.	WELDED WIRE FABRIC
W.W.M.	WELDED WIRE MESH
W.W.P.A.	WESTERN WOOD PRODUCTS ASSOCIATION



NOTES:
1. THE MAX. ALLOWABLE FTG. RISE = 15" & SLOPE = 1:2 U.N.O. IN THE SOILS REPORT.
2. FOR FOOTINGS WITHOUT EITHER BOTTOM OR TOP REINFORCEMENT, PROVIDE #5 @ 12" O.C. (2 MIN). TOP & BOTTOM IN AREA OF STEPPED FOOTING.
3. PROVIDE A MIN. LAP SPLICE FOR ALL DIAGONAL REINF. INTO CONT. FOOTING BEYOND STEPPED FOOTING AREA. SEE TYP. LAP SPLICE DTL. 4 / S1.02 FOR ADDL. INFO.
4. PROVIDE STIRRUPS IN SLOPED FOOTING SECTION IF STIRRUPS ARE REQUIRED IN TYPICAL CONT. FOOTING SECTION. SEE PLAN FOR ADDL. INFORMATION.

RRED FOOTING

8 SPECIAL INSPECTION NOTES

S1.01 NOT TO SCALE

Int… and is not any other p…

integrated designs by SOMAM, Inc.
ARCHITECTURE ENGINEERING INTERIOR DESIGN CONSTRUCTION MANAGMENT
6011 N. Fresno, Suite 130 · Fresno, California 93710
Phone (559) 436-0881 Fax (559) 436-0887 E-Mail: design@somam.com
www.integrateddesigns.com

Revision:	Revision Description:	Rev. Date:

Issue Date: 01-31-18
Date:
Designer:
Drawn:
DR:
PC:

Sheet Title: TYPICAL PROJECT NOTES

Project No: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Agency Approval Stamp:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC. FLS. SS
DATE 06 - 22 18
TRACKING #: 633?1-300

Stamp(s)

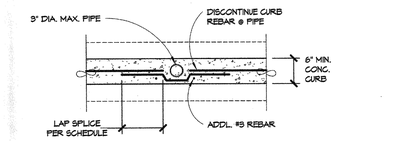
REGISTERED PROFESSIONAL ENGINEER
GALDORD R. RANSOM
No. S2556
31-20
STRUCTURAL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711
T7000

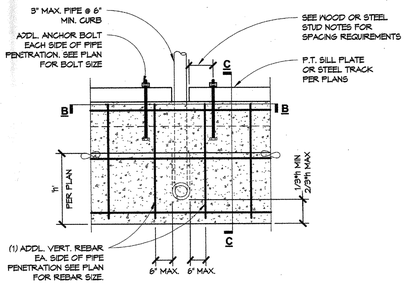
Job No.:

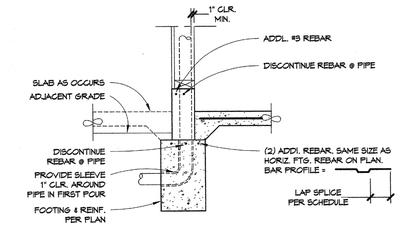
Sheet No.: S1.01

Release:



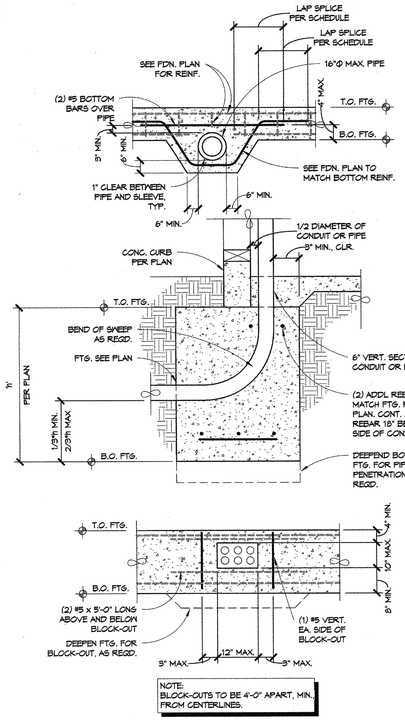
VIEW B-B





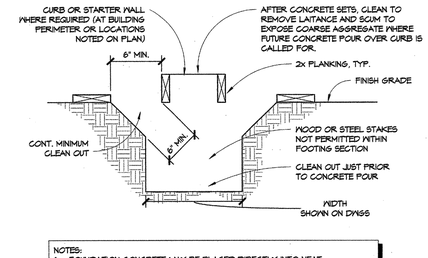
VIEW C-C

18 | PIPE & CONDUIT THRU CONC. CURB AND FTG.
S1.02 | NOT TO SCALE



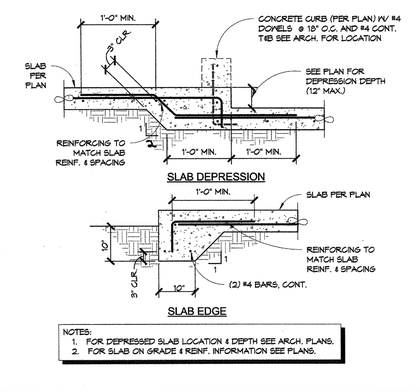
NOTE:
BLOCK-OUTS TO BE 4'-0" APART, MIN. FROM CENTERLINES.

14 | PIPES & CONDUITS THROUGH FOOTINGS
S1.02 | NOT TO SCALE



NOTES:
1. FOUNDATION CONCRETE MAY BE PLACED DIRECTLY INTO NEAT EXCAVATIONS PROVIDED THAT FOUNDATION TRENCH WALLS ARE STABLE AS DETERMINED BY THE GEOTECHNICAL ENGINEER, SUBJECT TO THE APPROVAL OF THE D.S.A./O.S.H.P.D. IN SUCH CASE, THE MINIMUM FORM WORK SHOWN ABOVE IS MANDATORY TO INSURE CLEAN EXCAVATIONS IMMEDIATELY PRIOR TO, AND DURING, THE PLACING OF CONCRETE.
2. FORMWORK NOT PERMITTED BELOW GRADE UNLESS FULLY FORMED.

9 | REQUIRED CONT. FTG. FORM WORK
S1.02 | NOT TO SCALE

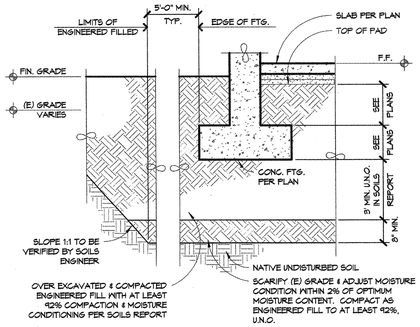


SLAB DEPRESSION

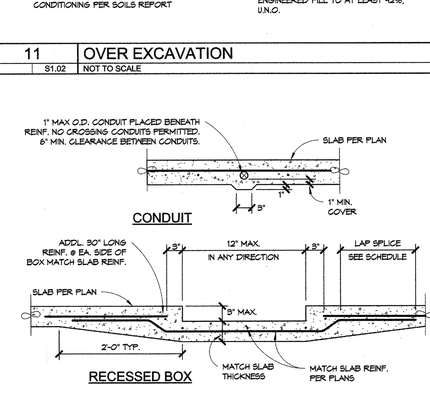
SLAB EDGE

NOTES:
1. FOR DEPRESSED SLAB LOCATION & DEPTH SEE ARCH. PLANS.
2. FOR SLAB ON GRADE & REINF. INFORMATION SEE PLANS.

10 | SLAB DEPRESSION AND EDGE
S1.02 | NOT TO SCALE

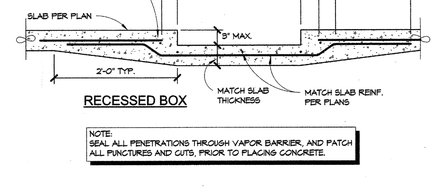


15 | PIPES BELOW/ADJACENT TO FTG.
S1.02 | NOT TO SCALE



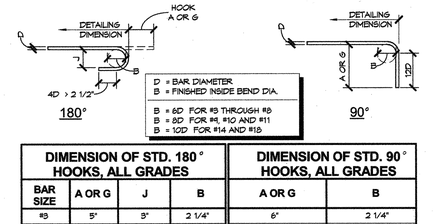
11 | OVER EXCAVATION
S1.02 | NOT TO SCALE

DOUBLE CURTAIN



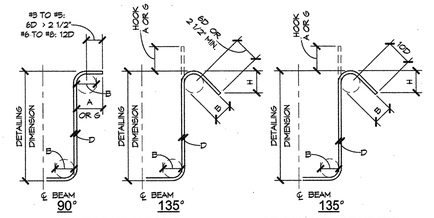
SINGLE CURTAIN

12 | CONDUITS AND BOXES IN SLAB
S1.02 | NOT TO SCALE



180° 90°

DIMENSION OF STD. 180° HOOKS, ALL GRADES				DIMENSION OF STD. 90° HOOKS, ALL GRADES	
BAR SIZE	A OR G	J	B	A OR G	B
#3	5"	3"	2 1/4"	6"	2 1/4"
#4	6"	4"	3"	8"	3"
#5	7"	5"	3 3/4"	10"	3 3/4"
#6	8"	6"	4 1/2"	1'-0"	4 1/2"
#7	10"	7"	5 1/4"	1'-2"	5 1/4"
#8	11"	8"	6"	1'-4"	6"
#9	1'-3"	1'-1 3/4"	9 1/2"	1'-7"	9 1/2"
#10	1'-5"	1'-1 1/4"	10 3/4"	1'-10"	10 3/4"
#11	1'-7"	1'-2 3/4"	12"	2'-0"	12"
#14	2'-3"	1'-9 3/4"	18 1/4"	2'-7"	18 1/4"
#18	3'-0"	2'-4 1/2"	24"	3'-5"	24"



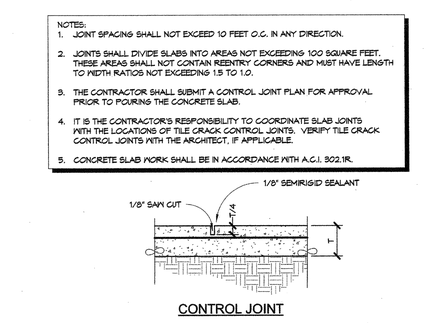
STIRRUP HOOKS
(TIE BENDS SIMILAR)

SEISMIC STIRRUP/TIE

BAR SIZE	B	90°	135°		135° SEISMIC HOOK			
		A OR G	A OR G	APPROX H.	SIZE	B	A OR G	APPROX H.
#3	1 1/2"	4"	4"	2 1/2"	#3	1 1/2"	4 1/4"	5"
#4	2"	4 1/2"	4 1/2"	3"	#4	2"	4 1/2"	5"
#5	2 1/2"	6"	5 1/2"	3 3/4"	#5	2 1/2"	5 1/2"	5 3/4"
#6	4 1/2"	1'-0"	8"	4 1/2"	#6	4 1/2"	8"	4 1/2"
#7	5 1/4"	1'-2"	9"	5 1/4"	#7	5 1/4"	9"	5 1/4"
#8	6"	1'-4"	10 1/2"	6"	#8	6"	10 1/2"	6"

6 | STANDARD REBAR HOOKS
S1.02 | NOT TO SCALE

NOTES:
1. JOINT SPACING SHALL NOT EXCEED 10 FEET O.C. IN ANY DIRECTION.
2. JOINTS SHALL DIVIDE SLABS INTO AREAS NOT EXCEEDING 100 SQUARE FEET. THESE AREAS SHALL NOT CONTAIN REENTRY CORNERS AND MUST HAVE LENGTH TO WIDTH RATIOS NOT EXCEEDING 1.5 TO 1.0.
3. THE CONTRACTOR SHALL SUBMIT A CONTROL JOINT PLAN FOR APPROVAL PRIOR TO POURING THE CONCRETE SLAB.
4. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE SLAB JOINTS WITH THE LOCATIONS OF TILE CRACK CONTROL JOINTS. VERIFY TILE CRACK CONTROL JOINTS WITH THE ARCHITECT, IF APPLICABLE.
5. CONCRETE SLAB WORK SHALL BE IN ACCORDANCE WITH A.C.I. 302.1R.



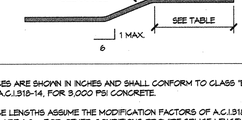
EXPANSION JOINT
FOR FLATWORK ONLY.
(SEE ARCH. AND/OR CIVIL DRAWINGS)

8 | CONCRETE SLAB JOINTS
S1.02 | NOT TO SCALE

1. FOUNDATIONS SHALL BEAR ON ENGINEERED FILL OR NATIVE SOIL A MINIMUM OF 18" BELOW ADJACENT GRADE OR FINISHED GRADE U.N.O. IN A SOILS REPORT.
2. MAXIMUM SIZE AGGREGATE SHALL BE AS FOLLOWS:
   - SLAB ON GRADE: 1"
   - CONCRETE OVER METAL DECK: 3/8"
   - FOOTINGS: 1 1/2"
   - COLUMNS & WALLS: 1 1/2"
3. MINIMUM COMPRESSIVE STRENGTH OF TYPE II CEMENT CONCRETE AT 28 DAYS SHALL BE AS FOLLOWS:
   - 4000 PSI NORMAL WEIGHT SLAB ON GRADE
   - 3000 PSI NORMAL WEIGHT FOOTINGS
   - 4000 PSI NORMAL WEIGHT CONCRETE COLUMNS & WALLS.
4. MAXIMUM WATER CEMENT RATIOS SHALL BE AS FOLLOWS:
   - 0.45 SLAB ON GRADE
   - 0.50 COLUMNS & WALLS
   - 0.58 FOOTINGS
5. THE FOLLOWING ARE MINIMUM CONCRETE COVER DIMENSIONS PER A.C.I. 318-14 SECTION 20.6.1. THEY ARE FROM FACE OF REINFORCING STEEL TO FACE OF CONCRETE. ALL REINFORCING STEEL SHALL MAINTAIN THE FOLLOWING CLEAR DISTANCES, U.N.O.
   - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
   - CONCRETE EXPOSED TO EARTH OR WEATHER: (NO. 6 THROUGH NO. 18) 2"; (NO. 5 AND SMALLER) 1 1/2"
   - CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS, WALLS AND JOISTS: (NO. 14 AND NO. 18) 1 1/2"; (NO. 11 AND SMALLER) 3/4"
     BEAMS AND COLUMNS: (ALL SIZES) 1 1/2"
     SHELLS AND FOLDED PLATE MEMBERS: (NO. 6 AND LARGER) 3/4"; (NO. 5 AND SMALLER) 1/2"
   - PLACE REINF. AT MID-THICKNESS FOR SLABS ON GRADE.
6. CONSTRUCTION LOADS SHALL NOT BE PLACED ON NEW CONCRETE CONSTRUCTION, INCLUDING CONCRETE TOPPING ON METAL DECK, FOR AT LEAST 7 DAYS AFTER CONCRETE PLACEMENT OR WITH APPROVAL BY ENGINEER.
7. ALL SPLICES IN CONTINUOUS REINFORCEMENT USED IN WALLS, FOOTINGS, ETC. SHALL HAVE A MINIMUM LAP AS DESCRIBED IN THE TYPICAL LAP SPLICE & / S1.02 DETAIL. SPLICES IN ADJACENT BARS SHALL NOT BE LESS THAN 4'-0" APART. VERTICAL WALL BARS SHALL BE SPLICED AT OR NEAR FLOOR LINES, OR WHERE SPECIFICALLY DETAILED TO BE SEPARATED.
8. ALL REINFORCEMENT CROSSING CONSTRUCTION JOINTS SHALL BE CONTINUOUS, OR SHALL BE MADE EFFECTIVELY CONTINUOUS BY USE OF FULLY DEVELOPED LAP SPLICES, DOWELS (WITH LAPPED SPLICES) OR APPROVED COUPLERS.
9. HORIZONTAL CONSTRUCTION JOINTS SHALL HAVE ENTIRE SURFACE REMOVED TO EXPOSE CLEAN AGGREGATE SOLIDLY EMBEDDED.
10. CONCRETE SHALL NOT BE DROPPED THROUGH REINF. STEEL (AS IN WALL) SO AS TO CAUSE SEGREGATION OF AGGREGATES. IN SUCH CASES, HOPPERS AND VERTICAL CHUTES OR TRUNKS SHALL BE USED. CHUTES OR TRUNKS SHALL BE OF VARIABLE LENGTHS SO THAT FREE UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED FIVE (5) FEET AND A SUFFICIENT NUMBER OF CHUTES AND TRUNKS SHALL BE USED TO ENSURE THE CONCRETE REMAINS LEVEL AT ALL TIMES.
11. ALL STEEL COLUMN BASE PLATES AND STEEL BEAMS BEARING ON CONCRETE SHALL BEAR UPON 1 1/2" OF NON-SHRINK, 5,000 PSI MIN. GROUT PADS AND LEVELING NUTS, U.N.O.
12. WHERE STEEL MEMBERS BEAR IN CONCRETE OR MASONRY WALLS, OPENINGS SHALL BE DRY-PACKED AFTER STEEL IS IN PLACE.
13. CONTRACTOR SHALL SUBMIT PROPOSED POUR SCHEDULE FOR ENGINEER'S APPROVAL PRIOR TO THE FORMING OR POURING OF ANY CONCRETE WORK.
14. PROVIDE 3/4" CHAMFER AT EXPOSED EDGES OF CONCRETE BEAMS, COLUMNS AND WALLS UNLESS NOTED OTHERWISE.
15. THE CONTRACTOR SHALL FURNISH AND INSTALL 1/2" PRE-MOLDED EXPANSION JOINTS IN ALL EXTERIOR WALKS AND SLABS AS INDICATED ON DRAWINGS, BUT IN NO CASE MORE THAN 24'-0" O.C.
16. ALL REINFORCING SHALL CONFORM TO A.S.T.M. A615 AND SHALL BE GRADE 40 FOR #3, GRADE 60 FOR #4 AND LARGER.
17. COLUMN SPIRALS SHALL CONFORM TO "STANDARD SPECIFICATION FOR COLD-DRAWING STEEL FOR CONCRETE REINFORCEMENT" ASTM A82 WITH A MINIMUM YIELD STRENGTH OF 60,000 PSI. FABRICATION SHALL CONFORM TO ACI MANUAL OF STANDARD PRACTICE.
18. WELDED WIRE FABRIC SHALL CONFORM TO A.S.T.M. A185.
19. ALL WELDING OF REINFORCING STEEL SHALL BE WITH LOW HYDROGEN ELECTRODES U.N.O. WELDING OF REINFORCING SHALL BE ALLOWED ONLY WHERE DETAILED ON DRAWINGS. ALL WELDING SHALL BE DONE IN ACCORDANCE WITH AMERICAN WELDING SOCIETY SPECIFICATIONS A.W.S. D1.4. WELDING SHALL NOT BE DONE WITHIN TWO BAR DIAMETERS OF ANY BENT PORTION OF A BAR WHICH HAS BEEN BENT COLD. WELDING OF CROSSING BARS SHALL NOT BE PERMITTED FOR ASSEMBLY OF REINFORCEMENT UNLESS AUTHORIZED BY THE STRUCTURAL ENGINEER OF RECORD. A.S.T.M. A706 REINFORCING SHALL BE USED FOR ALL REINFORCING THAT IS BEING WELDED.
20. WHERE DRILLED PIERS ARE USED, CONTRACTOR IS RESPONSIBLE TO PROVIDE MEASURES TO DRILL HOLE PER PLANS BY USE OF CASING IF UNFORESEEN SOIL CONDITIONS ARE ENCOUNTERED.

3 | FOUNDATION AND CONC. NOTES
S1.02 | NOT TO SCALE

LAP LENGTH (3,000 PSI CONCRETE)									
BAR SIZE	#3	#4	#5	#6	#7	#8	#9	#10	#11
BOTTOM BAR	16"	24"	36"	43"	62"	72"	81"	91"	101"
TOP BAR	21"	32"	47"	56"	81"	94"	105"	118"	131"



NOTES:
1. SPLICES ARE SHOWN IN INCHES AND SHALL CONFORM TO CLASS "B" SPLICES AS PER A.C.I.318-14, FOR 3,000 PSI CONCRETE.
2. SPLICE LENGTHS ASSUME THE MODIFICATION FACTORS OF A.C.I.318-14 SECTIONS 25.1.1 ARE 1.0. FOR OTHER CONDITIONS PROVIDE SPLICE LENGTHS IN ACCORDANCE WITH A.C.I.318-14.
3. USE THE SPLICE LENGTH GIVEN FOR TOP BARS WHEN MORE THAN 12" OF CONCRETE IS CAST BELOW HORIZONTAL BARS IN THE MEMBER. USE THE SPLICE LENGTH GIVEN FOR BOTTOM BARS FOR ALL OTHER CONDITIONS.

4 | LAP SPLICE SCHEDULE (CONC.)
S1.02 | NOT TO SCALE

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6011 N. Fresno, Suite 130 - Fresno, California 93710
Phone: (559) 436-0881 Fax: (559) 436-0887 E-Mail: design@somam.com
www.integrateddesigns.com

Revision:	Revision Description:	Rev. Date:

Sheet Title: TYPICAL FOUNDATION DETAILS
Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
Dates:
Designer: Designer
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EXPIRES 6-30-19
STRUCTURAL
STATE OF CALIFORNIA

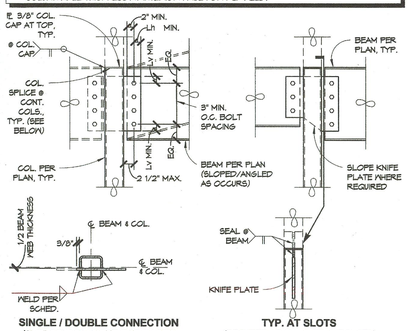
BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 103
FRESNO, CALIFORNIA 93711
17600

Job No.:

Sheet No.: S1.02

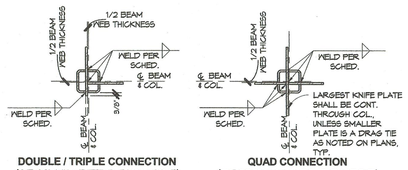
Release:

NOTES:
1. CLIP BEAM FLANGE(S) WHERE REQUIRED TO CLEAR ADJACENT BEAM FLANGES AT MULTI-BEAM CONNECTIONS.
2. KNIFE PLATE SLOT WIDTH = KNIFE PLATE THICKNESS + 1/8".
3. PJP WELDS ARE REQUIRED ALL AROUND COLUMN AT SPLICE LOCATIONS. LEAVE 1/16" OF COLUMN WALL THICKNESS AT INTERIOR FACE FOR PJP WELD.



SINGLE / DOUBLE CONNECTION
(SLOT COLUMN ⫩ ENTER ⫩ FROM SIDE

TYP. AT SLOTS
(REQUIRED AT ALL OVER CUT SLOTS)



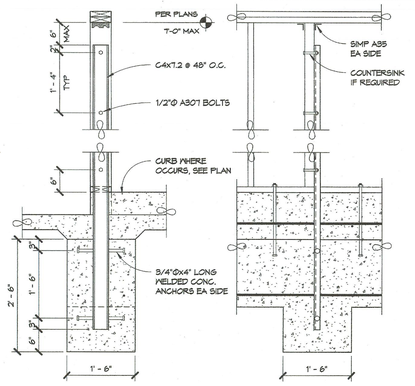
DOUBLE / TRIPLE CONNECTION
(CUT COLUMN ⫩ ENTER ⫩ FROM ABOVE)

QUAD CONNECTION
(CUT COLUMN ⫩ ENTER ⫩ FROM ABOVE)

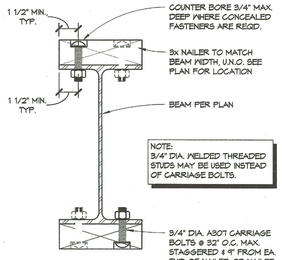
CONNECTION SCHEDULE

BEAM SIZE	PLATE	FILLET WELD SIZE	BOLTS NO.	BOLTS SIZE	BOLTS TYPE	DIMENSIONS Lh	DIMENSIONS Lv
W8x / G8x	3/8	1/4	(2)	3/4"Φ	A325N	1 1/2"	1 1/2"
W10x / G10x	3/8	1/4	(2)	3/4"Φ	A325N	1 1/2"	1 1/2"
W12x / G12x	3/8	1/4	(3)	3/4"Φ	A325N	1 1/2"	1 1/2"
W14x / G15x	3/8	1/4	(3)	3/4"Φ	A325N	1 1/2"	1 1/2"
W16x	3/8	1/4	(4)	3/4"Φ	A325N	1 1/2"	1 1/2"
W18x	3/8	1/4	(4)	1"Φ	A325N	2"	2"
W21x	1/2	5/16	(5)	1"Φ	A325N	2"	2"
W24x	1/2	5/16	(6)	1"Φ	A325N	2"	2"
W27x	1/2	5/16	(7)	1"Φ	A325N	2"	2"

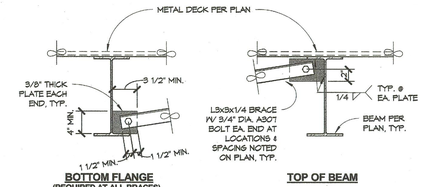
18 KNIFE PLATE CONNECT. SCHED.
S1.03 | 12" = 1'-0"

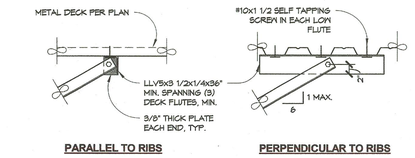


19 EXTERIOR KNEE WALL
S1.03 | NOT TO SCALE

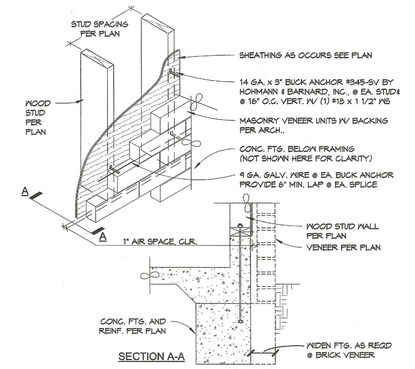


20 WOOD NAILERS ON 'W' BEAMS
S1.03 | NOT TO SCALE

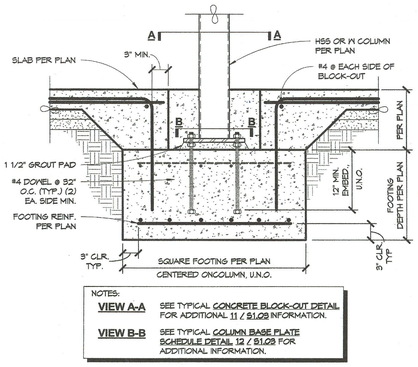




13 BEAM BOTTOM FLANGE BRACING
S1.03 | NOT TO SCALE

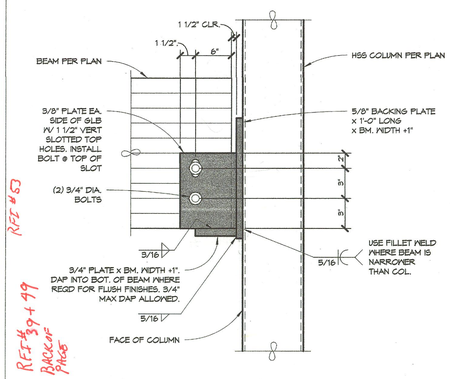


14 MASONRY VENEER TO WOOD FRAMING
S1.03 | NOT TO SCALE

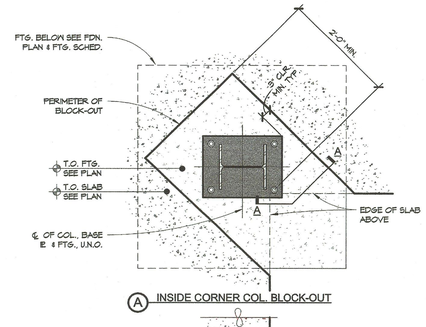


NOTES:
VIEW A-A SEE TYPICAL CONCRETE BLOCK-OUT DETAIL FOR ADDITIONAL 11 / S1.03 INFORMATION.
VIEW B-B SEE TYPICAL COLUMN BASE PLATE SCHEDULE DETAIL 12 / S1.03 FOR ADDITIONAL INFORMATION.

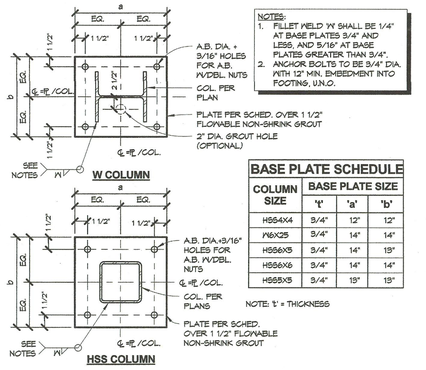
15 SQUARE FOOTING AT STEEL COL.
S1.03 | NOT TO SCALE



16 WOOD BEAM TO STEEL COLUMN (SIDE MOUNTED)
S1.03 | NOT TO SCALE



11 CONCRETE BLOCK-OUTS FOR STEEL COL.
S1.03 | NOT TO SCALE

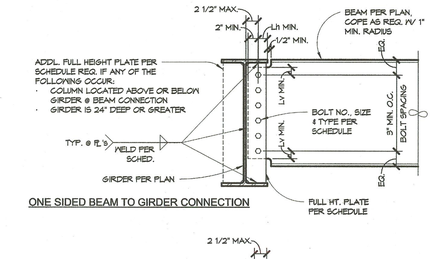


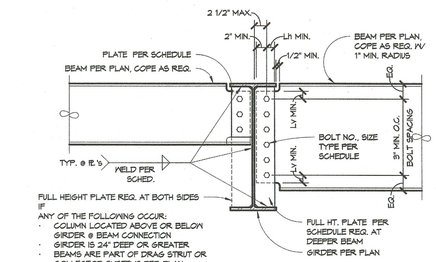
BASE PLATE SCHEDULE

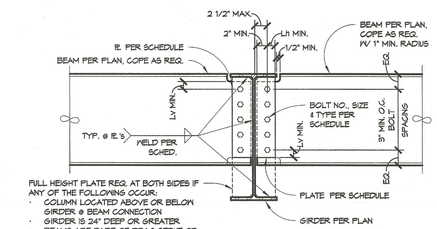
COLUMN SIZE	BASE PLATE SIZE 't'	'a'	'b'
HSS4X4	3/4"	12"	12"
HSS6X5	3/4"	14"	14"
HSS6X5	3/4"	14"	15"
HSS6X6	3/4"	14"	14"
HSS8X5	3/4"	13"	13"

NOTE: 't' = THICKNESS

12 COLUMN BASE PLATE SCHEDULE
S1.03 | NOT TO SCALE





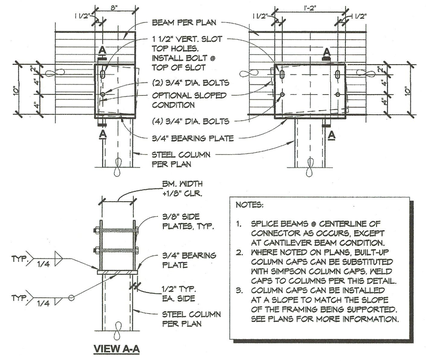


NOTE:
THIS DETAIL DOES NOT APPLY TO ANY CONDITION THAT IS NOT SPECIFICALLY ILLUSTRATED ABOVE. THEREFORE, FOR ANY OF THE FOLLOWING CONDITIONS SEE THE SPECIFIC DETAILS FOR MORE INFORMATION:
· BEAMS WITH THEIR UPPER FLANGE HIGHER THAN THE GIRDER UPPER FLANGE
· BEAMS WITH THEIR BOTTOM FLANGE LOWER THAN THE GIRDER BOTTOM FLANGE
· BEAMS DEEPER THAN THE GIRDER

CONNECTION SCHEDULE

BEAM SIZE	PLATE	FILLET WELD SIZE	BOLTS NO.	BOLTS SIZE	BOLTS TYPE	DIMENSIONS Lh	DIMENSIONS Lv
W8x / G8x	3/8	1/4	(2)	3/4"Φ	A325N	1 1/2"	1 1/2"
W10x / G10x	3/8	1/4	(2)	3/4"Φ	A325N	1 1/2"	1 1/2"
W12x / G12x	3/8	1/4	(3)	3/4"Φ	A325N	1 1/2"	1 1/2"
W14x / G15x	3/8	1/4	(3)	3/4"Φ	A325N	1 1/2"	1 1/2"
W16x	3/8	1/4	(4)	3/4"Φ	A325N	1 1/2"	1 1/2"
W18x	3/8	1/4	(4)	1"Φ	A325N	2"	2"
W21x	1/2	5/16	(5)	1"Φ	A325N	2"	2"
W24x	1/2	5/16	(6)	1"Φ	A325N	2"	2"
W27x	1/2	5/16	(7)	1"Φ	A325N	2"	2"

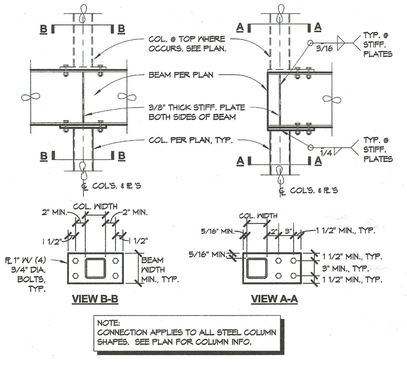
8 WOOD BEAM TO STEEL COLUMN (TOP MOUNTED)
S1.03 | NOT TO SCALE



7 STEEL BEAM TO STEEL GIRDER CONN. SCHED.
S1.03 | NOT TO SCALE

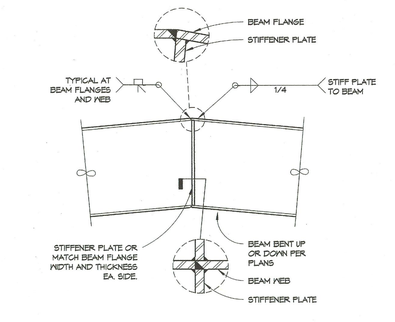
1. MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE LATEST EDITION OF THE "SPECIFICATION FOR STRUCTURAL STEEL BUILDING" PUBLISHED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION.
2. ALL BUTT WELDS SHALL BE COMPLETE JOINT PENETRATION WELDS.
3. ALL WELDING OF STRUCTURAL STEEL MEMBERS SHALL BE DONE BY CURRENTLY CERTIFIED WELDERS AND DONE IN CONFORMANCE WITH THE A.I.S.C. AND A.W.S. SPECIFICATIONS. WELDING IS NOT PERMITTED ON MEMBERS SUPPORTING LOADS.
4. WHERE THE CONTRACTOR REQUESTS WELDING TO BE USED IN LIEU OF BOLTED CONNECTIONS, SUCH WELDING SHALL BE DONE ONLY WITH THE ENGINEERS PRIOR APPROVAL. WHERE EMBEDDED PLATES OCCUR WITH SHEAR TABS, CONTRACTOR HAS OPTION TO FIELD WELD STEEL BEAM TO SHEAR TAB TO FACILITATE ERECTION WITH APPROVAL FROM ENGINEER. EMBEDDED PLATE MAY BE OVERSIZED BY 1" IN EACH DIRECTION TO FACILITATE FIELD WELDING. ANY TESTING AND/OR INSPECTION THE ENGINEER MAY DEEM NECESSARY TO BE ASSURED OF THE QUALITY OF SUCH WELDING, SHALL BE DONE AT THE CONTRACTOR'S EXPENSE.
5. A.S.T.M. A325 BOLTS SHALL BE USED UNLESS NOTED OTHERWISE. TWIST-OFF TYPE TENSION-CONTROL A.S.T.M. F1852 BOLT ASSEMBLIES MAY BE SUBSTITUTED FOR A.S.T.M. A325 BOLTS AT ANYTIME. WHERE A.S.T.M. A490 BOLTS ARE SPECIFIED ON THE PLANS TWIST-OFF TYPE TENSION-CONTROL A.S.T.M. F2280 BOLT ASSEMBLIES MAY BE SUBSTITUTED AT ANYTIME.
6. A.S.T.M. F1554 GRADE 36 ANCHOR BOLTS SHALL BE USED UNLESS NOTED OTHERWISE. A.S.T.M. F1554 GRADE 36 BOLTS MAY BE WELDED, WHILE GRADE 55 MAY BE WELDED ONLY IF IT IS ORDERED WITH SUPPLEMENT S1 AND THE CARBON EQUIVALENT FORMULA SPECIFIED IN SECTION S.1.5.2.1 OF THE A.S.T.M. GRADE 105 BOLTS MAY NOT BE WELDED.
7. A.S.T.M. A563 HEAVY-HEX NUTS SHALL BE USED UNLESS NOTED OTHERWISE. SEE A.S.T.M. A563 FOR THE APPROPRIATE GRADE AND FINISH OF THE NUTS WHICH VARY ACCORDING TO APPLICATION.
8. A.S.T.M. F436 WASHERS SHALL BE USED UNLESS NOTED OTHERWISE. AT SLIP CRITICAL CONNECTIONS A.S.T.M. F959 COMPRESSIBLE-WASHER-TYPE DIRECT TENSION INDICATORS SHALL BE USED UNLESS TWIST-OFF-TYPE TENSION-CONTROL BOLT ASSEMBLIES WITH A.S.T.M. F436 WASHERS ARE USED.
9. A.S.T.M. A108 SHEAR STUD CONNECTORS SHALL BE USED UNLESS NOTED OTHERWISE. THE MECHANICAL REQUIREMENTS MUST MEET A.W.S. D1.1 TABLE 7.1 FOR TYPE B SHEAR STUDS CONNECTORS. (Fy=50 ksi, Fu=65 ksi)
10. HOLES PUNCHED OR DRILLED IN BEAMS SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE ON THE DRAWING: HOLES FOR BOLTS SHALL BE 1/16" LARGER THAN THE NOMINAL DIAMETER OF THE BOLT WHERE CONNECTION IS OF SHEAR TYPE, AND 3/16" LARGER WHERE CONNECTION IS OF BEARING TYPE ON CONCRETE OR MASONRY.
11. ALL STRUCTURAL STEEL AND MISCELLANEOUS STEEL SHALL RECEIVE ONE SHOP COAT OF RED OXIDE, ZINC CHROMATE OR APPROVED EQUAL BASE.
12. ALL STEEL MEMBERS AND THEIR CONNECTIONS, EXPOSED TO EARTH OR WEATHER SHALL BE HOT DIPPED GALVANIZED, UNLESS NOTED OTHERWISE. CAP ALL VENT HOLES AT ENCLOSED HSS STEEL SECTIONS AND SEAL HOLES TO A WATER TIGHT CONDITION.
13. ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO THE FOLLOWING A.I.S.C. STANDARDS:
   - W (WIDE FLANGE) AND WT SHAPES SHALL BE A.S.T.M. A992 (Fy=50 ksi, Fu=65 ksi)
   - M, MT, S AND ST SHAPES SHALL BE A.S.T.M. A36 (Fy=36 ksi, Fu=58 ksi)
   - CHANNELS, ANGLES, PLATES 1/2" THICK OR LESS AND MISCELLANEOUS STEEL, SHALL BE A.S.T.M. A36. (Fy=36 ksi, Fu=58 ksi)
   - PLATES GREATER THAN 1/2" THICK SHALL BE A.S.T.M. A572 GR. 50. (Fy=50 ksi, Fu=65 ksi)
   - HP SHAPES SHALL BE A.S.T.M. A572 (Fy=50 ksi, Fu=65 ksi)
   - RECTANGULAR AND SQUARE HSS SHALL BE A.S.T.M. A500 GRADE B (Fy=46 ksi, Fu=58 ksi)
   - ROUND HSS SHALL BE A.S.T.M. A500 GRADE B (Fy=42 ksi, Fu=58 ksi)
   - PIPE SHALL CONFORM TO A.S.T.M. A53 GRADE B (Fy=35 ksi, Fu=60 ksi)
14. ALL ENDS OF EXPOSED STRUCTURAL SHAPES ± HOLLOW STRUCTURAL SHAPED STEEL MEMBERS SHALL HAVE 1/4" CAP PLATE WITH PARTIAL PENETRATION WELDS, U.N.O., GRIND SMOOTH, A.E.S.S.
15. ALL STEEL BEAMS SHALL HAVE INSTALLED STANDARD MILL TOLERANCE UP, TYP., U.N.O.
16. FILLER METAL AND WELDING FLUX: E70XX IN ACCORDANCE WITH A.W.S. D1.1-2010.
17. WHERE LATERAL FORCE RESISTING COLUMNS OCCUR, A DOUBLE NUT SHALL BE PROVIDED ABOVE THE BASE PLATE AT ALL THREADED ROD/BOLT LOCATIONS.
18. CONTRACTOR SHALL PROVIDE AN ALLOWANCE EQUAL TO 2% OF THE BID FOR STRUCTURAL STEEL, MISCELLANEOUS IRON, AND REINFORCING STEEL TO BE USED AT THE DISCRETION OF THE STRUCTURAL ENGINEER. UNUSED PORTION TO REVERT TO THE OWNER UPON COMPLETION OF THE PROJECT.

2 STEEL NOTES
S1.03 | NOT TO SCALE



NOTE:
CONNECTION APPLIES TO ALL STEEL COLUMN SHAPES. SEE PLAN FOR COLUMN INFO.

3 FLAT PLATE COLUMN CONNECTION
S1.03 | NOT TO SCALE



4 BENT BEAM SPLICE
S1.03 | NOT TO SCALE

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ARCHITECTURE ENGINEERING INTERIOR DESIGN CONSTRUCTION MANAGMENT
8011 N. Fresno, Suite 130 - Fresno, California 93710
Phone: (559) 436-0881 Fax: (559) 436-0887 E-Mail: design@somam.com
www.integrateddesigns.com

Revision:	Revision Description:	Rev. Date:

Sheet Title: TYPICAL STEEL DETAILS
Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
Date:
Designer: Dasdigner
DR:
PIC:

Agency Approval Stamp:

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OAKLAND R. RAMOS
No. 57356
CIVIL
01-31-20
STRUCTURAL
STATE OF CALIFORNIA

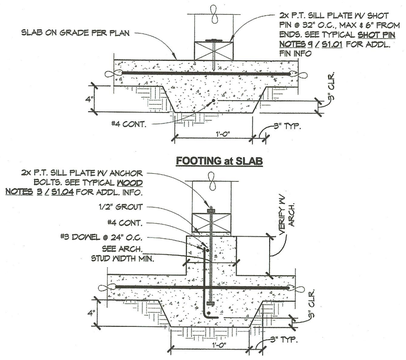
BROOKE-RANDOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711
17800

Job No.:

Sheet No.: S1.03

Release:

0029	Steel Bldg & Cantilevers	Xxrs Steel (13) Please reference attached. 1. Approver commenced to Ridge Construction detail 16/S3.03 for UL Planning 505 steel skeletons. Please confirm the typical connection plate for UL beam is 1/2" per floor soqe. 2. We have Yield of wood beams at connection of roof wood width and bottom of framing topped wood beams (right to work point as shown typically at all points is in correct location.	1. Stopped connection plate OK per structural; 2. Work is correct. Please confirm our interpretation is correct.	Closed
0049	Steel Cantilevers	Xxrs Steel (19) Please reference attached 1. 2 the clouded wood beam is fixed with work point at corner (as shown in snap-1). Top of concrete is 27'-10 1/2" (for per RH WOZ response room in page 2) and pipe at 3 1/2" per foot at 27'-9 5/8". But if we fix top of wood at 27'-6 5/8", as per architect comment. There will be a gap between the wood bearing (refer snap-2) of 5/8" RH 13 is bearing to 5/8ow xxrs work point at corner as per snap-1. 2 -2 we know detail 16/S1.03 is a standard detail used on similar projects with similar finishes without issue. Please confirm that the column connection is per detail 8/S1.03. Please provide a suitable connection arrangement.	1. Proceed with 17-9 3/8" as top of beam overdon. 2.Xxrs detail 16/S1.03, but use a 6" nominal dimension from the face of plate to center of bolts. For detail 8/S1.03, use Simpson ECCQQ8-SDS2.5 and offset away from the column connection adjacent.	Closed
0053	Ramps	Xxrs Steel (21) Please reference attached 1! we know detail 16/S1.03 is an approver corrected (reference page 2) for the wood header to toilet column connection. The plates and bolts not side mount connection will protect though wall finish and also may clash with 2. Please ensure the door opening at brown above. Please advise a suitable solution or confirm it is okay to proceed with detail 1B/S6.01 (reference page 3) for all wood header to column connections.	Detail 16/S1.03 is a standard detail used on similar projects with similar finishes without issue. Proceed with the detail as shown on approved drawings.	Closed



FOOTING at SLAB

FOOTING at CURB

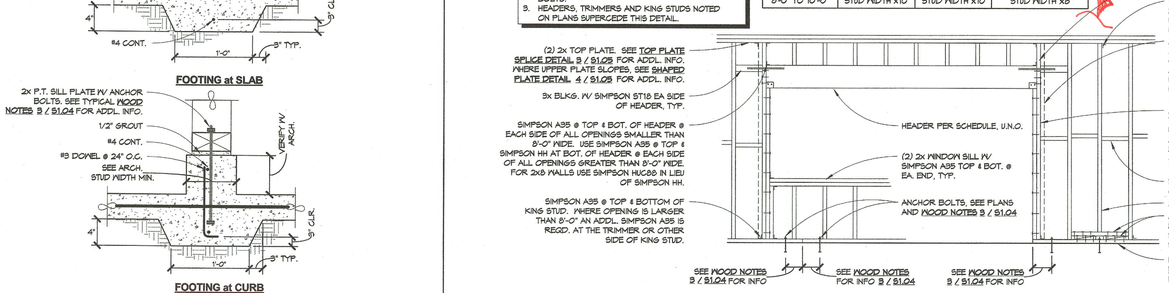
17 NON-BEARING WALL CONN. FOR WOOD

S1.04 NOT TO SCALE

NOTES:
1. WHERE METAL FASTENERS AND HARDWARE ARE IN CONTACT WITH PRESERVATIVE-TREATED WOOD, SEE WOOD NOTES 3 / S1.04 FOR ADDITIONAL REQUIREMENTS.
2. ALL WALL FRAMING REQUIREMENTS APPLY AT MULTI-LEVEL FRAMING OR OVER RAISED WOOD FLOORS, EXCEPT FOR P.T. SILL AND ANCHOR BOLTS.
3. HEADERS, TRIMMERS AND KING STUDS NOTED ON PLANS SUPERCEDE THIS DETAIL.

TYPICAL HEADER SIZE FOR OPENINGS, U.N.O.

OPENING WIDTH	2x8 LOAD BEARING WALL	2x6 LOAD BEARING WALL	NON-LOAD BEARING WALL PARTITIONS
0'-0" TO 3'-11"	STUD WIDTH x3	STUD WIDTH x3	STUD WIDTH x4
4'-0" TO 7'-11"	STUD WIDTH x3	STUD WIDTH x3	STUD WIDTH x6
8'-0" TO 10'-0"	STUD WIDTH x10	STUD WIDTH x10	STUD WIDTH x8



ATTACH KING STUDS TO HEADER W/ 4) 16d NAILS EACH END, USE (4) SIMPSON SDS 1/4x6 AT 4x KING POST

FOR EXTERIOR STUDS 4 WOOD POST WITH HEIGHT GREATER THAN 14'-0", USE SIMPSON A35 @ TOP & BOT., U.N.O.

AT OPENINGS 4'-0" OR LESS USE (1) 2x KING STUD. AT 4'-0" TO 8'-0" OPENINGS USE (2) 2x KING STUDS. AT OPENINGS OVER 8'-0" USE (1) 4x KING POST. ATTACH MULTIPLE 2x KING STUDS TOGETHER WITH 16d @ 18"O.C. STAGGERED.

AT OPENINGS 8'-0" OR LESS USE (1) 2x TRIMMER. AT OPENINGS OVER 8'-0" USE (1) 4x OR (2) 2x TRIMMER. ATTACH 2x TRIMMERS TO KING STUDS WITH 16d @ 18"O.C. STAGGERED AND 4x TRIMMERS WITH SIMPSON SDS 1/4x6 @ 16"O.C. STAGGERED.

MIN. 3x BLOCKING STAGGERED @ 8'-0" O.C. VERTICALLY MAX., U.N.O.

2x STUDS PER PLAN @ 16" O.C., U.N.O.

ADDL. 2x BLOCKING SHALL BE USED WHERE ANCHOR BOLT OCCURS AT STUD LOCATION. USE (3) 10d @ EACH SIDE OF STUD.

P.T. SILL PLATE. SEE WOOD NOTES 3 / S1.04 & FOUNDATION PLANS

13 WALL FRAMING AT OPNGS. FOR WOOD FRAMING

S1.04 NOT TO SCALE

012 Steel Clarification #2

Contractor: Kern Steel (3)Please reference attached.1. Please reference page 1.2, & 3 of attached sketch. Since there is no sufficient information for framing of clouded CS Column, we are providing the same, supported by wood header over door opening to roof elevation as shown on page 3. Please confirm our interpretation is correct. Please also furnish the profile for wood header. Also note that we are not providing the column to top of parapet although it is shown in high roof plan as shown in page 1.2. Please reference page 4 of attached sketch. Per clouded structural notes, anchor bolt needs to be provided with double nut and 3/16" diameter washer plate for lateral force resisting columns. However structural drawings do not indicate the lateral force resisting columns. Please furnish hole for lateral force resisting columns. However structural drawings do not indicate the lateral force resisting columns. Please furnish column location where clouded notes are applicable. Also note that we have followed 3/16" oversized hole and provided two nuts at top

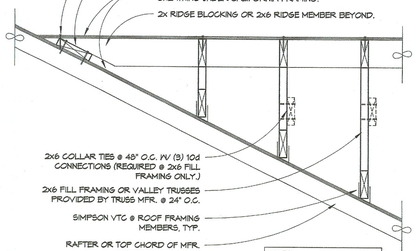
013 Steel Clarifications #3

Contractor: Kern Steel (4)Please reference attached.1. Refer page-1 of attached sketch. Since design drawings do not have sufficient information regarding sheathing elevation at clouded location, please furnish top of sheathing elevation at face of wall. This is needed to fix the top of columns.2. Refer page-2 & 3 of attached sketch. At yellow highlighted areas, we have fixed top of sheathing at bottom of leading roof deck elevation (refer page-3) at face of 8"-0" stud wall. Please confirm that our interpretation is correct.3. Refer page-2 & 4 of attached sketch. At red highlighted area, we are fixing bottom of sheathing/ top of framing steel ridge at 23'-8" and 18'-6" as shown (refer page-4). Please confirm that our interpretation is correct and advise if any changes needed.

Wood header is to be 8x8 per detail 13/S1.04. Provide column to top of parapet per RFI 11. Double nuts will not be required for any columns on this project.

Closed

Update: Related CCD 002 was sent to the DSA for Approval
See the attached plans, submitted in the RFI, showing the corrected elevations/dimensions in red.



NOTE: BRACE 2x6 FILL FRAMING TO ROOF FRAMING BELOW @ 8'-0" O.C. MAX.

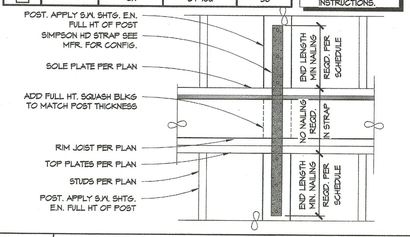
18 CALIFORNIA FILL FRAMING

S1.04 NOT TO SCALE

HOLDOWN SCHEDULE

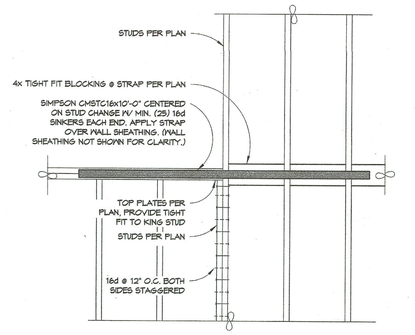
MARK	SIMPSON HOLDOWN TYPE	MIN. POST SIZE (U.N.O. ON PLANS)	FASTENERS AT EACH END	END LENGTH (IN.)
F1	MST48	4x	16-16d	15
F2	MST60	4x	28-16d	21
F3	MST72	4x	28-16d	27
F4	CMST14	6x	28-16d	30
F5	CMST12	6x	37-16d	36

NOTES:
1. APPLY STRAP OVER WALL SHEATHING. WALL SHEATHING NOT SHOWN FOR CLARITY.
2. REFER TO THE LATEST EDITION OF THE SIMPSON STRONG-TIE CATALOG FOR HOLDOWN INSTALLATION INSTRUCTIONS.



19 FLR TO FLR HOLDOWN SCHEDULE

S1.04 NOT TO SCALE



20 STRAP AT VARIABLE TOP PLATES

S1.04 NOT TO SCALE

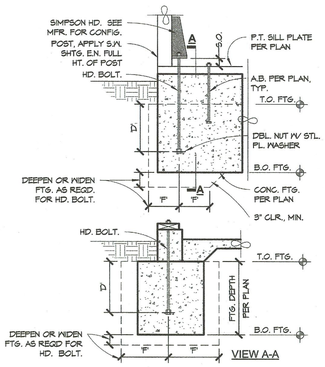
14 HOLDOWN SCHEDULE

S1.04 NOT TO SCALE

HOLDOWN SCHEDULE

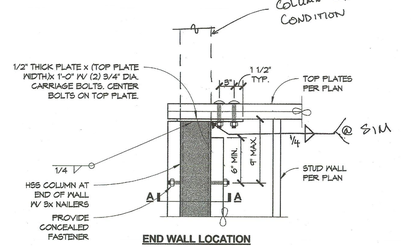
MARK	SIMPSON HOLDOWN TYPE	MIN. POST SIZE (U.N.O. ON PLANS)	STAND OFF (S.O.) (INCHES)	HOLDOWN BOLT TYPES: SIMPSON PRE-ASSEMBLED ANCHOR BOLT (PAB) BOLT DIA. (IN.)	WASHER SIZE (IN x IN x IN THICK)	MIN. EFFECTIVE DEPTH D (IN)	MIN. COVER F (IN)
A	HDU2-SDS2.5	(2) 2x *	1 3/8	5/8	1 1/2 x 1 1/2 x 3/8	6	9
B	HDU4-SDS2.5	(2) 2x *	1 3/8	5/8	1 1/2 x 1 1/2 x 3/8	6	9
C	HD5B	4x	3 1/8	3/4	2 x 2 x 3/8	8	12
D	HDU5-SDS2.5	4x	1 3/8	5/8	1 1/2 x 1 1/2 x 3/8	6	9
E	HD7B	4x	2 1/8	7/8	2 1/4 x 2 1/4 x 3/8	10	15
F	HDU8-SDS2.5	6x	1 1/2	7/8	2 1/4 x 2 1/4 x 3/8	10	15
G	HDQ8-SDS3	6x	2 3/8	7/8	2 1/4 x 2 1/4 x 3/8	15	23
H	HDU11-SDS2.5	6x	1 1/2	1	2 1/2 x 2 1/2 x 3/8	12	18
I	HHDQ11-SDS2.5	6x	7/8	1	2 1/2 x 2 1/2 x 3/8	12	18
J	HD9B	6x	3 5/8	7/8	2 3/4 x 2 3/4 x 3/8	13	20
K	HHDQ14-SDS2.5	6x6	7/8	1	2 1/2 x 2 1/2 x 3/8	18	27
L	HDU14-SDS2.5	6x6	1 9/16	1	2 1/2 x 2 1/2 x 3/8	18	27
M	HD12	6x6	3 5/8	1 1/8	2 3/4 x 2 3/4 x 3/8	13	20
N	HD19	6x6	3 5/8	1 1/4	3 x 3 x 1/2	16	24

1. REFER TO THE LATEST EDITION OF THE SIMPSON STRONG-TIE CATALOG FOR HOLDOWN INSTALLATION INSTRUCTIONS, AND FOR THE NUMBER AND TYPE OF FASTENERS REQUIRED FOR EACH TYPE OF HOLDOWN.
2. HOLDOWN BOLTS DO NOT REPLACE ADJACENT TYPICAL ANCHOR BOLTS.
3. WHERE BOLTS ATTACH HOLDOWN TO THE ADJACENT POST, NO COUNTER SINKING OF THE BOLTS IS ALLOWED.
4. DOUBLE NUTS SHALL BE SNUG TIGHT TO STEEL PLATE WASHER.
5. HOLDOWN BOLTS MUST BE TIED SECURELY INTO POSITION PRIOR TO THE POURING OF CONCRETE.
6. DOUBLE 2x POSTS MUST BE STITCH NAILED FULL HEIGHT WITH 10d @ 6" O.C., STAGGERED.

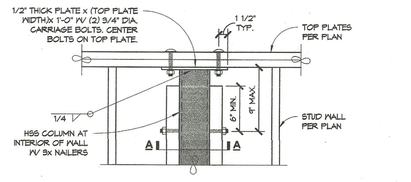


15 STEEL COLUMN WITH NAILERS IN WOOD FRMG.

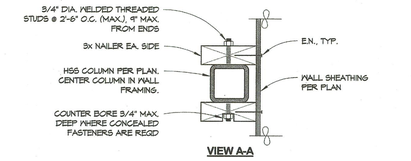
S1.04 NOT TO SCALE



END WALL LOCATION



INTERIOR WALL LOCATION

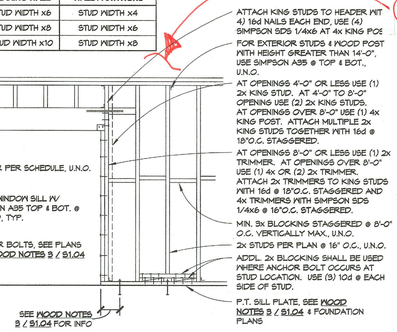


VIEW A-A

REF#27

16 STEEL COLUMN WITH NAILERS IN WOOD FRMG.

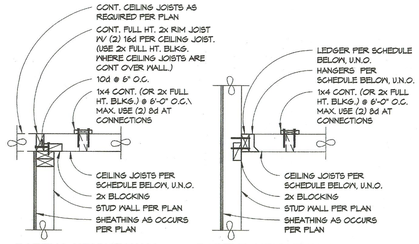
S1.04 NOT TO SCALE



NOTES:
1. SHEAR WALLS LESS THAN ONE VERT. PANEL IN HEIGHT DO NOT REQUIRE STAGGERED PANELS.
2. SEE WOOD NOTES 3 / S1.04 AND SHEARWALL SCHEDULE 4 / S1.04 FOR ADDL. INFO.
3. SEE TYP. WALL FRAMING DETAIL 13 / S1.04 FOR WALL CONST. INFO, U.N.O.

11 SHEAR WALL ELEVATION (SEGMENTED)

S1.04 NOT TO SCALE



PARTIAL HEIGHT WALL

PARTIAL HEIGHT WALL

CEILING JOIST SCHEDULE

MAX SPAN.	SIZE/SPACING	SIMPSON HANGER	LEDGE SIZE / ATTACHMENT
UP TO 8'-0"	2x4 @ 16" O.C.	LUS24	2x4 PV (2) 16d PER STUD
8'-1" TO 14'-0"	2x6 @ 16" O.C.	LUS26	2x6 PV (2) 16d PER STUD
14'-0" TO 18'-0"	2x8 @ 16" O.C.	LUS28	2x8 PV (3) 16d PER STUD

12 CEILING JOIST SCHEDULE

S1.04 NOT TO SCALE

7 NAILING SCHEDULE PER C.B.C. TABLE 2304.10.1

S1.04 NOT TO SCALE

NAILING SCHEDULE *

#	CONNECTION	FASTENING *a&f*	LOCATION
1.	JOIST TO SILL OR GIRDER	3 - 8d	TOENAIL
2.	BRIDGING TO JOIST	2 - 8d	TOENAIL EACH END
3.	1"x6" SUBFLOOR OR LESS TO EACH JOIST	2 - 8d	FACE NAIL
4.	1"x8" SUBFLOOR OR GREATER TO EACH JOIST	3 - 8d	FACE NAIL
5.	2" SUBFLOOR TO JOIST OR GIRDER	2 - 16d	BLIND & FACE NAIL
6.	SOLE PLATE TO JOIST OR BLOCKING	16d @ 16" O.C.	TYPICAL FACE NAIL
	SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL	3 - 16d @ 16"O.C.	BRACED WALL PANELS
7.	TOP PLATE TO STUD	2- 16d	END NAIL
8.	STUD TO SOLE PLATE	4 - 8d 2 - 16d	TOENAIL END NAIL
9.	DOUBLE STUDS	16d @ 24" O.C.	FACE NAIL
10.	DOUBLE 2x TOP PLATE	16d @ 16" O.C. 8 - 16d	FACE NAIL LAP SPLICE
	3x TOP PLATE OVER 2X PLATE	20d @ 16"O.C. 8 - 20d	FACE NAIL LAP SPLICE
11.	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	3 - 8d	TOENAIL
12.	RIM JOIST TO TOP PLATE	8d @ 6" O.C.	TOENAIL
13.	TOP PLATES, LAPS & INTERSECTIONS	2 - 16d	FACE NAIL
14.	CONTINUOUS HEADER, TWO PIECES	16d	16" O.C. @ EDGE
15.	CEILING JOISTS TO PLATE	3 - 8d	TOENAIL
16.	CONTINUOUS HEADER TO STUD	4 - 8d	TOENAIL
17.	CEILING JOISTS LAPS OVER PARTITIONS	3 - 16d MIN. TABLE 2308.10.4.1	FACE NAIL
18.	CEILING JOISTS TO PARALLEL RAFTERS	3 - 16d TABLE 2308.10.4.1	FACE NAIL
19.	RAFTER TO PLATE	3 - 8d	TOENAIL
20.	1" DIAGONAL BRACE TO EACH STUD AND PLATE	2 - 8d	FACE NAIL
21.	1"x8" SHEATHING TO EA. BEARING	3 - 8d	FACE NAIL
22.	WIDER THAN 1"x8" SHEATHING TO EACH BEARING	3 - 8d	FACE NAIL
23.	BUILT-UP CORNER STUDS	16d	24" O.C.
24.	BUILT-UP GIRDER AND BEAMS	20d @ 32" O.C.	FACE NAIL @ TOP AND BOT. STAG. ON OPPOSITE SIDES
		2 - 20d	FACE NAIL @ ENDS AND @ EA. SPLICE
25.	2" PLANKS	16d	@ EACH BEARING
26.	COLLAR TIE TO RAFTER	3 - 10d	FACE NAIL
27.	JACK RAFTER TO HIP	3 - 10d 2 - 16d	TOENAIL FACE NAIL
28.	ROOF RAFTER TO 2x RIDGE BEAM	2 - 16d 2 - 16d	TOENAIL FACE NAIL
29.	JOIST TO BAND JOIST	3 - 16d	FACE NAIL
30.	LEDGER STRIP	3 - 16d	FACE NAIL
31.	WOOD STRUCTURAL PANELS AND PARTICLEBOARD, SUBFLOOR, ROOF AND WALL SHEATHING (TO FRAMING)*f*	1/2" & LESS 6d *f* 19/32" to 3/4" 8d, 6d 7/8" to 1" 8d 1 1/8" to 1 1/4" 10d, 8d	-
	SINGLE FLOOR (COMBINATION SUBFLOOR-UNDERLAYMENT TO FRAMING)*g*	3/4" & LESS 6d 7/8" to 1" 8d 1 1/8" to 1 1/4" 10d / 8d	-
32.	PANEL SIDING (TO FRAMING)	1/2" & LESS 6d *g* 5/8" 8d *g*	-
33.	FIBERBOARD SHEATHING*d*	1/2" 6d 25/32" 8d	-
34.	INTERIOR PANELING*h*	1/4" to 3/8" 6d	-

a. COMMON NAILS ARE REQUIRED TO BE USED U.N.O. COMMON NAIL PROPERTIES ARE AS FOLLOWS:
- 6d = 0.113"Ø x 2" LONG
- 8d = 0.131"Ø x 2 1/2" LONG
- 10d = 0.148"Ø x 3" LONG
- 16d = 0.162"Ø x 3 1/2" LONG
- 20d = 0.192"Ø x 4" LONG

b. NAILS SPACED AT 6" O.C. AT EDGES, 12" AT INTERMEDIATE SUPPORTS EXCEPT 6" AT SUPPORTS WHERE SPANS ARE 48" OR MORE. FOR NAILING OF WOOD STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO SECTION 2305.

c. CORROSION-RESISTANT SIDING (6d = 1 7/8"X0.106"; 8d = 2 3/8"X0.128") OR CASING (6d = 2"X0.099"; 8d = 2 1/2"X0.113") NAIL.

d. FASTENERS SPACED 3" O.C. AT EXTERIOR EDGES AND 6" O.C. AT INTERMEDIATE SUPPORTS, WHEN USED AS STRUCTURAL SHEATHING. SPACING SHALL BE 6" O.C. E.N. 4 12" O.C. F.N. FOR NONSTRUCTURAL APPLICATIONS.

e. NAILS SPACED 6" ON PANEL EDGES, 12" AT INTERMEDIATE SUPPORTS.

f. ROOF SHEATHING APPLICATIONS, 8d ARE THE MINIMUM REQUIRED FOR WOOD STRUCTURAL PANELS.

g. NAILING DRIVEN INTO PRESERVATIVE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED OR EQUIVALENT.

h. THIS SCHEDULE WILL GOVERN UNLESS NOTED OTHERWISE ON PLANS.

i. FASTENERS USED FOR THE ATTACHMENT OF EXTERIOR WALL COVERINGS SHALL BE OF HOT-DIPPED ZINC-COATED GALVANIZED STEEL, WITH COATING PER A.S.T.M. A153.

8 LVL NOTES LAMINATED VENEER LUMBER

S1.04 NOT TO SCALE

1. ALL LVL TO BE 'REDLAM' AS MANUFACTURED BY REDBUILT, OR EQUIVALENT. SEE STRUCTURAL PLANS FOR SPECIFIC INFORMATION ON THE LVL'S BEING USED ON THIS PROJECT. SUBSTITUTE LSL'S MUST BE SUBMITTED AND APPROVED BY D.S.A./O.S.H.P.D. AS A CHANGE ORDER PRIOR TO INSTALLATION. THE CONTRACTOR SHALL BEAR ALL COSTS OF OBTAINING APPROVAL OF SUBSTITUTES, WITH NO GUARANTEE THAT THE SUBSTITUTE WILL BE ACCEPTED.
2. DESIGN AND FABRICATION SHALL CONFORM TO THE REQUIREMENTS OF THE 2016 C.B.C. AND I.C.C. E.S. REPORT E.S.R.-2988.
3. ALLOWABLE STRESS INCREASES FOR LOAD DURATION SHALL BE:
   - WIND OR SEISMIC 1.6
   - ROOF AND SNOW 1.15
   - FLOOR 1.0
4. ALLOWABLE STRESS INCREASES FOR REPETITIVE MEMBERS SHALL NOT BE TAKEN GREATER THAN 1.0 PER N.D.S.-15 SECTION 6.3.7.
5. NO HOLES OR NOTCHES OF ANY KIND ARE ALLOWED IN ANY LVL'S, UNLESS APPROVED BY THE STRUCTURAL ENGINEER.
6. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS SHOWN ON STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS, AND THEN VERIFY IN FIELD WALL LAYOUT DIMENSIONS. NOTIFY THE ENGINEER AND ARCHITECT PRIOR TO PROCEEDING IF ANY DISCREPANCIES EXIST.
7. LVL MEMBERS SHALL MEET THE MINIMUM PROPERTIES SHOWN BELOW:
   - Fb = 2,700 PSI
   - Fv = 285 PSI
   - Fc (PERPENDICULAR) = 750 PSI
   - Fc (PARALLEL) = 2,510 PSI
   - E = 2,000,000 PSI
   - Emin. = 968,710 PSI

3 WOOD NOTES

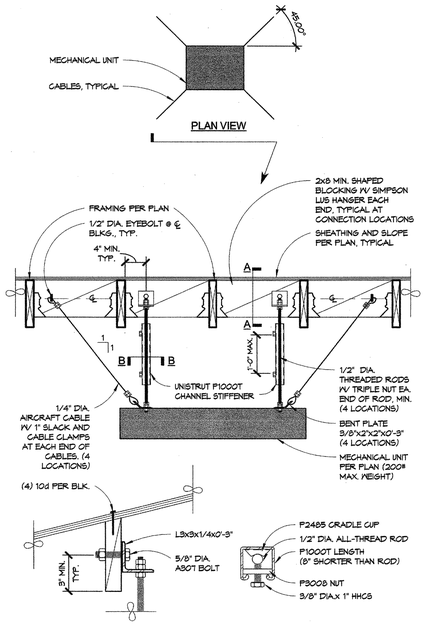
S1.04 NOT TO SCALE

1. ALL BOLTS SHALL BE MACHINE MADE TYPE F1554 GRADE 36 U.N.O.
2. BOLT HOLES IN WOOD SHALL BE OVERSIZED BY NOT MORE THAN 1/32".
3. ALL BOLTS AND LAG SCREWS SHALL BE PROVIDED WITH STANDARD STEEL WASHERS UNDER HEAD AND NUTS WHICH BEAR ON WOOD ACCORDING TO THE WASHER SCHEDULE BELOW, U.N.O.

WASHER SCHEDULE

BOLT SIZE	STEEL PLATE SQUARE	MALLEABLE IRON ROUND	STANDARD CUT WASHER
1/2"Ø	2 x 2 x 1/4"	2 1/2"Ø x 1/4"	1 9/16"Ø x 7/64"
5/8"Ø	2 1/2 x 2 1/2 x 1/4"	2 3/4"Ø x 5/16"	1 3/4"Ø x 1/8"
3/4"Ø	3 x 3 x 5/16"	3"Ø x 3/8"	2"Ø x 5/32"
7/8"Ø	3 1/2 x 3 1/2 x 5/8"	3 1/2"Ø x 7/16"	2 1/4"Ø x 11/64"
1"Ø	3 3/4 x 3 3/4 x 7/16"	4"Ø x 1/2"	2 1/2"Ø x 11/64"
1 1/8"Ø	4 x 4 x 7/16"	4 1/2"Ø x 9/16"	2 3/4"Ø x 11/64"
1 1/2"Ø	4 1/4 x 4 1/4 x 1/2"	5"Ø x 5/8"	3 1/2"Ø x 9/16"

4. BOLTS AND SCREWS SHALL BE TIGHTENED AT TIME OF ERECTION AND RETIGHTENED BEFORE CLOSING IN OR AT COMPLETION OF JOB.
5. ANCHOR AND/OR SILL BOLTS WITH UPSET THREADS ARE NOT PERMITTED.
6. ALL SILL PLATES SHALL BE BEDDED ON 1/2" CEMENT GROUT EXCEPT FOR NON-BEARING INTERIOR WALLS ON TRUE AND LEVEL FLOORS.
7. SILL PLATES UNDER ALL EXTERIOR AND BEARING WALLS SHALL BE BOLTED TO MASONRY OR CONCRETE WITH 5/8"Ø x 12" BOLTS SPACED NOT MORE THAN 4'-0" ON CENTER, WITH A MIN. OF 2 BOLTS FOR EACH PIECE OF SILL PLATE, U.N.O. SHEAR WALLS ABOVE 2 STORIES SHALL HAVE BOLTS SPACED NOT MORE THAN 4'-0" O.C., U.N.O. ALL SILL PLATE ANCHOR BOLTS SHALL HAVE MINIMUM 3" SQ. x 1/4" SLOTTED PLATE WASHERS (OR SIMPSON BPS) WITH A STANDARD CUT WASHER PLACED BETWEEN THE PLATE WASHER AND THE NUT, U.N.O. THE PLATE WASHER SHALL EXTEND TO WITHIN 1/2" OF THE EDGE OF THE BOTTOM PLATE ON THE SIDE(S) WITH SHEATHING.
8. ALL ANCHOR BOLTS IN WOOD SHALL BE SPACED 9 INCH MINIMUM AND 12 INCH MAXIMUM FROM END OF THE SILL PLATE, AND HAVE 7 INCH MINIMUM EMBEDMENT INTO CONCRETE OR MASONRY. ANY LOCATION WHERE A HOLE OR NOTCH LARGER THAN THE SILL PLATE THICKNESS OCCURS, SHALL HAVE ADDITIONAL ANCHOR BOLTS PLACED 4 INCHES TO 12 INCHES ON EACH SIDE OF THE HOLE OR NOTCH.
9. SILL PLATES AT ALL EXTERIOR WALLS AND INTERIOR WALLS THAT ARE COVERED WITH STRUCTURAL PLYWOOD (SHEAR PANEL) SHALL BE PRESERVATIVE-TREATED D.F. 5x THICK WITH THE SAME WIDTH AS STUDS, U.N.O. ALL OTHER WALLS THE SAME EXCEPT SILL PLATES MAY BE 2x THICK. ALL PRESERVATIVE-TREATED D.F. SHALL BEAR THE APWA QUALITY MARK. ALL CUTS OR HOLES SHALL BE PRE-TREATED PRIOR TO INSTALLATION.
10. NO SILL PLATE PIECE SHALL END WITHIN THE LENGTH OF SHEAR PANEL UNLESS SPECIFICALLY SHOWN AND DETAILED ON THE PLANS.
11. ALL EXPOSED FASTENERS SHALL HAVE ZINC-COATING CORROSION RESISTANCE.
12. ALL FASTENERS AND HARDWARE IN CONTACT WITH PRESERVATIVE-TREATED OR FIRE RETARDANT WOOD SHALL BE HOT DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE OR COPPER. THE COATING WEIGHT FOR ZINC-COATED FASTENERS SHALL BE IN ACCORDANCE WITH A.S.T.M. A 153. EXCEPTION: FASTENERS OTHER THAN NAILS, TIMBER RIVETS, WOOD SCREWS AND LAG SCREWS SHALL BE PERMITTED TO BE OF MECHANICALLY DEPOSITED ZINC COATED STEEL WITH COATING WEIGHTS IN ACCORDANCE WITH A.S.T.M. B 695, CLASS 55 MIN.
13. BEARING WALLS AND PARTITIONS SHALL HAVE DOUBLE TOP PLATES.
14. ALL FASCIA BOARDS MUST BE CONTINUOUS WITH NO SPLICES ALLOWED WITHIN 12'-0" MINIMUM FROM FRAMING CORNERS, UNLESS NOTED OTHERWISE.
15. PLYWOOD NOTES:
   - IN HORIZONTAL PLYWOOD DIAPHRAGMS, NO PANEL LESS THAN 24" WIDE SHALL BE USED. IN VERTICAL PLYWOOD SHEAR WALLS, NO PANEL LESS THAN 12" WIDE SHALL BE USED. PLYWOOD SHEETS SHALL NOT BE LESS THAN 8 SQUARE FEET AT ANY LOCATION. PROVIDE FULL SHEETS OF PLYWOOD WHEREVER POSSIBLE.
   - ANY PIECE OF PLYWOOD SPANNING ACROSS FEWER THAN 3 SUPPORTS SHALL BE BLOCKED ON ALL EDGES.
   - SHEAR WALL PLYWOOD SHALL BE BLOCKED AT ALL EDGES.
   - DIAPHRAGM AND SHEAR WALL NAILING SHALL CONFORM TO SECTION 2306.3 OF C.B.C. 2016.
   - NAILS SHALL NOT BE OVER DRIVEN AS TO CAUSE CRUSHING OF FACEPLY.
   - WHERE GLUING OF PLYWOOD IS REQUIRED, INSURE THAT CONTACT SURFACES ARE FREE OF DIRT, DUST, STANDING WATER OR OTHER DELETERIOUS MATTER. APPLY A BEAD OF GLUE ABOUT 1/4 INCH IN DIAMETER TO ALL CONTACT/ BEARING SURFACES. ON WIDE AREAS, APPLY GLUE IN SERPENTINE PATTERN. APPLY TWO BEADS OF GLUE ON JOISTS WHERE PANEL ENDS BUTT TO EACH OTHER. APPLY GLUE PROGRESSIVELY TO BUTTING EDGES OF PANELS AND INTO THE GROOVED EDGES OF TONGUE AND GROVE PANELS AS WORK PROGRESSES. ADHESIVE SHALL CONFORM TO A.P.A. SPEC AFG-01.
15. ALL STRUCTURAL WOOD SHALL CONFORM WITH THE FOLLOWING SPECIFICATIONS:
   - DOUGLAS FIR - LARCH ..... WCLIB OR WWPA RULES
   - PLYWOOD ..... U.S. PRODUCT STANDARD PS1-07 FOR STRUCTURAL PLYWOOD
16. MINIMUM GRADES SHALL BE AS FOLLOWS U.N.O. ON DRAWINGS:
   - STRUCTURAL FRAMING. ..... DF NO. 1 OR BETTER
   - 4x AND LARGER AND POST. ..... DF

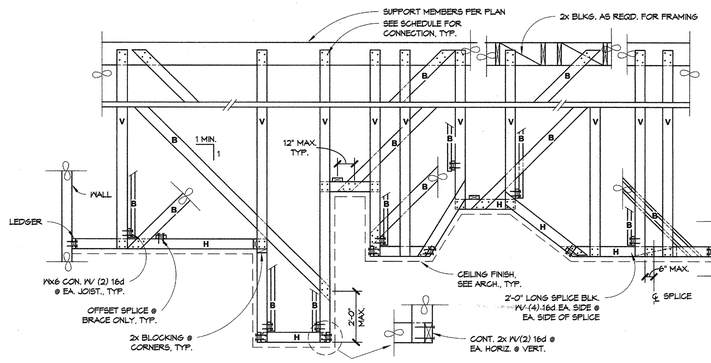


18 HANGING MECH. UNIT AT WOOD FRMG.
S1.05 NOT TO SCALE

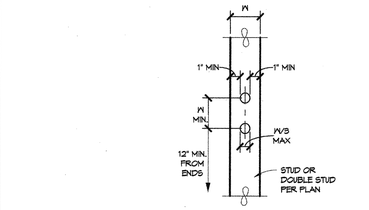
SOFFIT FRAMING SCHEDULE				
	H	V	B	LEDGER
MAX. O.C. MEMBER SPACING	2'-0"	2'-0"	8'-0"	-
MAX. LENGTH OF MEMBER	< 8'-0"	8'-0"	< 8'-0"	-
	8'-1" TO 12'-0" *		8'-1" TO 14'-0" **	
REQUIRED FASTENERS @ CONNECTIONS	(4) 16d OR SIMPSON LUS' HANGER	(4) 16d	(4) 16d	(2) 16d @ 16" O.C. @ WOOD STUDS AND 5/8" DIA. A.B. @ 48" O.C. @ CONC. & MASONRY

SCHEDULE ASSUMES:
- 2x4 DF #2 MEMBERS U.N.O.
- 15.0 PSF D.L. + 10 PSF L.L.
- MAX D.L. DEFLECTION = L/360
- (*) INDICATES 2x6 DF #2 MEMBER REQUIRED.
- (**) INDICATES DBL. 2x4 W/ 16d @ 12" O.C. STITCH NAILING IN 'I' SHAPE REQUIRED.

NOTES:
1. BRACES REQUIRED AT ALL VERTICAL OFFSETS. ALTERNATE DIRECTION OF BRACE WHERE POSSIBLE
2. SEE SCHEDULE FOR SPANS AND CONNECTIONS

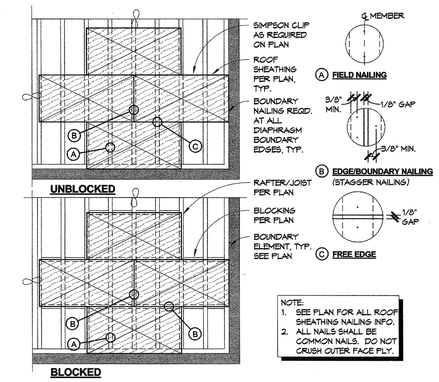


13 SOFFIT FRAMING FOR WOOD CONSTRUCTION
S1.05 NOT TO SCALE



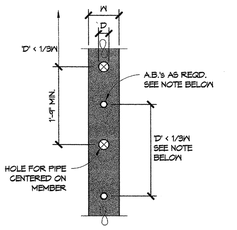
NOTES:
1. HOLES SHALL BE DRILLED.
2. HOLES SHALL BE CENTERED ON STUD/DOUBLE STUD.
3. WHERE THE MIN. 1" EDGE DISTANCE IS NOT MAINTAINED A SIMPSON H6S IS REQUIRED. SEE LATEST EDITION OF SIMPSON CATALOG FOR INSTALLATION INFORMATION.
4. MAX DIAMETER OF HOLE MAY BE INCREASED FROM W/3 TO 2W/3 IF STUD IS DOUBLED, AND NOT MORE THAN 2 SUCCESSIVE DOUBLED STUDS ARE DRILLED.

1 HOLES IN WOOD STUDS
S1.05 NOT TO SCALE



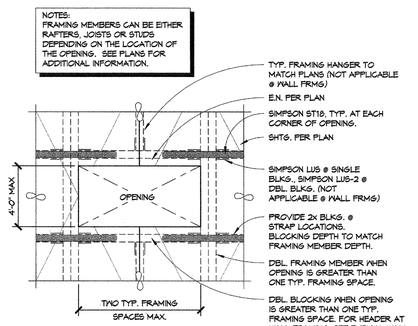
NOTE:
1. SEE PLAN FOR ALL ROOF SHEATHING NAILING INFO.
2. ALL NAILS SHALL BE COMMON NAILS. DO NOT CRUSH OUTER FACE PLY.

14 ROOF SHEATHING FOR WOOD DIAPHRAGMS
S1.05 NOT TO SCALE

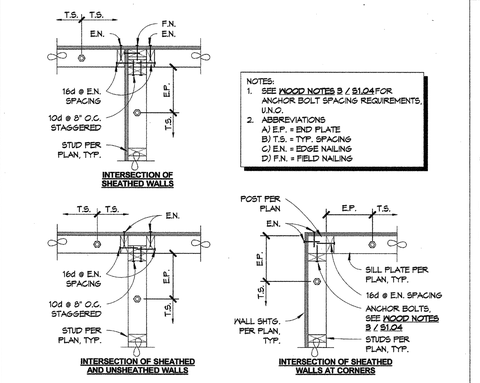


NOTES:
1. FOR 'D' GREATER THAN 1/3 WIDTH PROVIDE ANCHOR BOLTS AT 4" O.C. MAX ± 6" O.C. MIN. AT EACH SIDE OF HOLE. PROVIDE MIN. OF (2) A.B. PER SILL PIECE.
2. FOR 'D' GREATER THAN 1/3 WIDTH PROVIDE SIMPSON CTS218 ON INTERIOR SIDE OF SILL PLATE.
3. FOR MULTIPLE HOLES ℄ NOTCHES, MAINTAIN 18" CLEAR MIN. ℄ EXTEND STRAP 12" PAST LAST HOLE OR NOTCH.

10 OPENING IN SILL PLATE
S1.05 NOT TO SCALE

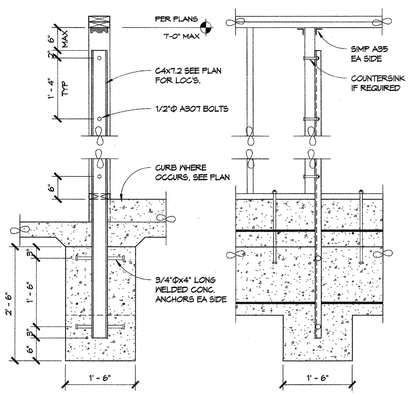


6 BLOCKING AROUND OPENING
S1.05 NOT TO SCALE

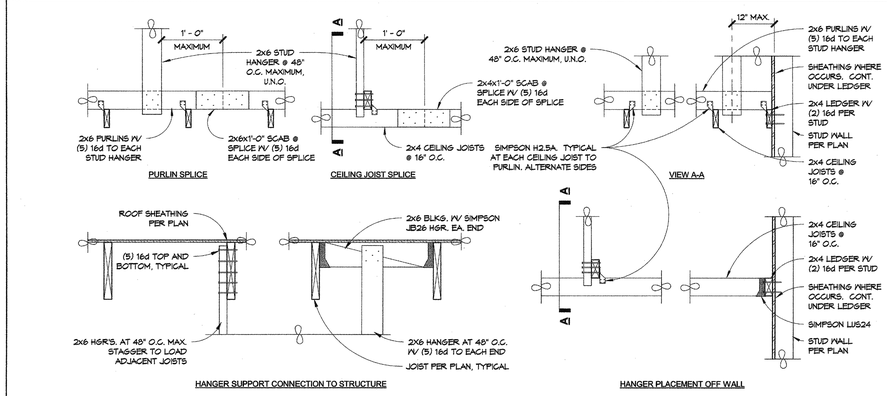


NOTES:
1. SEE WOOD NOTES 9 / S1.04 FOR ANCHOR BOLT SPACING REQUIREMENTS, U.N.O.
2. ABBREVIATIONS
A) E.P. = END PLATE
B) T.S. = TYP. SPACING
C) E.N. = EDGE NAILING
D) F.N. = FIELD NAILING

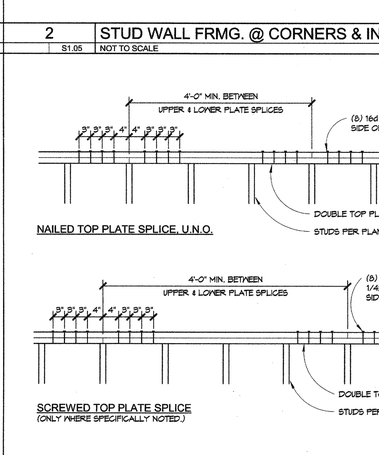
2 STUD WALL FRMG. @ CORNERS & INTERS.
S1.05 NOT TO SCALE



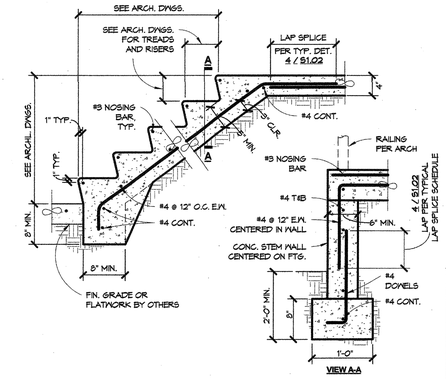
19 EXTERIOR KNEE WALL
S1.05 NOT TO SCALE



15 CEILING SUPPORT FRAMING TO WOOD JOIST
S1.05 NOT TO SCALE

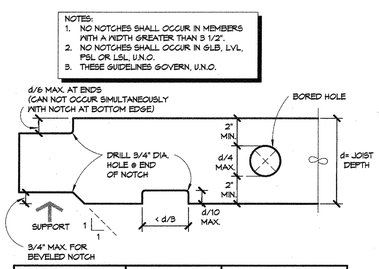


3 TOP PLATE SPLICES FOR WOOD
S1.05 NOT TO SCALE



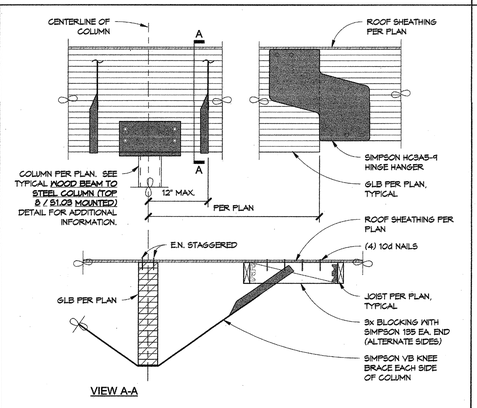
20 CONCRETE STAIRS ON GRADE
S1.05 NOT TO SCALE

NOTES:
1. NO NOTCHES SHALL OCCUR IN MEMBERS WITH A WIDTH GREATER THAN 3 1/2".
2. NO NOTCHES SHALL OCCUR IN GLB, LVL, PSL OR LSL, U.N.O.
3. THESE GUIDELINES GOVERN, U.N.O.

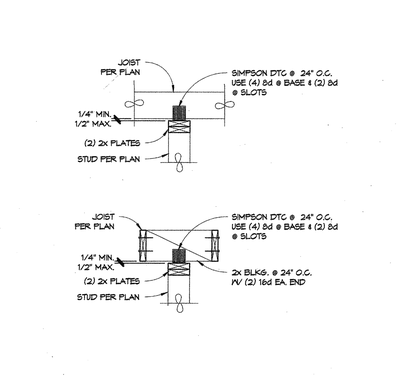


LOCATION	MAX BORED HOLE DIAMETER	MAX DEPTH OF NOTCH (TOP OR BOT.)
WITHIN 3d FROM SUPPORT	NOT PERMITTED	d/10
FROM 3d FROM SUPPORT TO 1/3 POINT OF SPAN	d/4	d/10
WITHIN 3d FROM SUPPORT	d/4	NOT PERMITTED

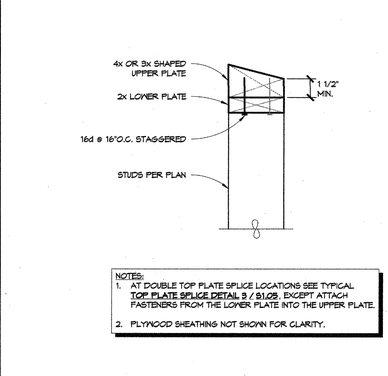
16 ALLOWABLE NOTCHES AND HOLES IN 2x OR 4x
S1.05 NOT TO SCALE



12 GLB TO GLB HINGE CONNECTION
S1.05 NOT TO SCALE



8 NON-BEARING WALL CONNECTION
S1.05 NOT TO SCALE



NOTES:
1. AT DOUBLE TOP PLATE SPLICE LOCATIONS SEE TYPICAL TOP PLATE SPLICE DETAIL 3 / S1.05, EXCEPT ATTACH FASTENERS FROM THE LOWER PLATE INTO THE UPPER PLATE.
2. PLYWOOD SHEATHING NOT SHOWN FOR CLARITY.

4 SHAPED TOP PLATE
S1.05 NOT TO SCALE

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6011 N. Fresno, Suite 130 - Fresno, California 93710
Phone (559) 436-0881 Fax (559) 436-0887 E-Mail: design@somam.com
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Sheet Title: TYPICAL WOOD DETAILS
Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

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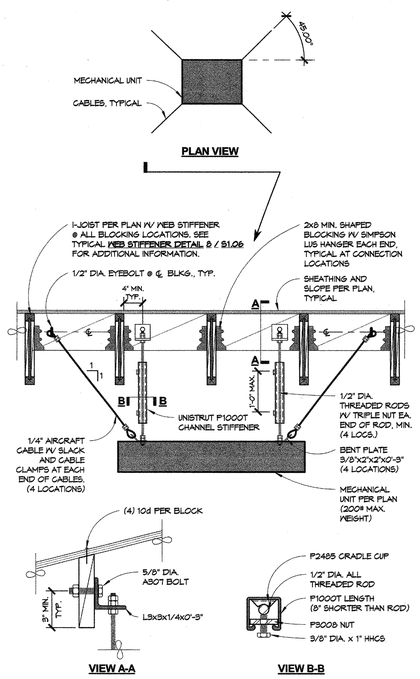
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711

Job No.:

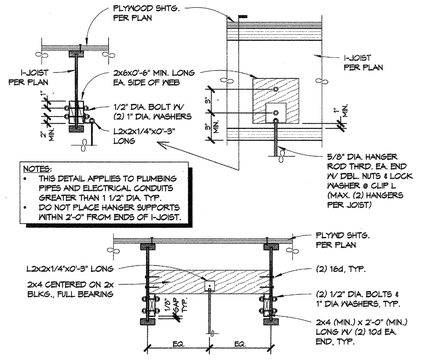
Sheet No.: S1.05

Release:



18 HANGING MECHANICAL UNIT AT I-JOISTS

S1.06 NOT TO SCALE



19 MECHL HANGER SUPPORT

S1.06 NOT TO SCALE

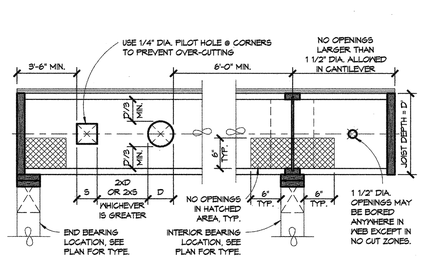
ANCHORAGE OF ALL EQUIPMENT TO BE INSTALLED, AS A PART OF THIS PROJECT SHALL BE DETAILED ON THESE PLANS, EXCEPT FOR THE FOLLOWING:

1. EQUIPMENT WEIGHING LESS HAN 400 POUNDS SUPPORTED DIRECTLY ON THE FLOOR OR ROOF.
2. FURNITURE (NON FIXED AND MOVABLE AS EXEMPTED BY C.B.C. 2016 CHAPTER 1 SECTION 105.2)
3. TEMPORARY OR MOVABLE EQUIPMENT NON FIXED AND MOVABLE AS EXEMPTED BY THE C.B.C. 2016 CHAPTER 1 SECTION 105.2).
4. EQUIPMENT WEIGHING LESS THAN 20 POUNDS SUPPORTED BY VIBRATION ISOLATORS.
5. EQUIPMENT WEIGHING LESS THAN 20 POUNDS SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL.

PERMANENT EQUIPMENT N ITEMS 1, 4, AND 5 MUST BE SUPPORTED AND ANCHORED TO RESIST THE FORCED THE PRESCRIBED BY CHAPTER 13 OR ASCE 7 AS MODIFIED BY THE C.B.C. 2016 SECTIONS 1619A/1510 AND THE ANCHORAGE SHALL BE APPROVED BY THE APPROPRIATE DESIGN PROFESSIONAL OF RECORD AND D.S.A./O.S.H.P.D. AS A PART OF FIELD REVIEWS/OBSERVATIONS. THE INSPECTOR OF RECORD SHALL ASSURE THAT THE ABOVE REQUIREMENTS ARE ENFORCED.

20 EQUIPMT. ANCHORAGE REQ'MT.

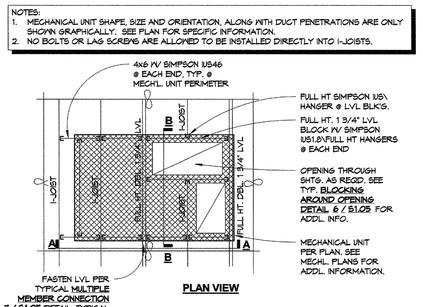
S1.06 NOT TO SCALE



NOTES:
1. ALL OPENINGS LARGER THAN 1 1/2" DIA. MUST BE PLACED IN CENTER THIRD OF I-JOIST DEPTH.
2. MAXIMUM SQUARE OPENING SIZE (S) IS 4" SQUARE.
3. MAXIMUM ROUND OPENING SIZE (D) IS 5" DIA.
4. DO NOT CUT FLANGE.
5. WHERE LARGER OPENINGS ARE REQUIRED CONTACT STRUCTURAL ENGINEER PRIOR TO CUTTING FOR ADDITIONAL REINFORCEMENT INFORMATION.
6. VERIFY ALL OPENINGS WITH MANUFACTURER REQUIREMENTS PRIOR TO INSTALLATION.

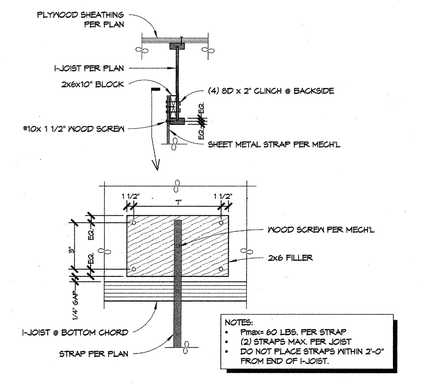
13 ALLOWABLE OPENINGS IN I-JOISTS

S1.06 NOT TO SCALE



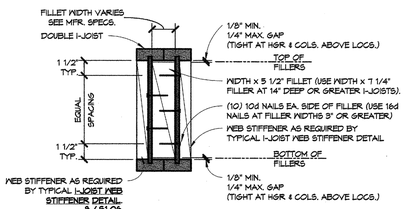
15 MECHANICAL UNIT ATTACHMENT FOR I-JOISTS

S1.06 NOT TO SCALE



16 MECH'L DUCT HANGER @ "I" JOIST

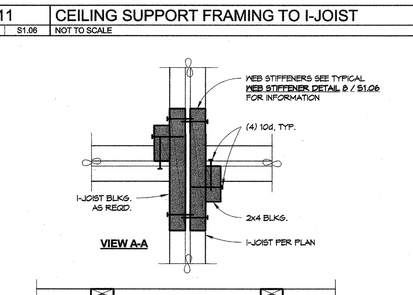
S1.06 NOT TO SCALE



NOTES:
1. WEB FILLER TO BE MFR. OR FIELD INSTALLED AT 24" O.C. OR AT JOIST HANGER SPACING, WHICHEVER IS LESS.
2. SEE MFR. SPECIFICATIONS FOR WEB FILLER THICKNESSES AND FASTENER INFORMATION.
3. IF MFR. FASTENER INFORMATION DIFFERS FROM THIS DETAIL, FOLLOW THE MANUFACTURER'S REQUIREMENTS.

9 DOUBLE I-JOIST FILLER

S1.06 NOT TO SCALE



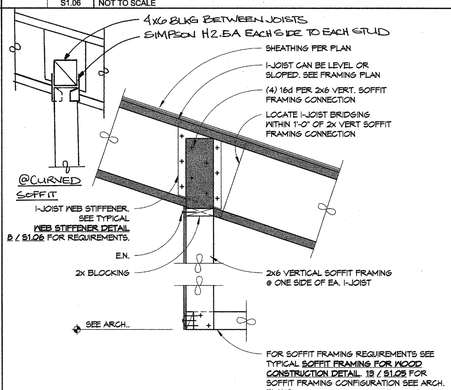
NOTES:
1. INSTALL BRIDGING TIGHTLY.
2. ADJACENT BRIDGING SHALL NOT TOUCH ONE ANOTHER.
3. LOWER ENDS OF CROSS BRIDGING SHALL BE DRIVEN UP AND NAILED AFTER PLYWOOD SHEATHING HAS BEEN NAILED OFF.

10 I-JOIST BRIDGING

S1.06 NOT TO SCALE

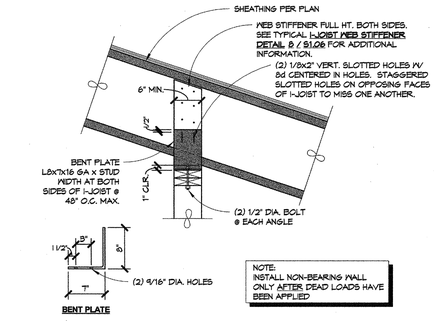
12 I-JOIST BLOCKING

S1.06 NOT TO SCALE



6 SOFFIT FRAMING TO I-JOISTS

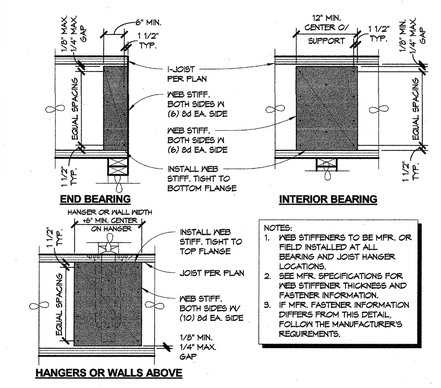
S1.06 NOT TO SCALE



NOTE:
INSTALL NON-BEARING WALL ONLY AFTER DEAD LOADS HAVE BEEN APPLIED

5 NON-BEARING WALL TO I-JOIST (SLOPED)

S1.06 NOT TO SCALE



8 I-JOIST WEB-STIFFENERS

S1.06 NOT TO SCALE

NOTES:
1. WEB STIFFENERS TO BE MFR. OR FIELD INSTALLED AT ALL BEARING AND JOIST HANGER LOCATIONS.
2. SEE MFR. SPECIFICATIONS FOR WEB STIFFENER THICKNESS AND FASTENER INFORMATION.
3. IF MFR. FASTENER INFORMATION DIFFERS FROM THIS DETAIL, FOLLOW THE MANUFACTURER'S REQUIREMENTS.

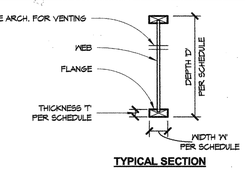
4 NON-BEARING WALL TO I-JOIST

S1.06 NOT TO SCALE

1. SEE DESIGN CRITERIA FOR ALL DESIGN LOADS.
2. ALL I-JOISTS TO BE AS MANUFACTURED BY REDBUILT, OR EQUIVALENT. SEE I-JOIST SCHEDULE BELOW AND STRUCTURAL PLANS FOR SPECIFIC INFORMATION ON THE I-JOISTS BEING USED ON THIS PROJECT. SUBSTITUTE I-JOISTS MUST BE SUBMITTED AND APPROVED BY D.S.A. AS A CHANGE ORDER PRIOR TO FABRICATION. THE CONTRACTOR SHALL BEAR ALL COSTS OF OBTAINING APPROVAL OF SUBSTITUTES, WITH NO GUARANTEE THAT THE SUBSTITUTE WILL BE ACCEPTED.
3. ALL CHORD MATERIAL SHALL BE LAMINATED VENEER LUMBER (LVL), NO M.S.R. ALLOWED.
4. ALL WEB MATERIAL SHALL BE A.P.A. RATED STRUCT. 1 PLYWOOD, OR "PERFORMANCE PLUS" OSB, MEETING P.S.1.-07 EXPOSURE 1.
5. DESIGN AND FABRICATION SHALL CONFORM TO THE REQUIREMENTS OF THE 2016 C.B.C., A.S.T.M. D5055-04, I.C.C. E.S. REPORT E.S.R.-2994 AND D.S.A. I.R. 23-4 REVISED ON 07-15-08 FOR SCHOOL PROJECTS.
6. ALLOWABLE STRESS INCREASES FOR LOAD DURATION SHALL BE:
   - WIND OR SEISMIC 1.6
   - ROOF OR SNOW 1.15
   - FLOOR 1.0
7. ALLOWABLE STRESS INCREASES FOR REPETITIVE MEMBERS SHALL BE NOT BE TAKEN AS GREATER THAN 1.0 PER N.D.S.-15 SECTION 1.3.6.
8. I-JOIST MANUFACTURE'S SHOP DRAWINGS, WITH DIMENSIONS, AND CALCULATIONS SHALL BE SUBMITTED TO THE ARCHITECT AND/OR STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION. THE MANUFACTURER SHALL VERIFY ALL DIMENSIONS SHOWN ON STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS, AND THEN VERIFY IN FIELD WALL LAYOUT DIMENSIONS PRIOR TO FABRICATION.
9. I-JOIST MANUFACTURE'S CALCULATIONS SHALL BE SIGNED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED.
10. I-JOISTS SHALL BE DESIGNED FOR ALL CONCENTRATED LOADS SHOWN ON DRAWINGS IN ADDITION TO THE UNIFORM LOADINGS SHOWN IN THE DESIGN CRITERIA.
11. SEE STRUCTURAL FRAMING PLANS AND MECHANICAL PLANS FOR LOCATION AND WEIGHT OF MECHANICAL UNITS. COORDINATE ADDITIONAL EQUIPMENT LOADS WITH THE MECHANICAL CONTRACTOR PRIOR TO FABRICATION.
12. ALL FLOOR JOISTS AND ROOF RAFTERS SUPPORTING PLASTER SHALL BE DESIGNED FOR A MAXIMUM TOTAL LOAD DEFLECTION OF L/240 AND A MAXIMUM LIVE SNOW OR WIND LOAD DEFLECTION OF L/360 PER C.B.C. TABLE 1604A.3. EFFECTS OF LONG TERM CREEP SHALL BE
13. I-JOIST MANUFACTURER TO PROVIDE WEB STIFFENERS ON EACH SIDE OF WEB AT ALL BEARING/SUPPORT LOCATIONS.
14. PROVIDE I-JOIST BLOCKING AT ALL BOTTOM CHORD BEARING CONDITIONS AND WHERE NOTED ON DRAWINGS.
15. GENERAL CONTRACTOR TO PROVIDE TEMPORARY ERECTION BRACING AND WEB BRACING AS REQUIRED BY I-JOIST MANUFACTURER.
16. EACH I-JOIST SHALL BE STAMPED WITH THE JOIST TYPE, MFR.'S NAME, # PLANT NO.
17. WHERE DOUBLE I-JOISTS ARE CALLED FOR ON PLANS, PROVIDE LOAD TRANSFER BLOCKINSPER TYPICAL DOUBLE I-JOIST FILLER DETAIL. 9 / S1.06
18. I-JOIST DESIGNER SHALL INCORPORATE ONE 400# VERTICAL LOAD ON THE BOTTOM CHORD OF ALL JOISTS FROM ONE SPRINKLER LINE. JOIST MUST BE DESIGNED TO CARRY THE LOAD AT ANY POINT.
19. FIRE SPRINKLER INSTALLER SHALL USE CONNECTORS WITH I.C.C. APPROVAL. NO 'COACH' TYPE SCREW CONNECTORS WILL BE ALLOWED AT ANY LOCATION.
20. THE I-JOIST MANUFACTURER IS REQUIRED TO PERFORM AN INSTALLATION REVIEW. UPON COMPLETION OF THAT REVIEW, THE MANUFACTURER SHALL SEND A LETTER TO THE ENGINEER OF RECORD STATING THAT THE REVIEW WAS PERFORMED, THE PRODUCTS WERE INSTALLED PERTHE MANUFACTURER'S RECOMMENDATIONS AND ALL WARRANTIES ARE IN EFFECT. THE INSTALLATION REVIEW IS TO BE PERFORMED NO LATER THAN 30 DAYS AFTER THE PRODUCTS HAVE BEEN INSTALLED. FAILURE TO PERFORM WILL RESULT IN THE FORFEITURE OF MANUFACTURER'S FUTURE PRODUCT ACCEPTANCE.

I-JOIST SCHEDULE

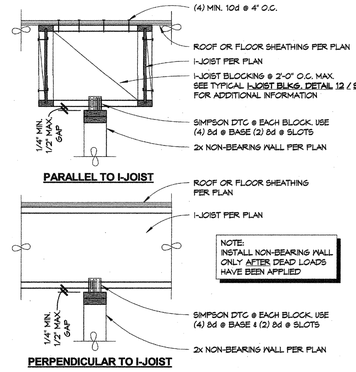
MARK #	D (in)	FLANGE ("T"xW")	FLANGE GRADE E (psi)	FLANGE GRADE Fb (psi)	WEB (in)	RESIST MOMENT (ft-lbs)	RESIST SHEAR (lbs)	EI x 10^6 (in^2-lbs)
RJ-01	11 7/8	1 1/2x2 1/2	1.9x10^6	2600	7/16	6,750	2,255	450
RJ-02	14	1 1/2x2 1/2	1.9x10^6	2600	7/16	8,080	2,540	666
RJ-03	16	1 1/2x3 1/2	1.9x10^6	2600	7/16	9,210	2,810	913
RJ-04	18	1 1/2x2 1/2	1.9x10^6	2600	7/16	10,380	3,080	1,205



TYPICAL SECTION

3 I-JOIST NOTES

S1.06 NOT TO SCALE



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Phone: (559) 435-3661 Fax: (559) 435-3662 e-Mail: design@somam.com
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Rev. Date:	Revision Description:

Sheet Title: TYPICAL I-JOIST DETAILS

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
Date:
Designer: Designer
DR:
PC:

Agency Approval Stamp:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC. FLS SS
DATE 06-22-18

TRACKING #: 63321-300

Stamp(s)

REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RANSOM
No. S2846
EXPIRES 3-31-20
STRUCTURAL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 103
FRESNO, CALIFORNIA 93711

17900

Job No.:

Sheet No.: S1.06

Release:

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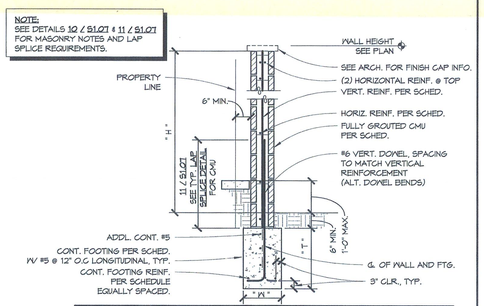
NOTE:
SEE DETAILS 10 / S1.07 & 11 / S1.07 FOR MASONRY NOTES AND LAP SPLICE REQUIREMENTS.



WALL SCHEDULE

H	BLOCK THICKNESS	W	T	VERTICAL REINF.		FOOTING REINF.
8'-0" MAX.	8"	1'-0"	5'-0"	#5 @ 16" O.C.	#5 @ 24" O.C.	(2) #5
6'-0" MAX.	8"	1'-0"	5'-0"	#5 @ 16" O.C.	#5 @ 24" O.C.	(2) #5

APPROVED
DIV. OF THE STATE ARCHITECT
LOS ANGELES REGIONAL OFFICE
BY TN FLS ✓ AC ✓
DATE 5/16/19

REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RANSOM
No. 52386
EXPIRES 1-31-20
CIVIL
CALIFORNIA

16 | FREE STANDING MASONRY WALL
AD4-S1 | NOT TO SCALE

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS - CIVIL ENGINEERS
7419 N. MAPLE AVE., SUITE 103
FRESNO, CALIFORNIA 93711
P:(559)449-8444 - F:(559)449-8404

NEW E.S. INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

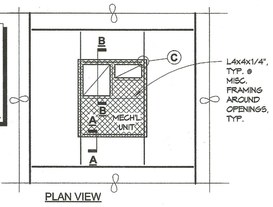
DRAWN BY SKDC
DATE 12/14/18
PROJ. # 17600

SHEET AD4-S

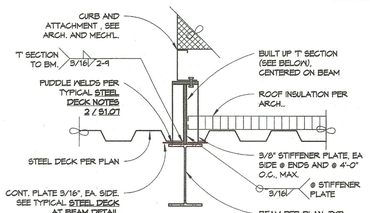
18 MECH'L UNIT CURB SUPPORT FOR STL. FRAMING

S1.07 NOT TO SCALE

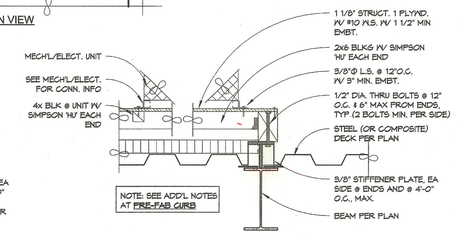
NOTES:
1. SEE TYPICAL MECHANICAL UNIT SUPPORT FRAMING B / S1.07 DETAIL FOR ADDITIONAL INFORMATION.
2. MECHANICAL UNIT SHAPE, SIZE AND ORIENTATION ARE ONLY SHOWN GRAPHICALLY. SEE PLAN FOR SPECIFIC INFORMATION.



PLAN VIEW

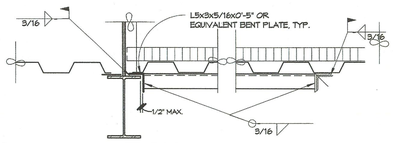


AT PRE-FAB CURB



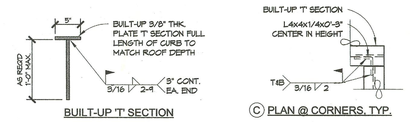
AT WOOD PLATFORM

VIEW A-A



VIEW B-B

(SIMILAR @ ANGLE TO ANGLE CONNECTIONS)



BUILT-UP 'T' SECTION

Ⓒ PLAN @ CORNERS, TYP.

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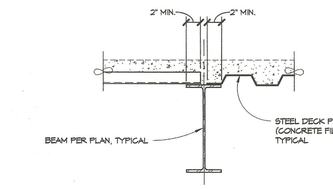
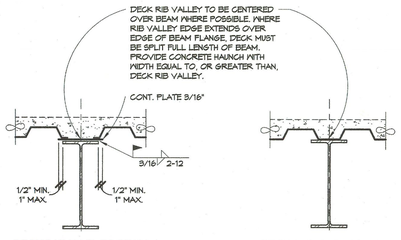
20 STEEL DECK AT BEAM

S1.07 NOT TO SCALE

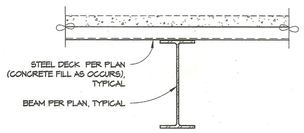


DECK TRANSITION AT BEAM



DECK PARALLEL TO BEAM (SPLIT DECK OVER BEAM)

DECK PARALLEL TO BEAM (RIB OVER BEAM)



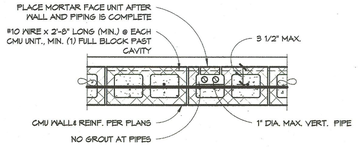
DECK PERPENDICULAR TO BEAM

NOTE:
ALL DECK ATTACHMENTS PER TYPICAL STEEL DECK NOTES 2 / S1.07

15 VENT PIPES IN CMU WALL

S1.07 NOT TO SCALE

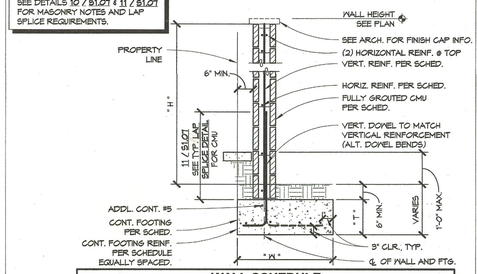
NOTES:
1. ONLY VERTICAL VENT PIPES ARE ALLOWED IN CMU WALLS. NO WATER, GAS, STEAM, OR ANY OTHER LINES ARE ALLOWED EXCEPT AS SPECIFICALLY NOTED ON PLANS. NO HORIZONTAL VENT LINES ARE TO BE PLACED IN CMU WALLS.
2. PIPES SHALL NOT INTERFERE WITH ANY REINFORCEMENT.
3. PIPES SHALL NOT BE PLACED WITHIN 2'-0" FROM ENDS OF CMU WALL OR OPENINGS.
4. CONDUITS IN MASONRY SHALL BE NO CLOSER THAN 3 DIAMETERS ON CENTER.
5. VERTICAL CONDUITS ARE ALLOWED TO JOG A MAXIMUM OF 8" HORIZONTALLY.
6. VERTICAL CONDUITS PLACED IN MASONRY SHALL NOT DISPLACE MORE THAN 2 OF NET CROSS SECTION. (EXAMPLE:(1) 1/2" DIA. PIPE SHALL BE SPACED 2'-0", OR (2) 1/2" DIA. PIPE SHALL BE SPACED NO LESS THAN 5'-4".
7. CONTRACTOR SHALL SUBMIT A PROPOSED CONDUIT LAYOUT PRIOR TO INSTALLATION.



16 FREE STANDING MASONRY WALL (PROP. LINE)

S1.07 NOT TO SCALE

NOTE:
SEE DETAILS 10 / S1.07 & 11 / S1.07 FOR MASONRY NOTES AND LAP SPLICE REQUIREMENTS.



WALL SCHEDULE						
H	BLOCK THICKNESS	W	T	VERTICAL REINF.	HORIZ. REINF.	FOOTING REINF.
10'-0" MAX.	8"	6'-6"	1'-6"	#5 @ 8" O.C.	#5 @ 24" O.C.	(4) #5
8'-0" MAX.	8"	5'-3"	1'-6"	#5 @ 16" O.C.	#5 @ 24" O.C.	(7) #5
6'-0" MAX.	8"	4'-0"	1'-6"	#5 @ 24" O.C.	#5 @ 24" O.C.	(5) #5

10 MASONRY NOTES

S1.07 NOT TO SCALE

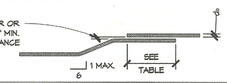
1. CONCRETE MASONRY UNITS SHALL BE NORMAL WEIGHT, TYPE "N" AS SET FORTH IN A.S.T.M. 90. THE FOLLOWING MINIMUM STRENGTH FOR TYPE 1, MOISTURE CONTROLLED UNITS SHALL BE:
   - 8" CMU f'm = 2000 PSI
   - 10" CMU f'm = 2000 PSI
   - 12" CMU f'm = 2000 PSI
   - 16" CMU f'm = 2000 PSI
2. SEE ARCHITECTURAL DRAWINGS FOR COLOR & SURFACE TREATMENT OF EXPOSED CMU UNITS.
3. MASONRY, EXCEPT MASONRY VENEER, SHALL BE CONSTRUCTED WITHIN THE TOLERANCES SPECIFIED IN A.C.I. 530.1/A5.C.E. 6/T.M.S. 602.
4. MASONRY UNITS SHALL BE LAID IN RUNNING BOND, U.N.O.
5. MORTAR SHALL CONFORM TO A.S.T.M. C270 AND BE TYPE "S" WITH A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS WITH MORTAR PROPORTIONS PER A.S.T.M. C270 TABLE 1. LIME SHALL BE THE LAST MATERIAL ADDED TO THE MIXER.
6. GROUT SHALL CONFORM TO A.S.T.M. C476 AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. THE WATER CONTENT EXPRESSED ON A SATURATED SURFACE-DAY BASIS SHALL NOT EXCEED 0.1 TIMES THE WEIGHT OF CEMENT. AGGREGATE FOR GROUT SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN A.S.T.M. C404.
7. GROUT SHOULD BE PUDDLED OR TAMPED WITH A 5/8" ROD OR A 1"X2" STICK AS IT IS PLACED.
8. ALL CELLS SHALL BE GROUTED SOLID BY EITHER A LOW LIFT (4'-0" MAX.) OR HIGH-LIFT GROUTING METHOD. LOW-LIFT GROUTED CONSTRUCTION MUST COMPLY WITH THE REQUIREMENTS OF C.B.C. 2104A.1.3.1.2.2. HIGH-LIFT GROUTED CONSTRUCTION MUST HAVE THE APPROVAL OF THE STRUCTURAL ENGINEER AND D.S.A./O.S.H.P.D., AND MUST COMPLY WITH THE REQUIREMENTS OF C.B.C. 2104A.1.3.1.2.3. IN EITHER GROUTING METHOD, THE GENERAL CONDITIONS OF 2104A.1.3.1.2. ALSO APPLY.
9. GROUTING OF BEAMS OVER OPENINGS SHALL BE DONE IN CONTINUOUS OPERATION WITH A MINIMUM 12" SEAT AT EACH END UNLESS PLACED IN TOTAL WITH WALL GROUTING. ALL WALL OPENINGS SHALL BE SHORED FOR A MINIMUM OF 28 DAYS AFTER COMPLETION OF GROUTING, U.N.O.
10. ALL REINFORCING SHALL CONFORM TO A.S.T.M. A615 AND SHALL BE GRADE 40 FOR #3 BAR, GRADE 60 FOR #4 BAR AND LARGER.
11. ALL WELDING OF REINFORCING STEEL SHALL BE WITH LOW HYDROGEN ELECTRODES U.N.O. WELDING OF REINFORCING SHALL BE ALLOWED ONLY WHERE DETAILED ON DRAWINGS. ALL WELDING SHALL BE DONE IN ACCORDANCE WITH AMERICAN WELDING SOCIETY SPECIFICATIONS A.W.S. D1.4. WELDING SHALL NOT BE DONE WITHIN TWO BAR DIAMETERS OF ANY BENT PORTION OF A BAR WHICH HAS BEEN BENT COLD. WELDING OF CROSSING BARS SHALL NOT BE PERMITTED FOR ASSEMBLY OF REINFORCEMENT UNLESS AUTHORIZED BY THE STRUCTURAL ENGINEER OF RECORD. A.S.T.M. A706 REINFORCING SHALL BE USED FOR ALL REINFORCING THAT IS BEING WELDED.
12. VERTICAL REINFORCING SHALL BE CENTERED IN WALL, U.N.O., AND PLACED IN OPEN END UNITS BEFORE BLOCK WORK BEGINS. VERTICAL REINFORCING SHALL BE HELD IN POSITION AT TOP, BOTTOM AND AT INTERVALS NOT EXCEEDING 192 BAR DIAMETERS.
13. HORIZONTAL REINFORCING SHALL BE LAID IN BOND BEAM UNITS AND SECURELY WIRED TO THE VERTICAL REINFORCING.
14. DOWELS IN ALL FOUNDATIONS SHALL HAVE THE SAME LOCATION, SIZE, AND SPACING AS THE VERTICAL MASONRY REINFORCING.
15. PROVIDE CLEANOUT OPENINGS FOR ALL WALLS AT THE BOTTOM OF EACH POUR IN ACCORDANCE WITH C.B.C. 2016 TITLE 24 CH. 21A AND DSA IR 21-2.13 REQUIREMENTS FOR HIGH LIFT GROUTING.
16. ANCHOR BOLTS MUST BE SET WITH TEMPLATES AND HELD IN PLACE PRIOR TO GROUTING. PROVIDE AT LEAST 1" OF GROUT BETWEEN THE HEAD OF THE ANCHOR BOLT AND THE INSIDE FACE OF MASONRY SHELL.
17. ALL ANCHOR BOLTS SHALL BE HEX-HEADED.
18. PLACEMENT OF REINFORCEMENT REQUIREMENTS:
   - THE CLEAR DISTANCE BETWEEN PARALLEL BARS SHALL NOT BE LESS THAN THE NOMINAL DIAMETER OF THE BAR, NOR LESS THAN 1".
   - IN COLUMNS AND PILASTERS, THE CLEAR DISTANCE BETWEEN VERTICAL BARS SHALL NOT BE LESS THAN 1.5 TIMES THE NOMINAL BAR DIAMETER, NOR LESS THAN 1 1/2".
   - REINFORCEMENT EMBEDDED IN GROUT SHALL HAVE A THICKNESS OF GROUT BETWEEN THE REINFORCEMENT AND MASONRY UNIT NOT LESS THAN 1/2".
19. REINFORCING BARS SHALL HAVE A MASONRY COVER NOT LESS THAN THE FOLLOWING:
   - MASONRY FACE EXPOSED TO EARTH OR WEATHER: 2" FOR #6 & GREATER BARS, 1 1/2" FOR #5 & SMALLER BARS.
   - MASONRY NOT EXPOSED TO EARTH OR WEATHER: 1 1/2" FOR ALL BAR SIZES.

11 LAP SPLICE SCHEDULE (CMU)

S1.07 NOT TO SCALE

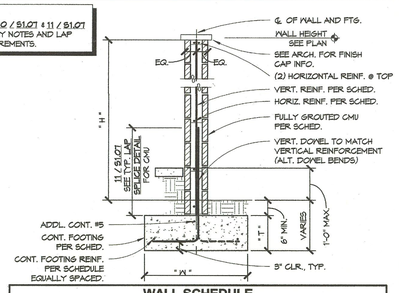
LAP LENGTH						
BAR SIZE	#3	#4	#5	#6	#7	#8
1 1/2 INCHES*	18"	36"	45"	54"	63"	72"

* MINIMUM REQUIRED COVER



NOTES:
1. THE SPLICE LENGTHS LISTED IN THE TABLE ABOVE ARE SHOWN IN INCHES AND REQUIRE THE FOLLOWING:
   - MASONRY COMPRESSIVE STRENGTH OF 2000 PSI
   - GRADE 60 STEEL FOR #4 AND LARGER BAR SIZES
   - GRADE 40 STEEL FOR #3 SIZE BAR
2. THE SPLICE LENGTHS LISTED IN THE TABLE ARE THE WORST CASE VALUE BETWEEN ALLOWABLE STRESS DESIGN AND STRENGTH DESIGN. THE ALLOWABLE STRESS DESIGN VALUES WERE DETERMINED IN ACCORDANCE WITH C.B.C. 2107A. THE STRENGTH DESIGN VALUES WERE DETERMINED IN ACCORDANCE WITH A.C.I. 530/A5.C.E. 5/TMS 402 SECTION 9.3.3.2 AND C.B.C. 2108A.
3. REINFORCEMENT LARGER THAN NO. 9 SHALL BE SPLICED USING MECHANICAL CONNECTIONS IN ACCORDANCE WITH A.C.I. 530/A5.C.E. 5/TMS 402 SECTION 2.1.10.7.3.
4. BARS SPLICED BY NON-CONTACT LAP SPLICES SHALL NOT BE SPACED FARTHER APART THAN ONE-FIFTH THE REQUIRED LENGTH OF LAP NOR MORE THAN 8".

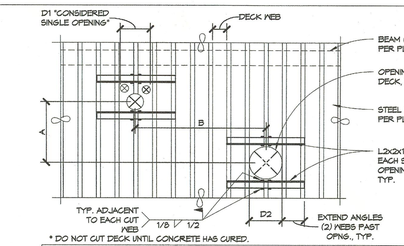
NOTE:
SEE DETAILS 10 / S1.07 & 11 / S1.07 FOR MASONRY NOTES AND LAP SPLICE REQUIREMENTS.



WALL SCHEDULE						
H	BLOCK THICKNESS	W	T	VERTICAL REINF.	HORIZ. REINF.	FOOTING REINF.
10'-0" MAX.	8"	5'-0"	1'-6"	#5 @ 8" O.C.	#5 @ 24" O.C.	(7) #5
8'-0" MAX.	8"	4'-3"	1'-6"	#5 @ 16" O.C.	#5 @ 24" O.C.	(5) #5
6'-0" MAX.	8"	3'-3"	1'-6"	#5 @ 24" O.C.	#5 @ 24" O.C.	(4) #5

12 FREE STANDING MASONRY WALLS

S1.07 NOT TO SCALE

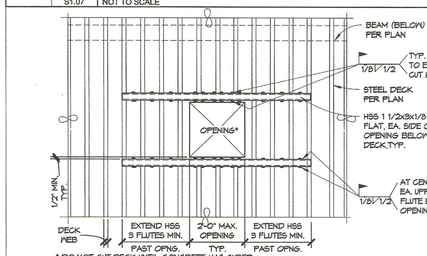


* DO NOT CUT DECK UNTIL CONCRETE HAS CURED.

NOTES:
1. OPENINGS D1 AND D2 CANNOT CUT MORE THAN TWO ADJACENT DECK WEBS.
2. HOLES LESS THAN 6" DIA. AND CUTTING NO MORE THAN ONE WEB NEED NO REINFORCING.
3. REINFORCING ANGLES MUST BE PLACED ON TOP OF STEEL DECK. WHERE ANGLES ARE PLACED BELOW DECK, PROVIDE 1/2" LONG WELD AT EACH LOW FLUTE EDGE FULL LENGTH OF ANGLE.
4. IF DIMENSION "A" IS GREATER THAN 4D1, 4D2, OR 32" WHICHEVER IS LARGER, THERE IS NO RESTRICTION ON DIMENSION "B".
5. IF DIMENSION "B" IS GREATER THAN 4D1, 4D2, OR 32" WHICHEVER IS LARGER, THERE IS NO RESTRICTION ON DIMENSION "A".
6. IF EITHER DIMENSIONS "A" OR "B" ARE LESS THAN 4D1, 4D2, OR 32" WHICHEVER IS LARGER, THE OPENING GROUP WILL BE CONSIDERED AS A SINGLE HOLE, AND MUST BE REINFORCED AS REQUIRED BY THE TYP. LARGE OPENING IN STEEL DECK DETAIL 6 / S1.07
7. OPENINGS NOT SHOWN ON PLANS SHALL BE REINFORCED BY SUBCONTRACTOR REQUIRING THE OPENINGS.

5 SMALL OPNGS. IN STL. DECK

S1.07 NOT TO SCALE



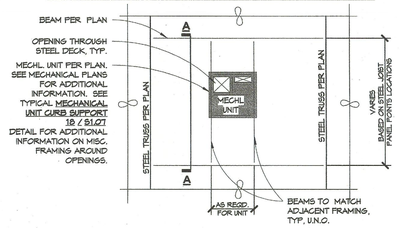
* DO NOT CUT DECK UNTIL CONCRETE HAS CURED.

NOTES:
1. HSS ARE PLACED BELOW DECK, PROVIDE 1/2" LONG WELD AT EACH LOW FLUTE EDGE FULL LENGTH OF HSS.
2. IN CONCRETE OVER STEEL DECK SITUATIONS, IF THE OPENING OR OPENING GROUP OCCURS IN ONE DECK UNIT, THE OPENING OR OPENING GROUP MAY BE CUT BEFORE POURING CONCRETE. IF THE OPENING OR OPENING GROUP CUTS THROUGH TWO DECK UNITS, THE DECK SHALL NOT BE CUT UNTIL CONCRETE HAS BEEN PLACED AND HAS CURED FOR SEVEN DAYS MIN. AT THE TIME OF POURING, SUITABLE SLEEVES OR BULKHEADS SHALL BE PLACED AROUND OPENINGS.
3. WHEN THE MAXIMUM DIMENSION OF AN OPENING OR OPENING GROUP EXCEEDS 24", THE SITUATION MUST BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR SUPPLEMENTAL SUPPORT STEEL REQUIREMENTS PRIOR TO PROCEEDING.
4. OPENINGS NOT SHOWN ON PLANS SHALL BE REINFORCED BY SUBCONTRACTOR REQUIRING THE OPENINGS.

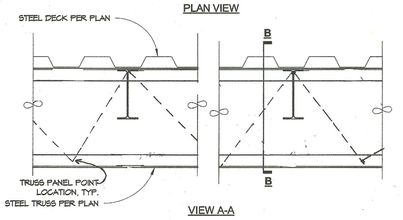
6 LARGE OPNGS. IN STL. DECK

S1.07 NOT TO SCALE

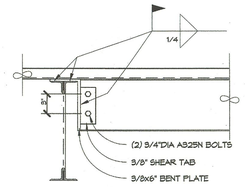
NOTES:
1. MECH'L UNIT SHAPE, SIZE AND ORIENTATION AE ONLY SHOWN GRAPHICALLY. SEE PLAN FOR SPECIFIC INFORMATION.
2. FOR CONNECTION BETWEEN WIDE FLANGE BEAMS SEE TYPICAL STEEL BEAM TO STEEL GIRDER CONNECTIONS SCHEDULE DETAIL 7 / S1.03



PLAN VIEW



VIEW A-A



VIEW B-B

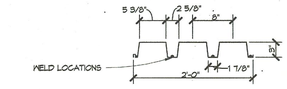
8 MECHL. UNIT SUPPORT FRAMING FOR STL. TRUSS

S1.07 NOT TO SCALE

2 STEEL ROOF DECK NOTES (PLN-24)

S1.07 NOT TO SCALE

- STEEL DECKING SHALL BE TYPE PLN-24 ACCUSTICAL AS MANUFACTURED BY VERCO MANUFACTURING COMPANY, 4340 NORTH 42ND AVENUE, PHOENIX, ARIZONA 85019. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATION AND I.C.C. REPORT NO. ESR-1788P. AN EQUIVALENT DECKING CAN BE SUBSTITUTED WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. THE EQUIVALENT DECKINGS MUST MEET OR EXCEED ALL OF THE CRITERIA STATED BELOW.
- DECKING SHALL BE FORMED FROM COLD ROLLED ZINC COATED STEEL (A.S.T.M. A 653) SHEETS WITH COATING DESIGNATION G60 (Z180) AND HAVE A MINIMUM YIELD STRENGTH OF 33,000 PSI. DECKING AT EXPOSED EXTERIOR APPLICATIONS SHALL HAVE ZINC COATING DESIGNATION G90 (Z275).
- THE DECK PROFILE AND DIMENSION SHALL BE AS FOLLOWS:



- DECK GAGE, WEIGHT AND PROPERTIES SHALL BE AS SELECTED BELOW:

GAGE	WEIGHT (PSF)	I (in⁴)	+S (in³)	-S (in³)	SELECTED DECK
22	2.2	0.788	0.384	0.429	
20	2.6	0.906	0.448	0.531	
18	3.5	1.267	0.652	0.739	✓
16	4.2	1.642	0.837	0.914	

- DECK ATTACHMENT TO SUPPORTS PERPENDICULAR TO RIBS SHALL BE WITH 1/2" DIA. EFFECTIVE ARC SPOT (PUDDLE) WELDS WITH A PATTERN AS SELECTED BELOW:

PATTERN DESIGNATION	PATTERN DIAGRAM	SELECTED PATTERN
24/4		✓

- TYPE AND SPACING OF DECK ATTACHMENT TO SUPPORTS PARALLEL WITH DECK RIBS SHALL BE AS SELECTED BELOW:

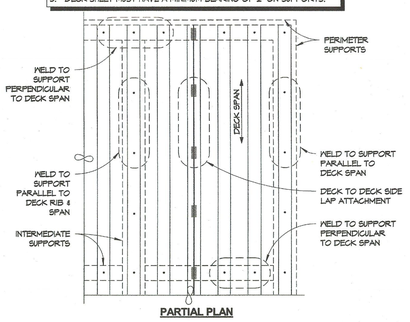
ATTACHMENT TYPE	ATTACHMENT SPACING	SELECTED ATTACHMENT
1/2" DIA. EFF. ARC SPOT WELD	24" O.C.	✓
1/2" DIA. EFF. ARC SPOT WELD	18" O.C.	
1/2" DIA. EFF. ARC SPOT WELD	12" O.C.	

- TYPE AND SPACING OF DECK ATTACHMENT AT SIDE LAPS SHALL BE AS SELECTED BELOW:

ATTACHMENT TYPE	ATTACHMENT SPACING	SELECTED ATTACHMENT
VERCO SIDELAP CONN. (VSC)	24" O.C.	✓
VERCO SIDELAP CONN. (VSC)	18" O.C.	
VERCO SIDELAP CONN. (VSC)	12" O.C.	
VERCO SIDELAP CONN. (VSC)	8" O.C.	
VERCO SIDELAP CONN. (VSC)	6" O.C.	
VERCO SIDELAP CONN. (VSC)	4" O.C.	

- SEE DECK ATTACHMENT LAYOUT DETAIL 3 / S1.07 FOR ADDITIONAL INFORMATION.
- ALL OPENINGS IN DECK SHALL BE REINFORCED AS SHOWN IN THE SMALL/LARGE OPENINGS IN STEEL DECK 5 / S1.07 & 6 / S1.07 DETAIL.
- NO MECHANICAL EQUIPMENT, DUCTS OR PIPING SHALL BE SUPPORTED DIRECTLY FROM DECK. ALL PIPING AND EQUIPMENT SHALL BE SUPPORTED FROM STRUCTURAL FRAMING MEMBERS. SEE S.M.A.C.N.A. FOR SEISMIC BRACING REQUIREMENTS FOR PIPING AND DUCTWORK.
- DECK SHALL BE SUPPORTED BY A MINIMUM OF (4) MEMBERS, (3-SPAN MINIMUM)

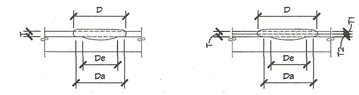
NOTES:
1. MINIMUM LENGTH OF DECK SHEETS SHALL BE TWO-SPANS.
2. ENDS OF DECK SHEETS MUST BE LAPPED A MINIMUM OF 2", AND LAPS MUST OCCUR OVER SUPPORTS.
3. DECK SHEET MUST HAVE A MINIMUM BEARING OF 2" ON SUPPORTS.



PARTIAL PLAN

3 DECK ATTACHMENT LAYOUT

S1.07 NOT TO SCALE



NOTE:
Da = (D - T)
De = (0.7D - 1.5T)

SINGLE THICKNESS OF SHEET

NOTE:
Da = (D - 2T)
De = (0.7D - 1.5T)

DOUBLE THICKNESS OF SHEET

GAGE	VISIBLE WELD SIZE
(0.0269") 22	3/4" DIA. (0.774)
(0.0346") 20	3/4" DIA. (0.786)
(0.0451") 18	7/8" DIA. (0.811)
(0.0566") 16	7/8" DIA. (0.836)

NOTE:
BASED ON 1/2" EFFECTIVE WELD
ARC SPOT WELDS PER A.W.S. D1.3/ D1.3M
D = (De + 1.5T)/0.7 VISIBLE DIAMETER
Da = RESULTING AVERAGE DIAMETER
De = EFFECTIVE DIAMETER

4 ARC SPOT WELDS PER AWS D1.3/D1.3M

S1.07 NOT TO SCALE

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Phone: (559) 436-0881 Fax: (559) 436-0887 E-Mail: design@somam.com
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Revision Description:	

Sheet Title: TYPICAL METAL DECK & CMU DETAILS

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
Date:
Designer: Designer
DR:
PC:

Agency Approval Stamp:

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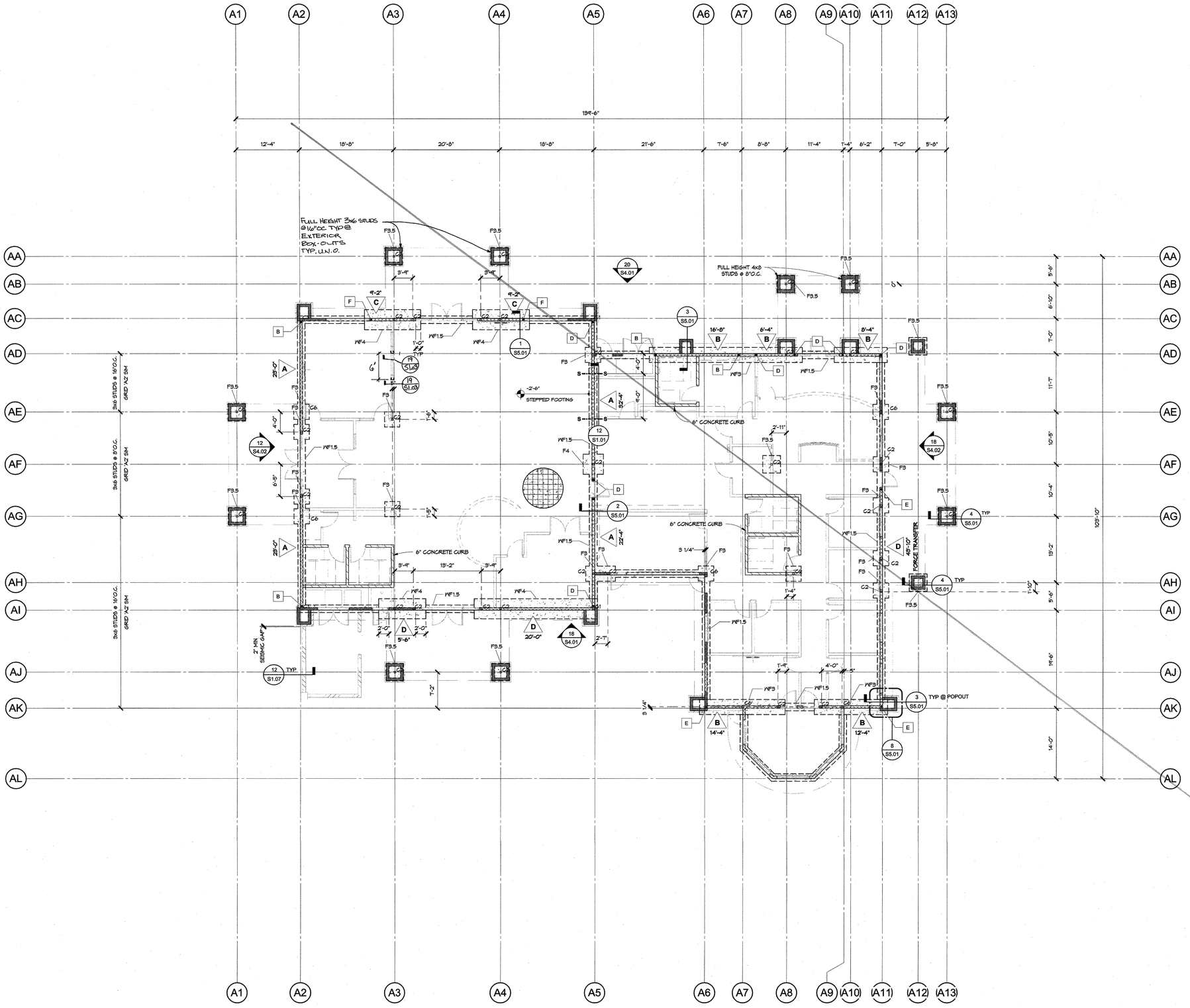
STATE OF CALIFORNIA

BROOKE-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
TOM BROOKE-RANSOM
7415 N. PALM AVE., SUITE 120
FRESNO, CALIFORNIA 93711

Job No.:

Sheet No.: S1.07

Release:



20	BUIDING A - FOUNDATION PLAN
S4.01 S2.01	1/8" = 1'-0"

1. SEE TYPICAL SHEETS FOR ALL GENERAL AND MATERIAL NOTES, AND ALL TYPICAL SCHEDULES AND DETAILS. THE INFORMATION ON THE TYPICAL SHEETS APPLY TO THE PROJECT AND ARE NOT SPECIFICALLY REFERENCED ON PLAN WORK, UNLESS NOTED OTHERWISE. IF TYPICAL DETAILS ARE SPECIFIED ON PLANS OR NOTES, THEY WILL BE REFERENCED WITH THE WORD "TYPICAL" FOLLOWED BY BOLD AND UNDERLINED TEXT STATING THE TITLE OF THE TYPICAL DETAIL OR NOTE.
2. ALL DIMENSIONS SHOWN ARE TO FACE OF STUDS AT EXTERIOR WALLS, CENTER OF COLUMN OR CENTER OF INTERIOR WALLS UNLESS NOTED OTHERWISE. EDGE OF SLAB AT PERIMETER OF BUILDING SHALL BE INDICATED IN SPECIFIC DETAILS AND ARCHITECTURAL. VERIFY ALL BUILDING DIMENSIONS WITH ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. NOTIFY THE ARCHITECT IMMEDIATELY IF THERE ARE ANY CONFLICTS WITH THE DIMENSIONS SHOWN.
3. ALL UNCLEAR AND/OR MISSING DETAILS SHALL BE BROUGHT TO THE STRUCTURAL ENGINEER'S ATTENTION BEFORE PROCEEDING WITH CONSTRUCTION.
4. ALL PAD AND CONTINUOUS FOOTINGS ARE TO BE CENTERED ON WALLS AND COLUMNS ABOVE UNLESS NOTED OTHERWISE.
5. SEE CIVIL AND/OR ARCHITECTURAL SITE PLAN FOR LOCATION AND DIMENSIONS OF SIDEWALKS, MOW STRIPS, PLANTERS AND OTHER LANDSCAPING FEATURES.
6. SEE ARCHITECTURAL AND PLUMBING PLANS FOR LOCATION OF ALL PIPING AND DRAINS. SEE TYPICAL DETAILS FOR STRUCTURAL REQUIREMENTS AT LOCATIONS WHERE PIPES INTERSECT OR ALIGN NEXT TO FOOTINGS AND SLABS.
7. ALL EMBEDDED ITEMS SHALL BE IN PLACE AND SECURED PRIOR TO POURING OF CONCRETE.
8. ALL SILL PLATE ANCHOR BOLTS SHALL BE SIZED, SPACED, PLACED, AND HAVE THE WASHER AND MATERIAL FINISH AS SPECIFIED IN THE WOOD NOTES. 3 / S1.04
9. CONTRACTOR SHALL SUBMIT CONTROL JOINT PLAN FOR APPROVAL BY A.O.R. AND S.E.O.R. PRIOR TO POURING SLAB. SEE TYPICAL CONTROL JOINT 2 / S1.02 DETAIL FOR ADDITIONAL INFORMATION.
10. ALL EXTERIOR WALLS SHALL HAVE 15/32" SHEATHING WITH 10d @ 6"O.C. E.N. AND 10d @ 12"O.C. F.N., UNLESS NOTED OTHERWISE. SEE WOOD NOTES 3 / S1.04 FOR SHEATHING MATERIAL.
11. ALL TOP OF FOOTINGS SHALL BE 1'-0" BELOW FINISH SLAB, UNLESS NOTED OTHERWISE.
12. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1	WOOD FOUNDATION NOTES
S2.01	NOT TO SCALE

BEARING WALL. ALL BEARING, EXTERIOR AND SHEAR WALLS SHALL BE 2x6 STUDS @ 16"O.C., U.N.O. SEE TYPICAL WALL FRAMING AT OPENINGS FOR WOOD FRAMING DETAIL 18 / S1.04

NON-BEARING WALL. PROVIDE 2x STUDS @ 16"O.C. SEE ARCHITECTURAL PLANS FOR THICKNESS OF STUDS REQUIRED. SEE TYPICAL NON-BEARING WALL 8 / S1.03 DETAIL FOR TOP OF WALL CONNECTION.

SHEAR WALL. X-X" INDICATES THE MINIMUM SHEAR WALL LENGTH. IF THE LENGTH IS NOT NOTED, THEN THE LENGTH IS EQUAL TO THE TOTAL WALL LENGTH. SEE TYPICAL SHEAR WALL SCHEDULE 4 / S1.04 AND BEARING WALL NOTE ABOVE.

HOLDOWN. HOLDOWNS MAY REQUIRE LARGER FOOTINGS NOT SHOWN. SEE TYPICAL HOLDOWN SCHEDULE 14 / S1.04

"XX" AS INDICATED ON PLAN. SEE TYPICAL SCHEDULES AS FOLLOWS:
C# - COLUMN SCHEDULE
P# - PILASTER SCHEDULE
W# - WALL SCHEDULE
F# - FOUNDATION SCHEDULE

F.F. +X'-X" FINISHED FLOOR (F.F.) OR TOP OF FRAMING (T.O.F.). +X'-X" IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

DEPRESSED SLAB. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.

CONCRETE SLAB. PROVIDE 4" THICK MINIMUM SLAB WITH #4 @ 18" O.C. EACH WAY, U.N.O. SEE TYPICAL FOUNDATION AND CONCRETE NOTES 3 / S1.02 AND ARCHITECTURAL DRAWINGS FOR SUB-GRADE AND VAPOR BARRIER INFORMATION.

3	WOOD LEGEND
S2.01	NOT TO SCALE

STRUCTURAL COLUMN SCHEDULE

MARK	TYPE	COMMENTS
C1	6x6	
C2	HSS4X4X1/4	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C3	W6X25	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C4	HSS6X5X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C5	HSS6X6X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C6	HSS8X5X1/2	SEE TYPICAL COLUMN BASE PLATE SCHEDULE

STRUCTURAL FOUNDATION SCHEDULE

MARK	TYPE	REINFORCEMENT
F2	2'-0"SQUARE x 1'-6"DEEP	(2)#5 T&B EACH WAY
F2.5	2'-6"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3.5	3'-6"SQUARE x 1'-6"DEEP	(4)#5 T&B EACH WAY
F4	4'-0"SQUARE x 1'-6"DEEP	(5)#5 T&B EACH WAY
WF1	1'-0"Wx1'-6"D CONTN. FOOTING	(1)#5 T&B LONGITUDINAL BARS
WF1.5	1'-6"WIDE x 1'-6"DEEP CONTN. FTG.	(2)#5 T&B LONGITUDINAL BARS
WF2	2'-0"Wx1'-6"D Contin. Footing	(3)#5 T&B LONGITUDINAL BARS; #5 @ 12'O.C. T&B TRANSVERSE
WF3	3'-0"Wx2'-0"D Contin. Footing	(4)#5 T&B LONGITUDINAL BARS; #5 @ 12'O.C. TIES TRANSVERSE
WF4	4'-0"Wx2'-0"D Contin. Footing	(5)#5 T&B LONGITUDINAL BARS; #5 @ 12'O.C. TIES TRANSVERSE

4	STRUCTURAL SCHEDULES
	NOT TO SCALE

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6011 N. Fresno, Suite 130 - Fresno, California 93710
Phone (559) 436-0881 Fax (559) 436-0887 E-Mail: design@somam.com
www.integratedesigns.com

Revision:	Revision Description:	Rev. Date:
Revision:	Revision Description:	Rev. Date:

Sheet Title: A FOUNDATION PLAN

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
Date:
Designer: Designer
DR:
PC:

Agency Approval Stamp:

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DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC FLS SS V-1
DATE 06-22-18

TRACKING #: 63321-300

Stamp(s)

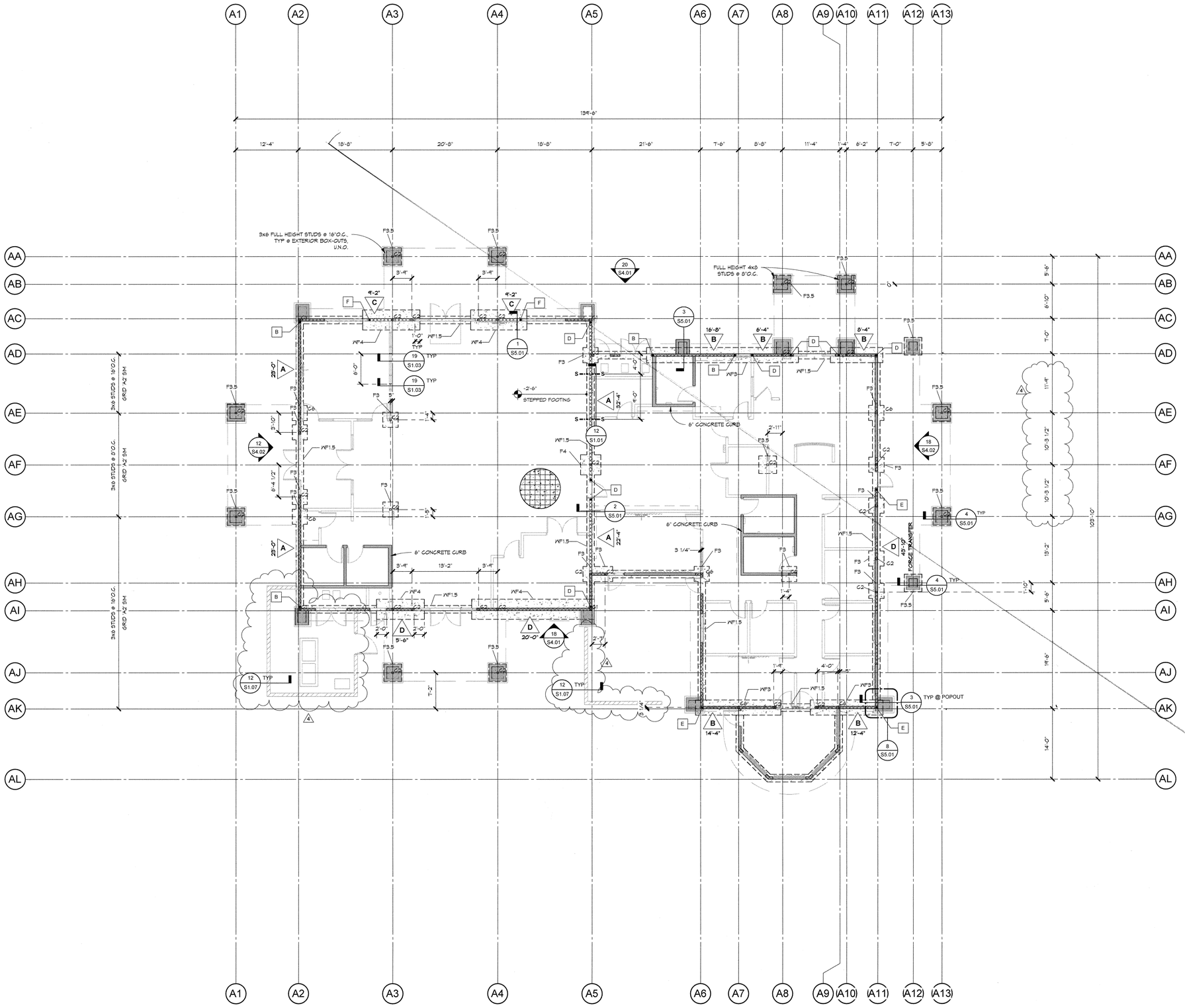
REGISTERED PROFESSIONAL ENGINEER
CHRISTOPHER R. RAMOS
No. S24
EXPIRES
03-31-20
STRUCTURAL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 103
FRESNO, CALIFORNIA 93711
17060

Job No.:

Sheet No.: S2.01

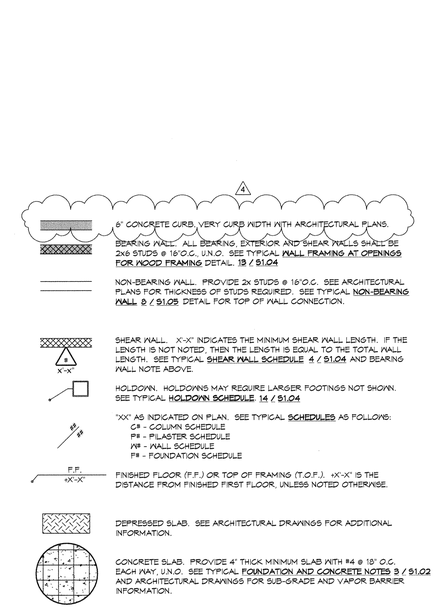
Release:



20	BUIDING A - FOUNDATION PLAN
S4.01 S2.01	1/8" = 1'-0"

1. SEE TYPICAL SHEETS FOR ALL GENERAL AND MATERIAL NOTES, AND ALL TYPICAL SCHEDULES AND DETAILS. THE INFORMATION ON THE TYPICAL SHEETS APPLY TO THE PROJECT AND ARE NOT SPECIFICALLY REFERENCED ON PLAN WORK, UNLESS NOTED OTHERWISE. IF TYPICAL DETAILS ARE SPECIFIED ON PLANS OR NOTES, THEY WILL BE REFERENCED WITH THE WORD "TYPICAL" FOLLOWED BY **BOLD AND UNDERLINED** TEXT STATING THE TITLE OF THE TYPICAL DETAIL OR NOTE.
2. ALL DIMENSIONS SHOWN ARE TO FACE OF STUDS AT EXTERIOR WALLS, CENTER OF COLUMN OR CENTER OF INTERIOR WALLS UNLESS NOTED OTHERWISE. EDGE OF SLAB AT PERIMETER OF BUILDING SHALL BE INDICATED IN SPECIFIC DETAILS AND ARCHITECTURAL. VERIFY ALL BUILDING DIMENSIONS WITH ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. NOTIFY THE ARCHITECT IMMEDIATELY IF THERE ARE ANY CONFLICTS WITH THE DIMENSIONS SHOWN.
3. ALL UNCLEAR AND/OR MISSING DETAILS SHALL BE BROUGHT TO THE STRUCTURAL ENGINEER'S ATTENTION BEFORE PROCEEDING WITH CONSTRUCTION.
4. ALL PAD AND CONTINUOUS FOOTINGS ARE TO BE CENTERED ON WALLS AND COLUMNS ABOVE UNLESS NOTED OTHERWISE.
5. SEE CIVIL AND/OR ARCHITECTURAL SITE PLAN FOR LOCATION AND DIMENSIONS OF SIDEWALKS, MOW STRIPS, PLANTERS AND OTHER LANDSCAPING FEATURES.
6. SEE ARCHITECTURAL AND PLUMBING PLANS FOR LOCATION OF ALL PIPING AND DRAINS. SEE TYPICAL DETAILS FOR STRUCTURAL REQUIREMENTS AT LOCATIONS WHERE PIPES INTERSECT OR ALIGN NEXT TO FOOTINGS AND SLABS.
7. ALL EMBEDDED ITEMS SHALL BE IN PLACE AND SECURED PRIOR TO POURING OF CONCRETE.
8. ALL SILL PLATE ANCHOR BOLTS SHALL BE SIZED, SPACED, PLACED, AND HAVE THE WASHER AND MATERIAL FINISH AS SPECIFIED IN THE **WOOD NOTES**, 3 / S1.04
9. CONTRACTOR SHALL SUBMIT CONTROL JOINT PLAN FOR APPROVAL BY A.O.R. AND S.E.O.R. PRIOR TO POURING SLAB. SEE TYPICAL **CONTROL JOINT** 6 / S1.02 DETAIL FOR ADDITIONAL INFORMATION.
10. ALL EXTERIOR WALLS SHALL HAVE 15/32" SHEATHING WITH 10d @ 6"O.C. E.N. AND 10d @ 12"O.C. F.N., UNLESS NOTED OTHERWISE. SEE **WOOD NOTES** 3 / S1.04 FOR SHEATHING MATERIAL.
11. ALL TOP OF FOOTINGS SHALL BE 1'-0" BELOW FINISH SLAB, UNLESS NOTED OTHERWISE.
12. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1	WOOD FOUNDATION NOTES
S2.01	NOT TO SCALE



3	WOOD LEGEND
S2.01	NOT TO SCALE

STRUCTURAL COLUMN SCHEDULE		
MARK	TYPE	COMMENTS
C1	6x6	
C2	HSS4X4X1/4	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C3	W6X25	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C4	HSS6X5X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C5	HSS6X6X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C6	HSS5X5X1/2	SEE TYPICAL COLUMN BASE PLATE SCHEDULE

STRUCTURAL FOUNDATION SCHEDULE		
MARK	TYPE	REINFORCEMENT
F2	2'-0"SQUARE x 1'-6"DEEP	(2)#5 T&B EACH WAY
F2.5	2'-6"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3.5	3'-6"SQUARE x 1'-6"DEEP	(4)#5 T&B EACH WAY
F4	4'-0"SQUARE x 1'-6"DEEP	(5)#5 T&B EACH WAY
WF1	1'-0"Wx1'-6"D CONTIN. FOOTING	(1)#5 T&B LONGITUDINAL BARS
WF1.5	1'-6"WIDE x 1'-6"DEEP CONTIN. FTG.	(2)#5 T&B LONGITUDINAL BARS
WF2	2'-0"Wx1'-6"D Contin. Footing	(3)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. T&B TRANSVERSE
WF3	3'-0"Wx2'-0"D Contin. Footing	(4)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. TIES TRANSVERSE
WF4	4'-0"Wx2'-0"D Contin. Footing	(5)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. TIES TRANSVERSE

4	STRUCTURAL SCHEDULES
	NOT TO SCALE

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Phone (559) 439-0901 Fax (559) 439-0905 E-Mail: design@somam.com
www.integrateddesigns.com

Revision:	Revision Description:	Rev. Date:
1	ADDENDUM #2	08/16/18
2	ADDENDUM #4	09-07-18

Sheet Title: A FOUNDATION PLAN

Project Name & Address: NEW E.S. INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 9-07-18
Drawn: Designer
Date:
DR:
PIC:

Agency Approval Stamp:
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DIV. OF THE STATE ARCHITECT
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03-118394
AC_____FLS_____SS_____
DATE_____
TRACKING #: 63321-300

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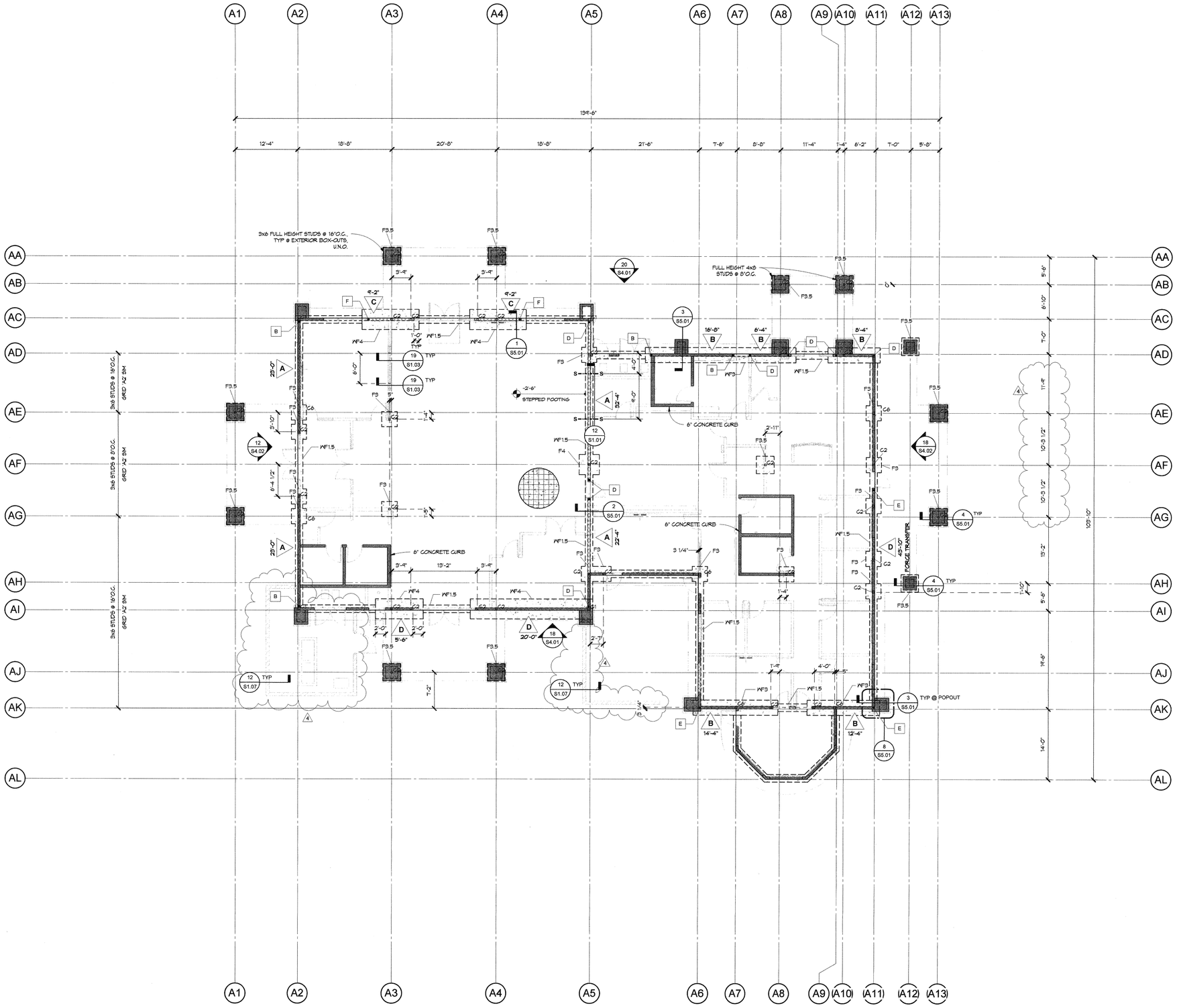
CLIFFORD R. RANSOM No.S2386 EXPIRES 12/31/19 REGISTERED PROFESSIONAL ENGINEER CIVIL STATE OF CALIFORNIA
BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711

Job No.:

Sheet No.: S2.01

Release: ADDENDUM NO. 4



20	BUIDING A - FOUNDATION PLAN
S4.01 S2.01	1/8" = 1'-0"

1. SEE TYPICAL SHEETS FOR ALL GENERAL AND MATERIAL NOTES, AND ALL TYPICAL SCHEDULES AND DETAILS. THE INFORMATION ON THE TYPICAL SHEETS APPLY TO THE PROJECT AND ARE NOT SPECIFICALLY REFERENCED ON PLAN WORK, UNLESS NOTED OTHERWISE. IF TYPICAL DETAILS ARE SPECIFIED ON PLANS OR NOTES, THEY WILL BE REFERENCED WITH THE WORD "TYPICAL" FOLLOWED BY BOLD AND UNDERLINED TEXT STATING THE TITLE OF THE TYPICAL DETAIL OR NOTE.
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4. ALL PAD AND CONTINUOUS FOOTINGS ARE TO BE CENTERED ON WALLS AND COLUMNS ABOVE UNLESS NOTED OTHERWISE.
5. SEE CIVIL AND/OR ARCHITECTURAL SITE PLAN FOR LOCATION AND DIMENSIONS OF SIDEWALKS, MOW STRIPS, PLANTERS AND OTHER LANDSCAPING FEATURES.
6. SEE ARCHITECTURAL AND PLUMBING PLANS FOR LOCATION OF ALL PIPING AND DRAINS. SEE TYPICAL DETAILS FOR STRUCTURAL REQUIREMENTS AT LOCATIONS WHERE PIPES INTERSECT OR ALIGN NEXT TO FOOTINGS AND SLABS.
7. ALL EMBEDDED ITEMS SHALL BE IN PLACE AND SECURED PRIOR TO POURING OF CONCRETE.
8. ALL SILL PLATE ANCHOR BOLTS SHALL BE SIZED, SPACED, PLACED, AND HAVE THE WASHER AND MATERIAL FINISH AS SPECIFIED IN THE WOOD NOTES, 3 / S1.04.
9. CONTRACTOR SHALL SUBMIT CONTROL JOINT PLAN FOR APPROVAL BY A.O.R. AND S.E.O.R. PRIOR TO POURING SLAB. SEE TYPICAL CONTROL JOINT 8 / S1.02 DETAIL FOR ADDITIONAL INFORMATION.
10. ALL EXTERIOR WALLS SHALL HAVE 15/32" SHEATHING WITH 10d @ 6"O.C. E.N. AND 10d @ 12"O.C. F.N., UNLESS NOTED OTHERWISE. SEE WOOD NOTES 3 / S1.04 FOR SHEATHING MATERIAL.
11. ALL TOP OF FOOTINGS SHALL BE 1'-0" BELOW FINISH SLAB, UNLESS NOTED OTHERWISE.
12. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1	WOOD FOUNDATION NOTES
S2.01	NOT TO SCALE

6" CONCRETE CURB, VERY CURB WIDTH WITH ARCHITECTURAL PLANS.

BEARING WALL. ALL BEARING, EXTERIOR AND SHEAR WALLS SHALL BE 2x6 STUDS @ 16"O.C., U.N.O. SEE TYPICAL WALL FRAMING AT OPENINGS FOR WOOD FRAMING DETAIL, 13 / S1.04

NON-BEARING WALL. PROVIDE 2X STUDS @ 16"O.C. SEE ARCHITECTURAL PLANS FOR THICKNESS OF STUDS REQUIRED. SEE TYPICAL NON-BEARING WALL 8 / S1.05 DETAIL FOR TOP OF WALL CONNECTION.

SHEAR WALL. X-X' INDICATES THE MINIMUM SHEAR WALL LENGTH. IF THE LENGTH IS NOT NOTED, THEN THE LENGTH IS EQUAL TO THE TOTAL WALL LENGTH. SEE TYPICAL SHEAR WALL SCHEDULE 4 / S1.04 AND BEARING WALL NOTE ABOVE.

HOLDOWN. HOLDOWNS MAY REQUIRE LARGER FOOTINGS NOT SHOWN. SEE TYPICAL HOLDOWN SCHEDULE 14 / S1.04

"XX" AS INDICATED ON PLAN. SEE TYPICAL SCHEDULES AS FOLLOWS:
C# - COLUMN SCHEDULE
P# - PILASTER SCHEDULE
W# - WALL SCHEDULE
F# - FOUNDATION SCHEDULE

F.F. +X-X' FINISHED FLOOR (F.F.) OR TOP OF FRAMING (T.O.F.). +X-X' IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

DEPRESSED SLAB. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.

CONCRETE SLAB. PROVIDE 4" THICK MINIMUM SLAB WITH #4 @ 18" O.C. EACH WAY, U.N.O. SEE TYPICAL FOUNDATION AND CONCRETE NOTES 3 / S1.02 AND ARCHITECTURAL DRAWINGS FOR SUB-GRADE AND VAPOR BARRIER INFORMATION.

3	WOOD LEGEND
S2.01	NOT TO SCALE

STRUCTURAL COLUMN SCHEDULE

MARK	TYPE	COMMENTS
C1	6x6	
C2	HSS4X4X1/4	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C3	W6X25	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C4	HSS6X5X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C5	HSS6X6X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C6	HSS5X5X1/2	SEE TYPICAL COLUMN BASE PLATE SCHEDULE

STRUCTURAL FOUNDATION SCHEDULE

MARK	TYPE	REINFORCEMENT
F2	2'-0"SQUARE x 1'-6"DEEP	(2)#5 T&B EACH WAY
F2.5	2'-6"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3.5	3'-6"SQUARE x 1'-6"DEEP	(4)#5 T&B EACH WAY
F4	4'-0"SQUARE x 1'-6"DEEP	(5)#5 T&B EACH WAY
WF1	1'-0"WX1'-6"D CONTIN. FOOTING	(1)#5 T&B LONGITUDINAL BARS
WF1.5	1'-6"WIDE x 1'-6"DEEP CONTIN. FTG.	(2)#5 T&B LONGITUDINAL BARS
WF2	2'-0"WX1'-6"D Contin. Footing	(3)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. T&B TRANSVERSE
WF3	3'-0"WX2'-0"D Contin. Footing	(4)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. TIES TRANSVERSE
WF4	4'-0"WX2'-0"D Contin. Footing	(5)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. TIES TRANSVERSE

4	STRUCTURAL SCHEDULES
	NOT TO SCALE

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www.integratedesigns.com

Revision:	Revision Description:	Rev. Date:
1	ADDENDUM #2	08/16/18
2	ADDENDUM #4	09-07-18

Sheet Title: A FOUNDATION PLAN

Project Name & Address: NEW E.S. INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 9-07-18
Dates:
Designer: Designer
DR:
PC:

Agency Approval Stamp:

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DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC SC FLS SH SS TN
DATE 5/16/19

TRACKING #: 63321-300

ADDENDUM #4

Stamp(s)

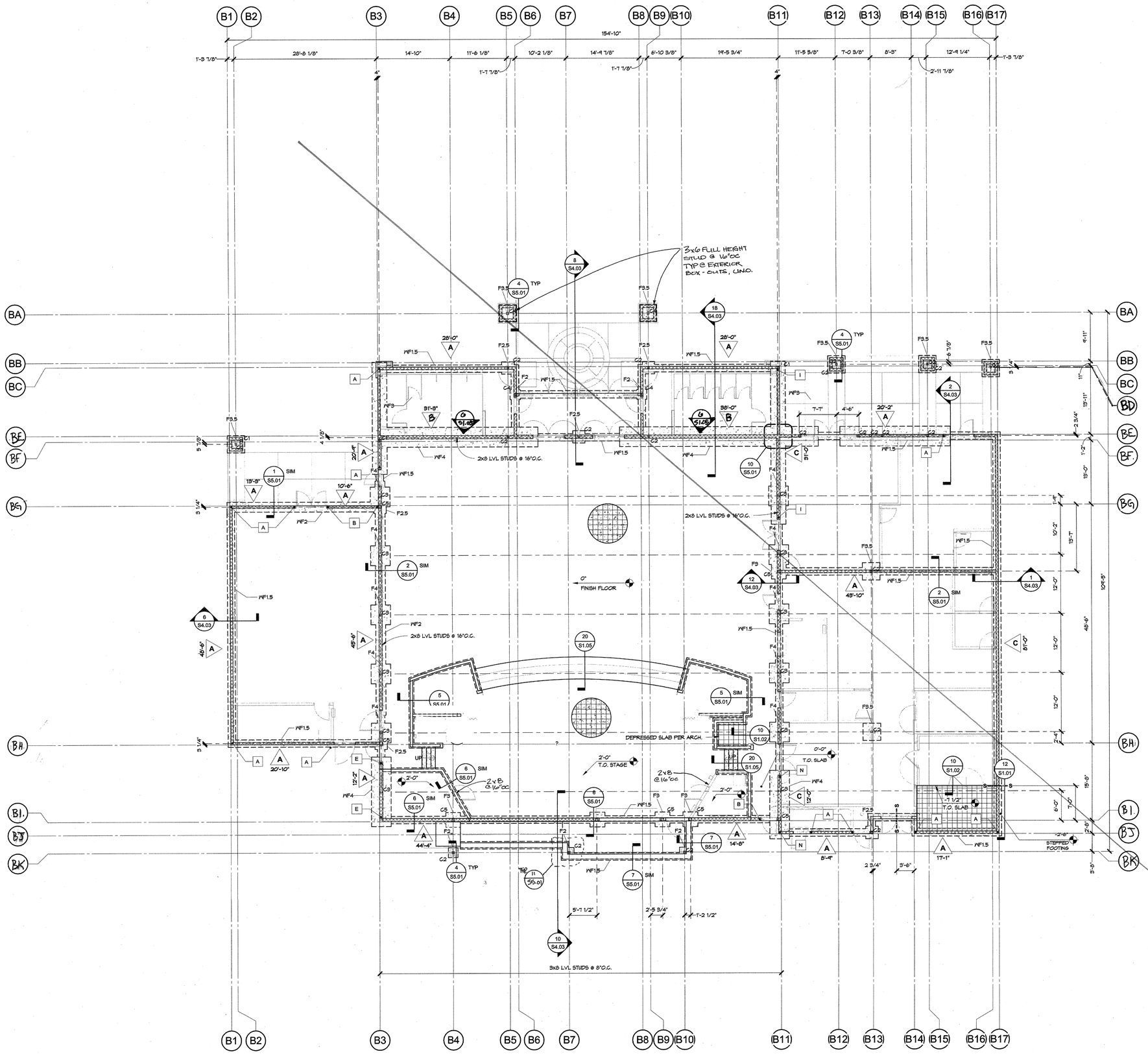
REGISTERED PROFESSIONAL ENGINEER
GAYLORD R RANSOM
No. S2386
EXPIRES 12-31-20
CIVIL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711
17000

Job No.:

Sheet No.:
S2.01

Release: ADDENDUM NO. 4



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8. ALL SILL PLATE ANCHOR BOLTS SHALL BE SIZED, SPACED, PLACED, AND HAVE THE WASHER AND MATERIAL FINISH AS SPECIFIED IN THE **WOOD NOTES**, 3 / S1.04
9. CONTRACTOR SHALL SUBMIT CONTROL JOINT PLAN FOR APPROVAL BY A.O.R. AND S.E.O.R. PRIOR TO POURING SLAB. SEE TYPICAL **CONTROL JOINT**, 8 / S1.02 DETAIL FOR ADDITIONAL INFORMATION.
10. ALL EXTERIOR WALLS SHALL HAVE 15/32" SHEATHING WITH 10d @ 6"O.C. E.N. AND 10d @ 12"O.C. F.N., UNLESS NOTED OTHERWISE. SEE **WOOD NOTES** 3 / S1.04 FOR SHEATHING MATERIAL.
11. ALL TOP OF FOOTINGS SHALL BE 1'-0" BELOW FINISH SLAB, UNLESS NOTED OTHERWISE.
12. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1	WOOD FOUNDATION NOTES
S2.02	NOT TO SCALE

BEARING WALL. ALL BEARING, EXTERIOR AND SHEAR WALLS SHALL BE 2x6 STUDS @ 16"O.C., U.N.O. SEE TYPICAL **WALL FRAMING AT OPENINGS FOR WOOD FRAMING** DETAIL/ S1.04

NON-BEARING WALL. PROVIDE 2x STUDS @ 16"O.C. SEE ARCHITECTURAL PLANS FOR THICKNESS OF STUDS REQUIRED. SEE TYPICAL **NON-BEARING WALL**, 8 / S1.05 DETAIL FOR TOP OF WALL CONNECTION.

SHEAR WALL. X-X" INDICATES THE MINIMUM SHEAR WALL LENGTH. IF THE LENGTH IS NOT NOTED, THEN THE LENGTH IS EQUAL TO THE TOTAL WALL LENGTH. SEE TYPICAL **SHEAR WALL SCHEDULE** 4 / S1.04 AND BEARING WALL NOTE ABOVE.

HOLDOWN. HOLDOWNS MAY REQUIRE LARGER FOOTINGS NOT SHOWN. SEE TYPICAL **HOLDOWN SCHEDULE** 14 / S1.04

"XX" AS INDICATED ON PLAN. SEE TYPICAL **SCHEDULES** AS FOLLOWS:
C# - COLUMN SCHEDULE
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W# - WALL SCHEDULE
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FINISHED FLOOR (F.F.) OR TOP OF FRAMING (T.O.F.). "X'-X" IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

DEPRESSED SLAB. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.

CONCRETE SLAB. PROVIDE 4" THICK MINIMUM SLAB WITH #4 @ 18" O.C. EACH WAY, U.N.O. SEE TYPICAL **FOUNDATION AND CONCRETE NOTES** 3 / S1.02 AND ARCHITECTURAL DRAWINGS FOR SUB-GRADE AND VAPOR BARRIER INFORMATION.

2	WOOD LEGEND
S2.02	NOT TO SCALE

STRUCTURAL COLUMN SCHEDULE

MARK	TYPE	COMMENTS
C1	6X6	
C2	HSS4X4X1/4	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C3	W6X25	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C4	HSS6X5X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C5	HSS6X6X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C6	HSS5X5X1/2	SEE TYPICAL COLUMN BASE PLATE SCHEDULE

STRUCTURAL FOUNDATION SCHEDULE

MARK	TYPE	REINFORCEMENT
F2	2'-0"SQUARE x 1'-6"DEEP	(2)#5 T&B EACH WAY
F2.5	2'-6"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
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F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3.5	3'-6"SQUARE x 1'-6"DEEP	(4)#5 T&B EACH WAY
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4	STRUCTURAL SCHEDULES
	NOT TO SCALE

20	BUILDING B - FOUNDATION PLAN
S4.01 S2.02	1/8" = 1'-0"

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www.integrateddesigns.com

Revision Description: Rev. Date:
Revision: Revision: Rev. Date:
Revision Description: Rev. Date:

Sheet Title: B FOUNDATION PLAN

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
Drafter: Designer: SOC DR: FC:

Agency Approval Stamp:

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DIV. OF THE STATE ARCHITECT
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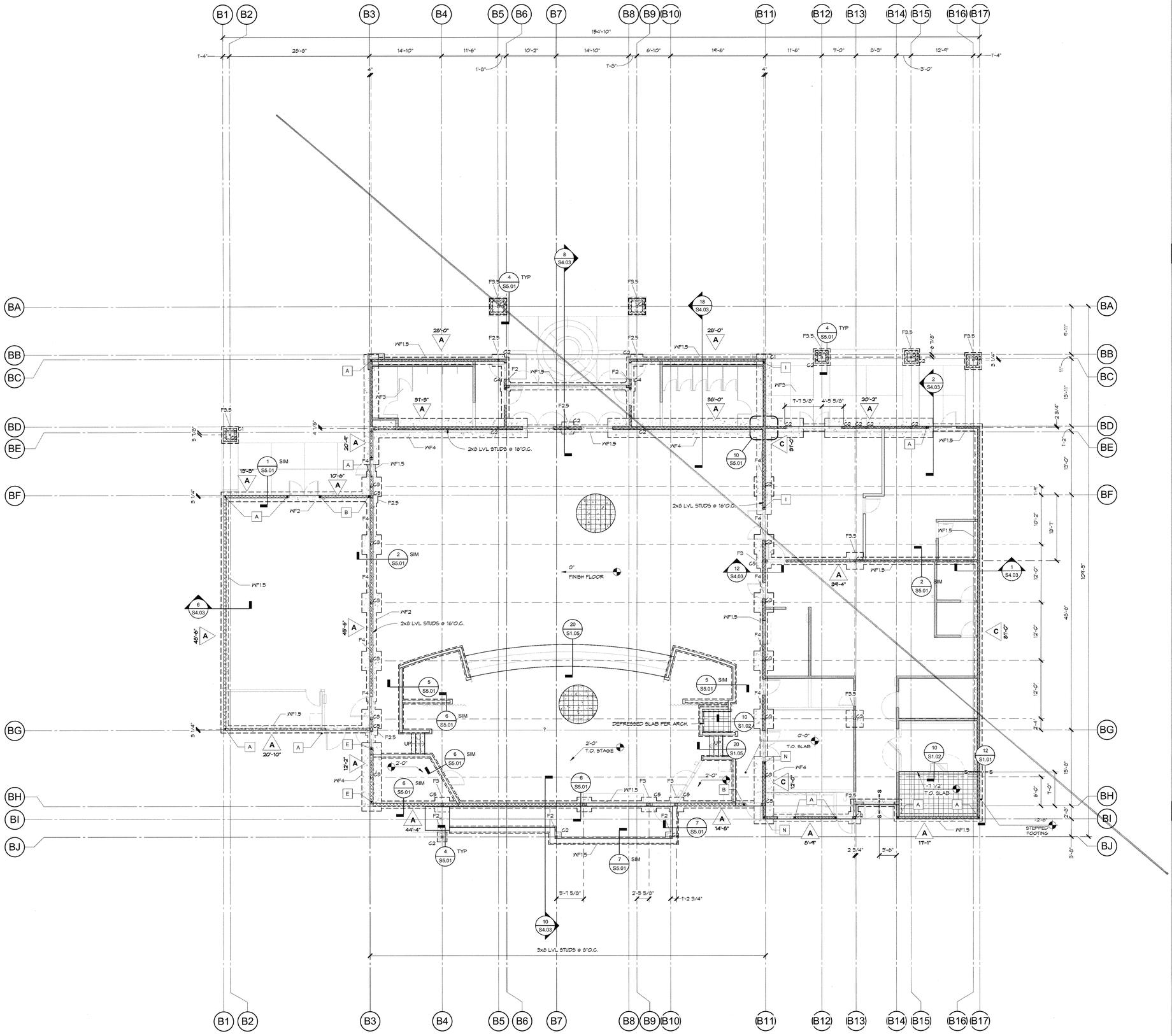
REGISTERED PROFESSIONAL ENGINEER CALIFORNIA No. S2 EXPIRES 31-20 STRUCTURAL STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711

Job No.:

Sheet No.: S2.02

Release:



1. SEE TYPICAL SHEETS FOR ALL GENERAL AND MATERIAL NOTES, AND ALL TYPICAL SCHEDULES AND DETAILS. THE INFORMATION ON THE TYPICAL SHEETS APPLY TO THE PROJECT AND ARE NOT SPECIFICALLY REFERENCED ON PLAN WORK, UNLESS NOTED OTHERWISE. IF TYPICAL DETAILS ARE SPECIFIED ON PLANS OR NOTES, THEY WILL BE REFERENCED WITH THE WORD "TYPICAL" FOLLOWED BY **BOLD AND UNDERLINED** TEXT STATING THE TITLE OF THE TYPICAL DETAIL OR NOTE.
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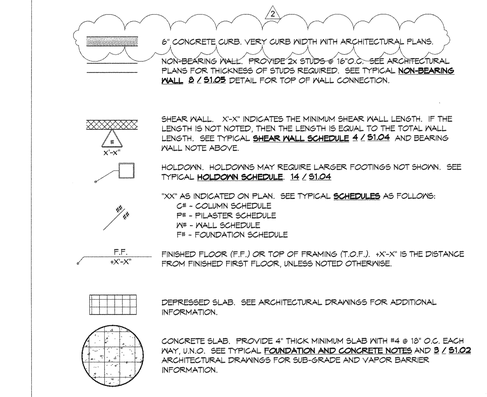
5. SEE CIVIL AND/OR ARCHITECTURAL SITE PLAN FOR LOCATION AND DIMENSIONS OF SIDEWALKS, MOW STRIPS, PLANTERS AND OTHER LANDSCAPING FEATURES.
6. SEE ARCHITECTURAL AND PLUMBING PLANS FOR LOCATION OF ALL PIPING AND DRAINS. SEE TYPICAL DETAILS FOR STRUCTURAL REQUIREMENTS AT LOCATIONS WHERE PIPES INTERSECT OR ALIGN NEXT TO FOOTINGS AND SLABS.
7. ALL EMBEDDED ITEMS SHALL BE IN PLACE AND SECURED PRIOR TO POURING OF CONCRETE.
8. ALL SILL PLATE ANCHOR BOLTS SHALL BE SIZED, SPACED, PLACED, AND HAVE THE WASHER AND MATERIAL FINISH AS SPECIFIED IN THE **WOOD NOTES**, 3 / S1.04
9. CONTRACTOR SHALL SUBMIT CONTROL JOINT PLAN FOR APPROVAL BY A.O.R. AND S.E.O.R. PRIOR TO POURING SLAB. SEE TYPICAL **CONTROL JOINT** 8 / S1.02 DETAIL FOR ADDITIONAL INFORMATION.
10. ALL EXTERIOR WALLS SHALL HAVE 15/32" SHEATHING WITH 10d @ 6"O.C. E.N. AND 10d @ 12"O.C. F.N., UNLESS NOTED OTHERWISE. SEE **WOOD NOTES** 3 / S1.04 FOR SHEATHING MATERIAL.
11. ALL TOP OF FOOTINGS SHALL BE 1'-0" BELOW FINISH SLAB, UNLESS NOTED OTHERWISE.
12. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1	WOOD FOUNDATION NOTES
S2.02	NOT TO SCALE



2	WOOD LEGEND
S2.02	NOT TO SCALE

STRUCTURAL COLUMN SCHEDULE		
MARK	TYPE	COMMENTS
C1	6x6	
C2	HSS4X4X1/4	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C3	W6X25	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C4	HSS6X5X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C5	HSS6X6X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C6	HSS5X5X1/2	SEE TYPICAL COLUMN BASE PLATE SCHEDULE

STRUCTURAL FOUNDATION SCHEDULE		
MARK	TYPE	REINFORCEMENT
F2	2'-0"SQUARE x 1'-6"DEEP	(2)#5 T&B EACH WAY
F2.5	2'-6"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3.5	3'-6"SQUARE x 1'-6"DEEP	(4)#5 T&B EACH WAY
F4	4'-0"SQUARE x 1'-6"DEEP	(5)#5 T&B EACH WAY
WF1	1'-0"WX1'-6"D CONTIN. FOOTING	(1)#5 T&B LONGITUDINAL BARS
WF1.5	1'-6"WIDE x 1'-6"DEEP CONTIN. FTG.	(2)#5 T&B LONGITUDINAL BARS
WF2	2'-0"WX1'-6"D contin. Footing	(3)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. T&B TRANSVERSE
WF3	3'-0"WX2'-0"D contin. Footing	(4)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. TIES TRANSVERSE
WF4	4'-0"WX2'-0"D contin. Footing	(5)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. TIES TRANSVERSE

4	STRUCTURAL SCHEDULES
	NOT TO SCALE

20	BUILDING B - FOUNDATION PLAN
S4.01 S2.02	1/8" = 1'-0"

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6011 N. Fresno, Suite 130 - Fresno, California 93710
Phone (559) 435-3881 Fax (559) 436-0887 E-Mail: design@somam.com
www.integrateddesigns.com

Revision Description:	Rev. Date:
ADDENDUM #2	09/18/19
Revision:	Revision Description:	Rev. Date:
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1		

Sheet Title: B FOUNDATION PLAN
Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date:	Date:	Designer:	DR:	PC:
01-31-18		SOC		

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TRACKING #: 63321-300

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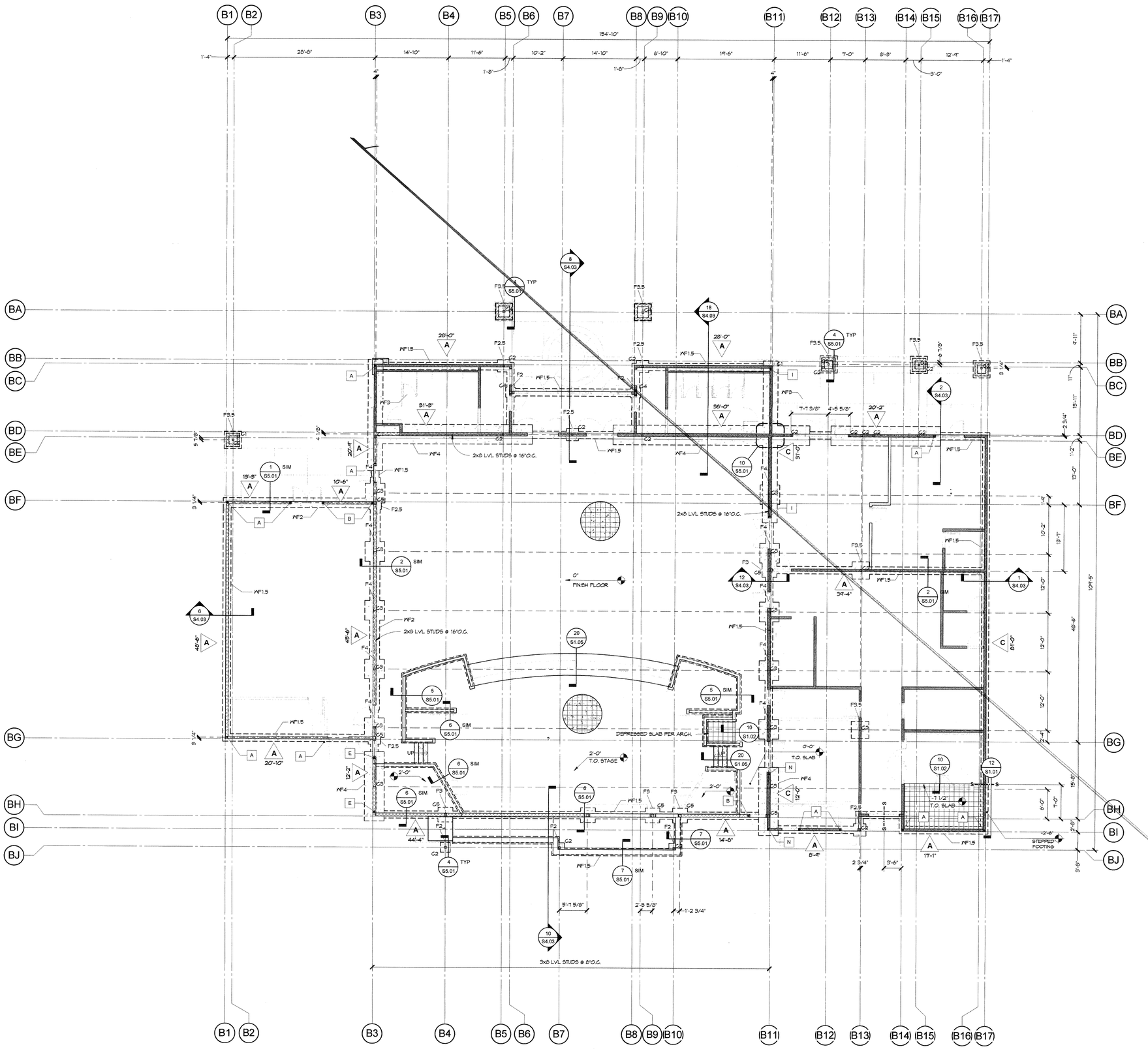
REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RANSOM
No. S2386
CIVIL
1-31-20
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711

Job No.:

Sheet No.: S2.02

Release: ADDENDUM NO. 2



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9. CONTRACTOR SHALL SUBMIT CONTROL JOINT PLAN FOR APPROVAL BY A.O.R. AND S.E.O.R. PRIOR TO POURING SLAB. SEE TYPICAL **CONTROL JOINT** 8 / S1.02 DETAIL FOR ADDITIONAL INFORMATION.
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11. ALL TOP OF FOOTINGS SHALL BE 1'-0" BELOW FINISH SLAB, UNLESS NOTED OTHERWISE.
12. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1	WOOD FOUNDATION NOTES
S2.02	NOT TO SCALE

6" CONCRETE CURB. VERY CURB WIDTH WITH ARCHITECTURAL PLANS.

NON-BEARING WALL. PROVIDE 2x STUDS @ 16"O.C.--SEE ARCHITECTURAL PLANS FOR THICKNESS OF STUDS REQUIRED. SEE TYPICAL **NON-BEARING WALL** 8 / S1.05 DETAIL FOR TOP OF WALL CONNECTION.

SHEAR WALL. X-X" INDICATES THE MINIMUM SHEAR WALL LENGTH. IF THE LENGTH IS NOT NOTED, THEN THE LENGTH IS EQUAL TO THE TOTAL WALL LENGTH. SEE TYPICAL **SHEAR WALL SCHEDULE** 4 / S1.04 AND BEARING WALL NOTE ABOVE.

HOLDOWN. HOLDOWNS MAY REQUIRE LARGER FOOTINGS NOT SHOWN. SEE TYPICAL **HOLDOWN SCHEDULE** 14 / S1.04

"XX" AS INDICATED ON PLAN. SEE TYPICAL **SCHEDULES** AS FOLLOWS:
C# - COLUMN SCHEDULE
F# - PILASTER SCHEDULE
WF - WALL SCHEDULE
F# - FOUNDATION SCHEDULE

FINISHED FLOOR (F.F.) OR TOP OF FRAMING (T.O.F.). +X-X" IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

DEPRESSED SLAB. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.

CONCRETE SLAB. PROVIDE 4" THICK MINIMUM SLAB WITH #4 @ 18" O.C. EACH WAY, U.N.O. SEE TYPICAL **FOUNDATION AND CONCRETE NOTES** AND 3 / S1.02 ARCHITECTURAL DRAWINGS FOR SUB-GRADE AND VAPOR BARRIER INFORMATION.

2	WOOD LEGEND
S2.02	NOT TO SCALE

STRUCTURAL COLUMN SCHEDULE

MARK	TYPE	COMMENTS
C1	6x6	
C2	HSS4X4X1/4	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C3	P6X25	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C4	HSS6X5X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C5	HSS6X6X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C6	HSS5X5X1/2	SEE TYPICAL COLUMN BASE PLATE SCHEDULE

STRUCTURAL FOUNDATION SCHEDULE

MARK	TYPE	REINFORCEMENT
F2	2'-0"SQUARE x 1'-6"DEEP	(2)#5 T&B EACH WAY
F2.5	2'-6"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
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4	STRUCTURAL SCHEDULES
	NOT TO SCALE

20	BUILDING B - FOUNDATION PLAN
S4.01 S2.02	1/8" = 1'-0"

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Phone (559) 439-5881 Fax (559) 436-0887 Email: design@somam.com
www.integratedesigns.com

Revision:	Rev. Date:	Revision Description:
	09/16/18	ADDENDUM #2

Sheet Title: B FOUNDATION PLAN

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18 | Date: | Designer: SOC | DR: | PIC:

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TRACKING #: 63321-300
ADDENDUM # 2

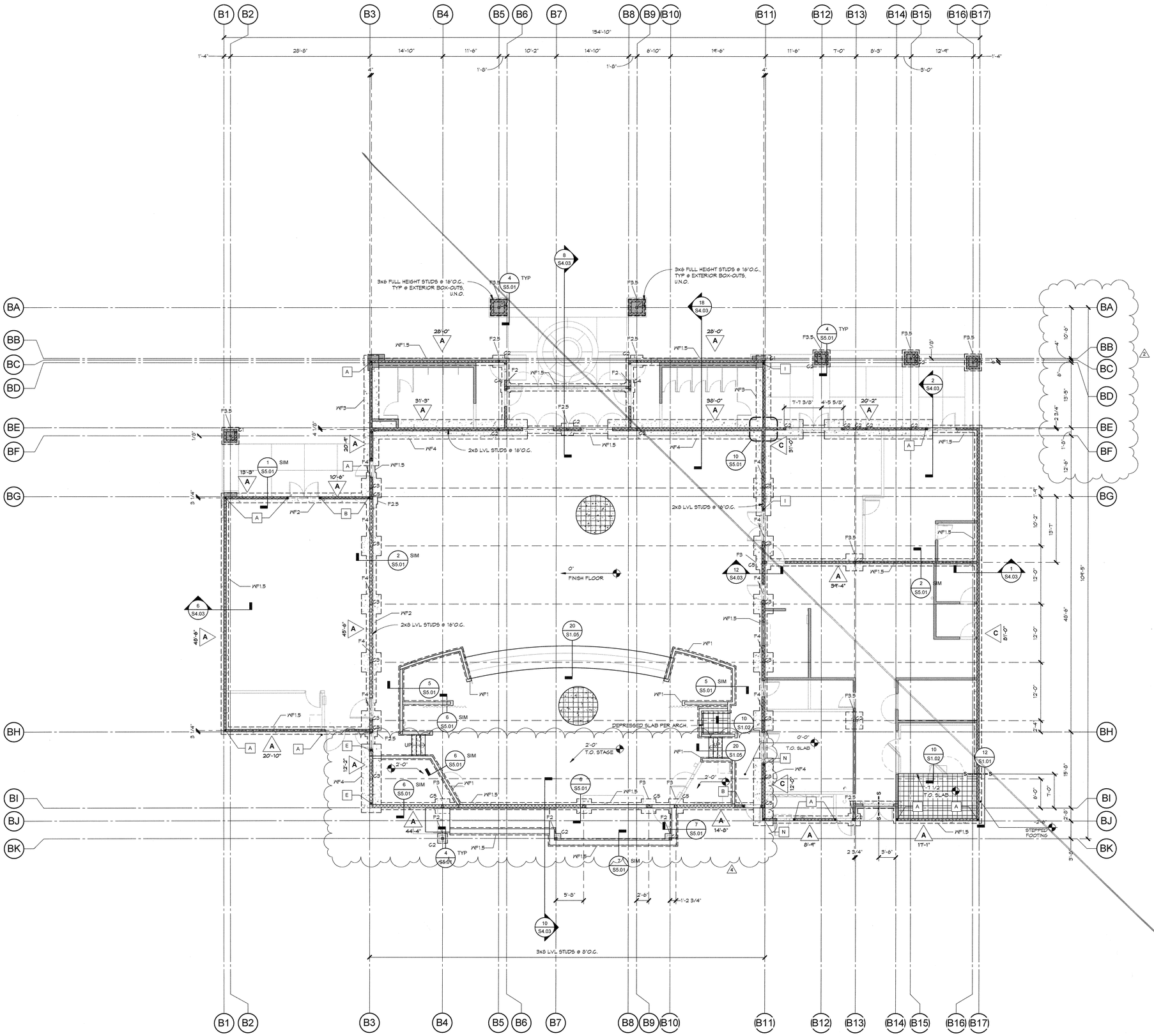
Stamp(s)
REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RANSOM
No. 52336
EXPIRES 12-31-20
CIVIL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711
(559) 431-7900

Job No.:

Sheet No.: S2.02

Release: ADDENDUM NO. 2



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1 WOOD FOUNDATION NOTES
S2.02 NOT TO SCALE

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2 WOOD LEGEND
S2.02 NOT TO SCALE

STRUCTURAL COLUMN SCHEDULE

MARK	TYPE	COMMENTS
C1	6x6	
C2	HSS4X4X1/4	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C3	P6X25	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
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STRUCTURAL FOUNDATION SCHEDULE

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4 STRUCTURAL SCHEDULES
NOT TO SCALE

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www.integrateddesigns.com

Revision:	Revision Description:	Rev. Date:
1	ADDENDUM #2	08/16/18
2	ADDENDUM #4	09-07-18

Sheet Title: B FOUNDATION PLAN
Project Name & Address: NEW E.S. INCREMENT 2, BAKERSFIELD CITY SCHOOL DISTRICT, @ CITADEL ROAD & MARDI GRAS COURT
Issue Date: 9-07-18
Date:
Designer: SOC
DR.
PC:

Agency Approval Stamp:
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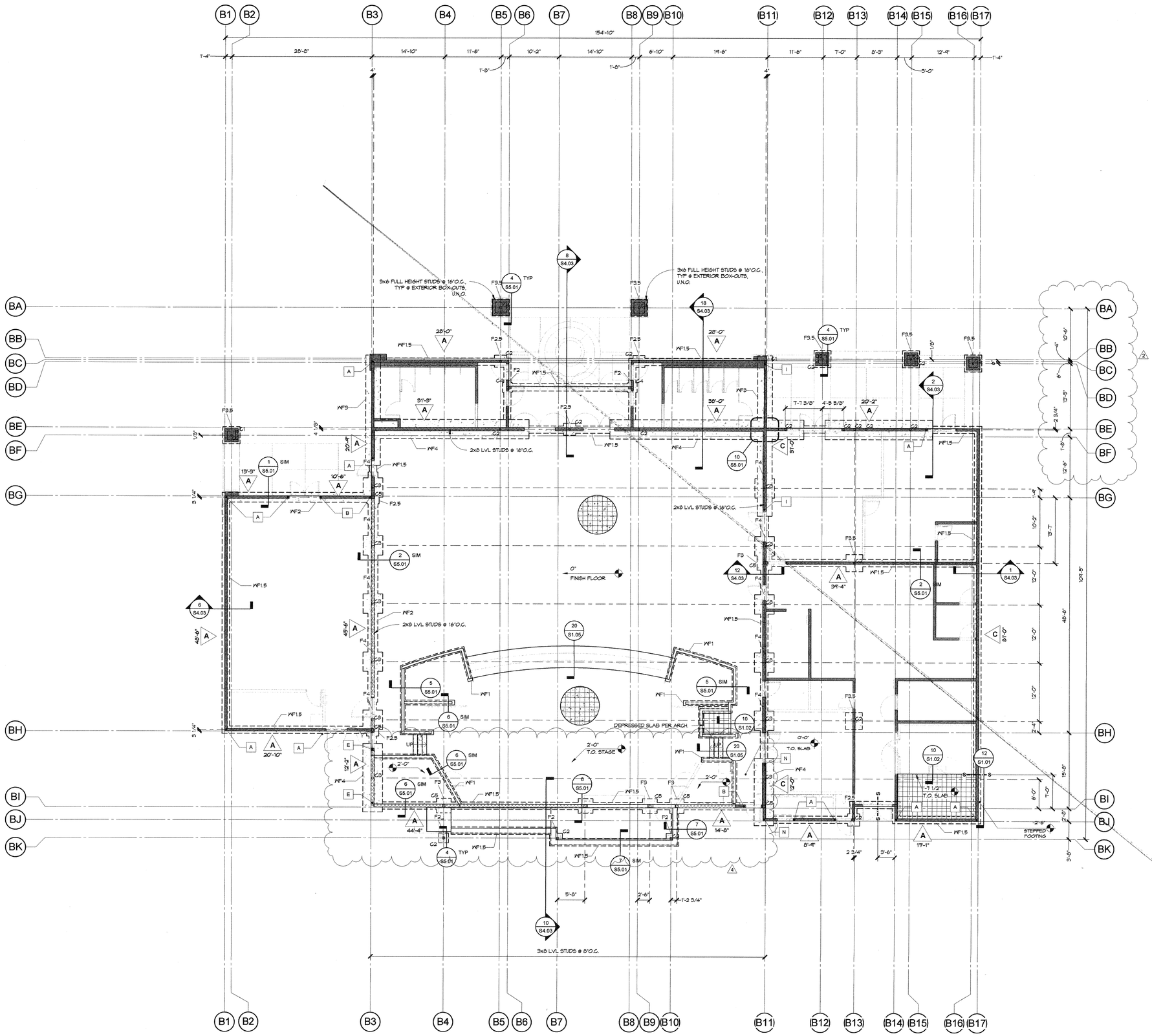
03-118394
AC.____FLS.____SS.____
DATE____
TRACKING #: 63321-300

Stamp(s)
REGISTERED PROFESSIONAL ENGINEER GAYLORD R. RANSON No.52386 EXPIRES 3-31-20 CIVIL
BROOKS-RANSON ASSOCIATES STRUCTURAL ENGINEERS CIVIL ENGINEERS 7415 N. PALM AVE., SUITE 100 FRESNO, CALIFORNIA 93711

Job No.:

Sheet No.: S2.02
Release: ADDENDUM NO. 4

20 BUILDING B - FOUNDATION PLAN
S4.01 S2.02 1/8" = 1'-0"



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6. SEE ARCHITECTURAL AND PLUMBING PLANS FOR LOCATION OF ALL PIPING AND DRAINS. SEE TYPICAL DETAILS FOR STRUCTURAL REQUIREMENTS AT LOCATIONS WHERE PIPES INTERSECT OR ALIGN NEXT TO FOOTINGS AND SLABS.
7. ALL EMBEDDED ITEMS SHALL BE IN PLACE AND SECURED PRIOR TO POURING OF CONCRETE.
8. ALL SILL PLATE ANCHOR BOLTS SHALL BE SIZED, SPACED, PLACED, AND HAVE THE WASHER AND MATERIAL FINISH AS SPECIFIED IN THE **WOOD NOTES** 3 / S1.04
9. CONTRACTOR SHALL SUBMIT CONTROL JOINT PLAN FOR APPROVAL BY A.O.R. AND S.E.O.R. PRIOR TO POURING SLAB. SEE TYPICAL **CONTROL JOINT** 3 / S1.02 DETAIL FOR ADDITIONAL INFORMATION.
10. ALL EXTERIOR WALLS SHALL HAVE 15/32" SHEATHING WITH 10d @ 6"O.C. E.N. AND 10d @ 12"O.C. F.N., UNLESS NOTED OTHERWISE. SEE **WOOD NOTES** 3 / S1.04 FOR SHEATHING MATERIAL.
11. ALL TOP OF FOOTINGS SHALL BE 1'-0" BELOW FINISH SLAB, UNLESS NOTED OTHERWISE.
12. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1	WOOD FOUNDATION NOTES
S2.02	NOT TO SCALE

6" CONCRETE CURB, VERY CURB WIDTH WITH ARCHITECTURAL PLANS.

NON-BEARING WALL. PROVIDE 2x STUDS @ 16"O.C.-SEE ARCHITECTURAL PLANS FOR THICKNESS OF STUDS REQUIRED. SEE TYPICAL **NON-BEARING WALL** 8 / S1.02 DETAIL FOR TOP OF WALL CONNECTION.

SHEAR WALL. X-X" INDICATES THE MINIMUM SHEAR WALL LENGTH. IF THE LENGTH IS NOT NOTED, THEN THE LENGTH IS EQUAL TO THE TOTAL WALL LENGTH. SEE TYPICAL **SHEAR WALL SCHEDULE** 4 / S1.04 AND BEARING WALL NOTE ABOVE.

HOLDOWN. HOLDOWNS MAY REQUIRE LARGER FOOTINGS NOT SHOWN. SEE TYPICAL **HOLDOWN SCHEDULE** 14 / S1.04

"XX" AS INDICATED ON PLAN. SEE TYPICAL **SCHEDULES** AS FOLLOWS:
- C# - COLUMN SCHEDULE
- P# - PILASTER SCHEDULE
- W# - WALL SCHEDULE
- F# - FOUNDATION SCHEDULE

FINISHED FLOOR (F.F.) OR TOP OF FRAMING (T.O.F.). +X-X" IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

DEPRESSED SLAB. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.

CONCRETE SLAB. PROVIDE 4" THICK MINIMUM SLAB WITH #4 @ 18" O.C. EACH WAY, U.N.O. SEE TYPICAL **FOUNDATION AND CONCRETE NOTES** AND 3 / S1.02 ARCHITECTURAL DRAWINGS FOR SUB-GRADE AND VAPOR BARRIER INFORMATION.

2	WOOD LEGEND
S2.02	NOT TO SCALE

STRUCTURAL COLUMN SCHEDULE		
MARK	TYPE	COMMENTS
C1	6x6	
C2	HSS4X4X1/4	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C3	W6X25	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C4	HSS6X6X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C5	HSS6X6X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C6	HSS5X5X1/2	SEE TYPICAL COLUMN BASE PLATE SCHEDULE

STRUCTURAL FOUNDATION SCHEDULE		
MARK	TYPE	REINFORCEMENT
F2	2'-0"SQUARE x 1'-6"DEEP	(2)#5 T&B EACH WAY
F2.5	2'-6"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3.5	3'-6"SQUARE x 1'-6"DEEP	(4)#5 T&B EACH WAY
F4	4'-0"SQUARE x 1'-6"DEEP	(5)#5 T&B EACH WAY
WF1	1'-0"WX1'-6"D CONTIN. FOOTING	(1)#5 T&B LONGITUDINAL BARS
WF1.5	1'-6"WIDE x 1'-6"DEEP CONTIN. FTG.	(2)#5 T&B LONGITUDINAL BARS
WF2	2'-0"WX1'-6"D Cont.in. Footing	(3)#5 T&B LONGITUDINAL BARS; #5 @ 12'O.C. T&B TRANSVERSE
WF3	3'-0"WX2'-0"D Cont.in. Footing	(4)#5 T&B LONGITUDINAL BARS; #5 @ 12'O.C. TIES TRANSVERSE
WF4	4'-0"WX2'-0"D Cont.in. Footing	(5)#5 T&B LONGITUDINAL BARS; #5 @ 12'O.C. TIES TRANSVERSE

4	STRUCTURAL SCHEDULES
	NOT TO SCALE

20	BUILDING B - FOUNDATION PLAN	
S4.01 S2.02	1/8" = 1'-0"	

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ARCHITECTURE ENGINEERING INTERIOR DESIGN CONSTRUCTION MANAGMENT
6011 N. Fresno, Suite 130 - Fresno, California 93710
Phone (559) 439-2651 Fax (559) 436-2653 E-Mail: design@somam.com
www.integrateddesigns.com

Revision Description:	Rev. Date:
ADDENDUM #2	08/16/18
ADDENDUM #4	09-07-18

Revision:	Revision Description:	Rev. Date:
2		
4		

Sheet Title: B FOUNDATION PLAN

Project Name & Address: NEW E.S. INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 9-07-18
Date:
Designer: SOC
DR:
PC:

Agency Approval Stamp:

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DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC SC FLS SI SS TN
DATE 5/16/19
TRACKING #: 63321-300
ADDENDUM # 4

Stamp(s)

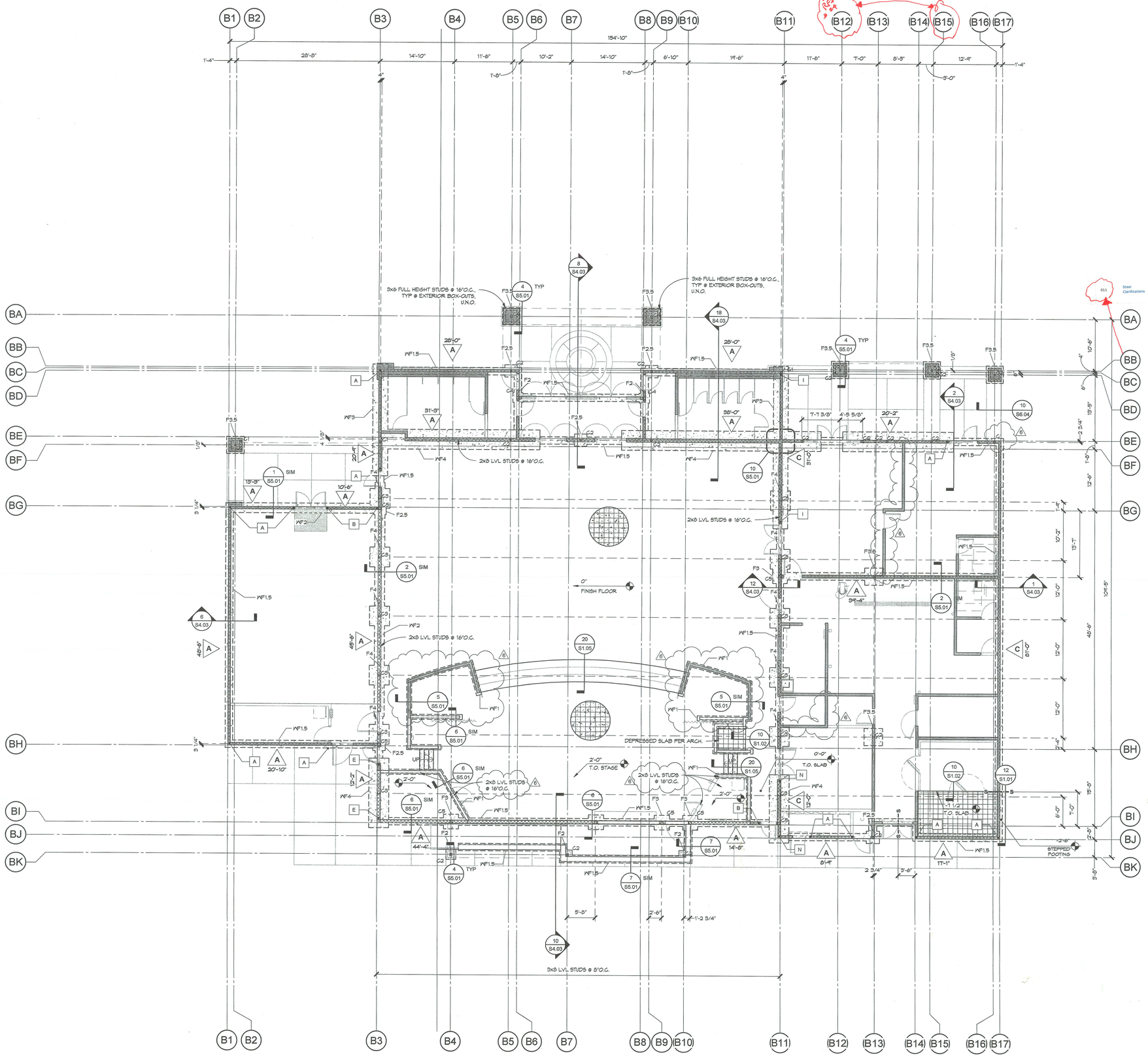
REGISTERED PROFESSIONAL ENGINEER
CA LLOYD R. RANSOM
No. S2386
EXPIRES 6-30-19
STRUCTURAL

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
LLOYD R. RANSOM
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711
TEL (559) 431-2700

Job No.:

Sheet No.: S2.02

Release: ADDENDUM NO. 4



1. SEE TYPICAL SHEETS FOR ALL GENERAL AND MATERIAL NOTES, AND ALL TYPICAL SCHEDULES AND DETAILS. THE INFORMATION ON THE TYPICAL SHEETS APPLY TO THE PROJECT AND ARE NOT SPECIFICALLY REFERENCED ON PLAN WORK, UNLESS NOTED OTHERWISE. IF TYPICAL DETAILS ARE SPECIFIED ON PLANS OR NOTES, THEY WILL BE REFERENCED WITH THE WORD "TYPICAL" FOLLOWED BY BOLD AND UNDERLINED TEXT STATING THE TITLE OF THE TYPICAL DETAIL OR NOTE.
2. ALL DIMENSIONS SHOWN ARE TO FACE OF STUDS AT EXTERIOR WALLS, CENTER OF COLUMN OR CENTER OF INTERIOR WALLS UNLESS NOTED OTHERWISE. EDGE OF SLAB AT PERIMETER OF BUILDING SHALL BE INDICATED IN SPECIFIC DETAILS AND ARCHITECTURAL. VERIFY ALL BUILDING DIMENSIONS WITH ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. NOTIFY THE ARCHITECT IMMEDIATELY IF THERE ARE ANY CONFLICTS WITH THE DIMENSIONS SHOWN.
3. ALL UNCLEAR AND/OR MISSING DETAILS SHALL BE BROUGHT TO THE STRUCTURAL ENGINEER'S ATTENTION BEFORE PROCEEDING WITH CONSTRUCTION.
4. ALL PAD AND CONTINUOUS FOOTINGS ARE TO BE CENTERED ON WALLS AND COLUMNS ABOVE UNLESS NOTED OTHERWISE.
5. SEE CIVIL AND/OR ARCHITECTURAL SITE PLAN FOR LOCATION AND DIMENSIONS OF SIDEWALKS, MOW STRIPS, PLANTERS AND OTHER LANDSCAPING FEATURES.
6. SEE ARCHITECTURAL AND PLUMBING PLANS FOR LOCATION OF ALL PIPING AND DRAINS. SEE TYPICAL DETAILS FOR STRUCTURAL REQUIREMENTS AT LOCATIONS WHERE PIPES INTERSECT OR ALIGN NEXT TO FOOTINGS AND SLABS.
7. ALL EMBEDDED ITEMS SHALL BE IN PLACE AND SECURED PRIOR TO POURING OF CONCRETE.
8. ALL SILL PLATE ANCHOR BOLTS SHALL BE SIZED, SPACED, PLACED, AND HAVE THE WASHER AND MATERIAL FINISH AS SPECIFIED IN THE WOOD NOTES, 3 / S1.04.
9. CONTRACTOR SHALL SUBMIT CONTROL JOINT PLAN FOR APPROVAL BY A.O.R. AND S.E.O.R. PRIOR TO POURING SLAB. SEE TYPICAL CONTROL JOINT 8 / S1.02 DETAIL FOR ADDITIONAL INFORMATION.
10. ALL EXTERIOR WALLS SHALL HAVE 15/32" SHEATHING WITH 10d @ 6"O.C. E.N. AND 10d @ 12"O.C. F.N., UNLESS NOTED OTHERWISE. SEE WOOD NOTES 3 / S1.04 FOR SHEATHING MATERIAL.
11. ALL TOP OF FOOTINGS SHALL BE 1'-0" BELOW FINISH SLAB, UNLESS NOTED OTHERWISE.
12. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1 WOOD FOUNDATION NOTES
S2.02 NOT TO SCALE

Contractor: Kern Steel (2)Please reference attached.1. Please reference page 1 of attached sketch. We have considered 1/8" offset dimension shown in structural plan S2.02 as typo error and fixed the columns to GL BB. Please confirm this is correct or if columns need to in fact be offset 1/8"-2. Please reference page 2 of attached sketch. Since design drawings do not have sufficient information regarding clouded columns height, we have fixed top of column at top of parapet elevation (Since columns are shown on high roof plan). Please confirm this is correct. If not please provide required changes.

The columns at grid BB do not need to be offset 1/8" Correct, top of column height to go to top of parapet elevation.

Closed

6" CONCRETE CURB. VERY CURB WIDTH WITH ARCHITECTURAL PLANS.

NON-BEARING WALL. PROVIDE 2x STUDS @ 16"O.C. SEE ARCHITECTURAL PLANS FOR THICKNESS OF STUDS REQUIRED. SEE TYPICAL NON-BEARING WALL 8 / S1.03 DETAIL FOR TOP OF WALL CONNECTION.

SHEAR WALL. X-X" INDICATES THE MINIMUM SHEAR WALL LENGTH. IF THE LENGTH IS NOT NOTED, THEN THE LENGTH IS EQUAL TO THE TOTAL WALL LENGTH. SEE TYPICAL SHEAR WALL SCHEDULE 4 / S1.04 AND BEARING WALL NOTE ABOVE.

HOLDOWN. HOLDOWNS MAY REQUIRE LARGER FOOTINGS NOT SHOWN. SEE TYPICAL HOLDOWN SCHEDULE 14 / S1.04

"XX" AS INDICATED ON PLAN. SEE TYPICAL SCHEDULES AS FOLLOWS:
C# - COLUMN SCHEDULE
F# - PILASTER SCHEDULE
W# - WALL SCHEDULE
F# - FOUNDATION SCHEDULE

F.F. +X-X" FINISHED FLOOR (F.F.) OR TOP OF FRAMING (T.O.F.). +X-X" IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

DEPRESSED SLAB. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION.

CONCRETE SLAB. PROVIDE 4" THICK MINIMUM SLAB WITH #4 @ 18" O.C. EACH WAY, U.N.O. SEE TYPICAL FOUNDATION AND CONCRETE NOTES AND 3 / S1.02 ARCHITECTURAL DRAWINGS FOR SUB-GRADE AND VAPOR BARRIER INFORMATION.

2 WOOD LEGEND
S2.02 NOT TO SCALE

STRUCTURAL COLUMN SCHEDULE

MARK	TYPE	COMMENTS
C1	6x6	
C2	HSS4X4X1/4	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C3	P16X25	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C4	HSS6X5X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C5	HSS6X6X5/16	SEE TYPICAL COLUMN BASE PLATE SCHEDULE
C6	HSS5X5X1/2	SEE TYPICAL COLUMN BASE PLATE SCHEDULE

STRUCTURAL FOUNDATION SCHEDULE

MARK	TYPE	REINFORCEMENT
F2	2'-0"SQUARE x 1'-6"DEEP	(2)#5 T&B EACH WAY
F2.5	2'-6"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3	3'-0"SQUARE x 1'-6"DEEP	(3)#5 T&B EACH WAY
F3.5	3'-6"SQUARE x 1'-6"DEEP	(4)#5 T&B EACH WAY
F4	4'-0"SQUARE x 1'-6"DEEP	(5)#5 T&B EACH WAY
WF1	1'-0"WX1'-6"D CONTIN. FOOTING	(1)#5 T&B LONGITUDINAL BARS
WF1.5	1'-6"WIDE x 1'-6"DEEP CONTIN. FTG.	(2)#5 T&B LONGITUDINAL BARS
WF2	2'-0"WX1'-6"D Contin. Footing	(3)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. T&B TRANSVERSE
WF3	3'-0"WX2'-0"D Contin. Footing	(4)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. TIES TRANSVERSE
WF4	4'-0"WX2'-0"D Contin. Footing	(5)#5 T&B LONGITUDINAL BARS; #5 @ 12"O.C. TIES TRANSVERSE

4 STRUCTURAL SCHEDULES
NOT TO SCALE

20 BUILDING B - FOUNDATION PLAN
S4.01 S2.02 1/8" = 1'-0"

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5011 N. Fresno, Suite 101 - Fresno, California 93710
Phone: (559) 436-0881 Fax: (559) 436-0887 E-Mail: design @somam.com
www.integratedddesigns.com

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1	ADDENDUM #2	08/16/18
2	ADDENDUM #4	08/12/18
3	ADDENDUM #6	

Sheet Title: B FOUNDATION PLAN

Project Name & Address: NEW E.S. INCREMENT 2, BAKERSFIELD CITY SCHOOL DISTRICT, @ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 9-07-18
Date:
Designer: SOC
DR:
PC:

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DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC. SE FLS. SH-SS TN
DATE 12/20/18

TRACKING #: 63321-300

Stamp(s)

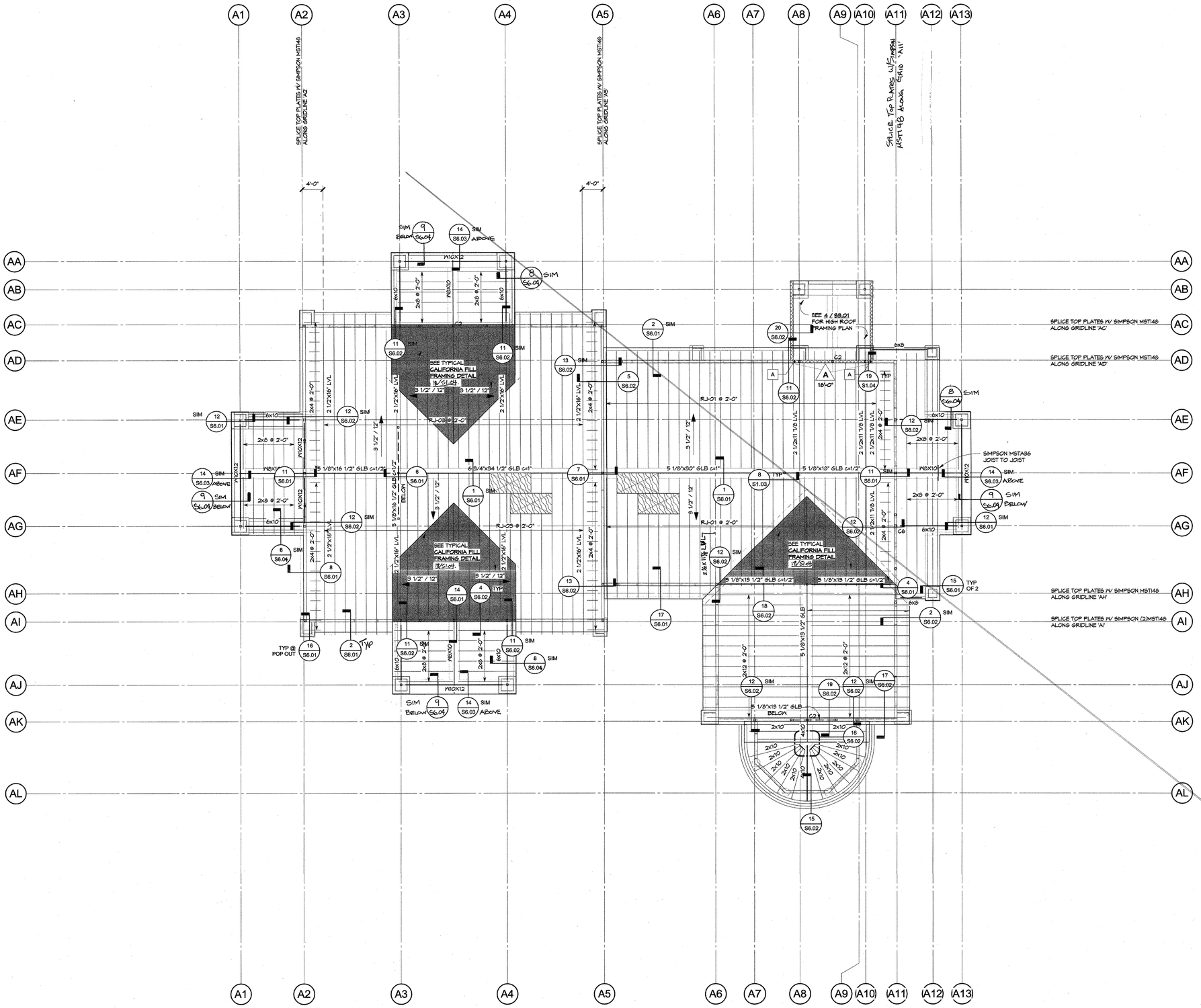
REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RANSOM
No. 52388
EXPIRES 12-31-20
CIVIL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
1700
7415 N. PALM AVENUE, SUITE 100
FRESNO, CALIFORNIA 93711

Job No.:

Sheet No.: S2.02

Release: ADDENDUM NO. 6



1. SEE TYPICAL SHEETS FOR ALL GENERAL AND MATERIAL NOTES, AND ALL TYPICAL SCHEDULES AND DETAILS. THE INFORMATION ON THE TYPICAL SHEETS APPLY TO THE PROJECT AND ARE NOT SPECIFICALLY REFERENCED ON PLAN WORK, UNLESS NOTED OTHERWISE. IF TYPICAL DETAILS ARE SPECIFIED ON PLANS OR NOTES, THEY WILL BE REFERENCED WITH THE WORD "TYPICAL" FOLLOWED BY **BOLD AND UNDERLINED** TEXT STATING THE TITLE OF THE TYPICAL DETAIL OR NOTE.
2. ALL UNCLEAR AND/OR MISSING DETAILS SHALL BE BROUGHT TO THE STRUCTURAL ENGINEER'S ATTENTION BEFORE PROCEEDING WITH CONSTRUCTION.
3. ALL EXTERIOR WALLS AND EXTERIOR SOFFITS SHALL HAVE 15/32" SHEATHING WITH 10d @ 6"O.C. E.N. AND 10d @ 12"O.C. F.N., UNLESS NOTED OTHERWISE. SEE **WOOD NOTES** 9 / S1.04 FOR SHEATHING MATERIAL.
4. ALL TOP OF FRAMING ABOVE FINISH SLAB VARIES, SEE PLAN FOR INFORMATION.
5. VERIFY ROOF SLOPE(S) WITH ARCHITECTURAL PLANS.
6. AT OPENINGS IN DIAPHRASM OR WALL SHEATHING SEE TYPICAL **BLOCKING AROUND OPENING** 6 / S1.03 DETAIL FOR INFORMATION, UNLESS NOTED OTHERWISE.
7. TOP PLATE SPLICES SHALL BE NAILED PER THE TYPICAL **TOP PLATE SPLICE** 3 / S1.03 DETAIL.
8. WHERE FRAMING MEMBERS ARE BUILT UP OF MULTIPLE MEMBERS, SEE TYPICAL **MULTIPLE-MEMBER CONNECTION** 7 / S1.03 DETAIL FOR FASTENER INFORMATION.
9. ALL FRAMING SHALL BE D.F. NO. 1 OR BETTER, UNLESS OTHERWISE NOTED. SEE TYPICAL **WOOD NOTES** 3 / S1.04 ADDITIONAL INFORMATION.
10. COORDINATE WITH ARCHITECTURAL IF ADDITIONAL FRAMING BLOCKING IS REQUIRED FOR SUSPENDED CEILING, SOFFITS AND FINISHES.
11. DIAPHRASM SHEATHING SHALL BE BOUNDARY NAILED TO ALL BLOCKING, BEAM AND RAFTERS THAT ARE IN LINE WITH SHEAR WALLS.
12. WHERE HEADERS ARE NOT SPECIFICALLY NOTED ON PLAN, SEE TYPICAL **WALL FRAMING AT OPENINGS** 13 / S1.04 DETAIL FOR HEADER INFORMATION.
13. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1 WOOD FRAMING NOTES
S3.01 | 12" = 1'-0"

WALL BELOW. SEE FRAMING PLAN AT LEVEL BELOW.

"XX" AS INDICATED ON PLAN. SEE TYPICAL **SCHEDULES** AS FOLLOWS:
C# - COLUMN SCHEDULE
P# - PILASTER SCHEDULE
W# - WALL SCHEDULE
F# - FOUNDATION SCHEDULE

F.F. +X'-X" — FINISHED FLOOR (F.F.) OR TOP OF FRAMING (T.O.F.). +X'-X" IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

HEADER. WHERE "SIZE" IS NOT INDICATED, SEE TYPICAL **WALL FRAMING AT OPENINGS FOR WOOD FRAMING DETAIL** 13 / S1.04

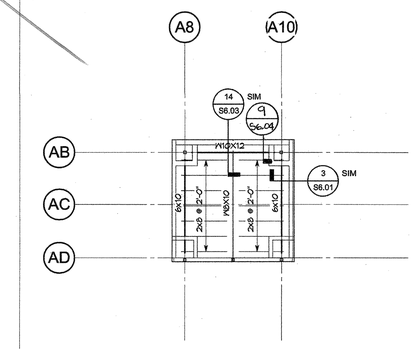
ROOF SHEATHING. PROVIDE 5/8" THICK SHEATHING WITH 8d NAILS @ 6:6:12, U.N.O. SEE TYPICAL **ROOF SHEATHING - UNBLOCKED** DETAIL 14 / S1.05
- FIELD
- PANEL EDGE
- CONTINUOUS PANEL EDGE & BOUNDARY

2 WOOD LEGEND
S3.01 | 1/4" = 1'-0"

HANGER SCHEDULE	
MEMBER SIZE	SIMPSON HANGER
2x4	HU524
2x6	HU526
2x12	HU212
2 1/2x11 7/8 LVL	U810
2 1/2x14 LVL	U814
2 1/2x16 LVL	U814
2 1/2x18 LVL	U814
RJ-01	U810
RJ-02	U814
RJ-03	U814
RJ-04	U814

NOTES:
1. USE 10d x 1 1/2" NAILS TYP, U.N.O.
2. USE SKEWED/SLOPED HANGERS AS INDICATED & REQUIRED.
3. HANGERS BASED ON 'SIMPSON' C2017-18 CATALOG.
4. SEE SECTIONS FOR ALTERNATE HANGER REQUIREMENTS.
5. FILL ALL NAIL HOLES.

3 HANGER SCHEDULE
S3.01 | NOT TO SCALE



4 BUILDING A HIGH RIDGE
S4.01 | S3.01 | 1/8" = 1'-0"

20 BUILDING A - ROOF FRAMING PLAN
S6.01 | S3.01 | 1/8" = 1'-0"

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www.integrateddesigns.com

Revision:	Revision Description:	Rev. Date:

Sheet Title: A ROOF FRAMING PLAN

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
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Designer: Designer
DR:
PC:

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OFFICE OF REGULATION SERVICES
03-118394
AC. FLS SS
DATE 06-22-18

TRACKING #: 63321-300

Stamp(s)

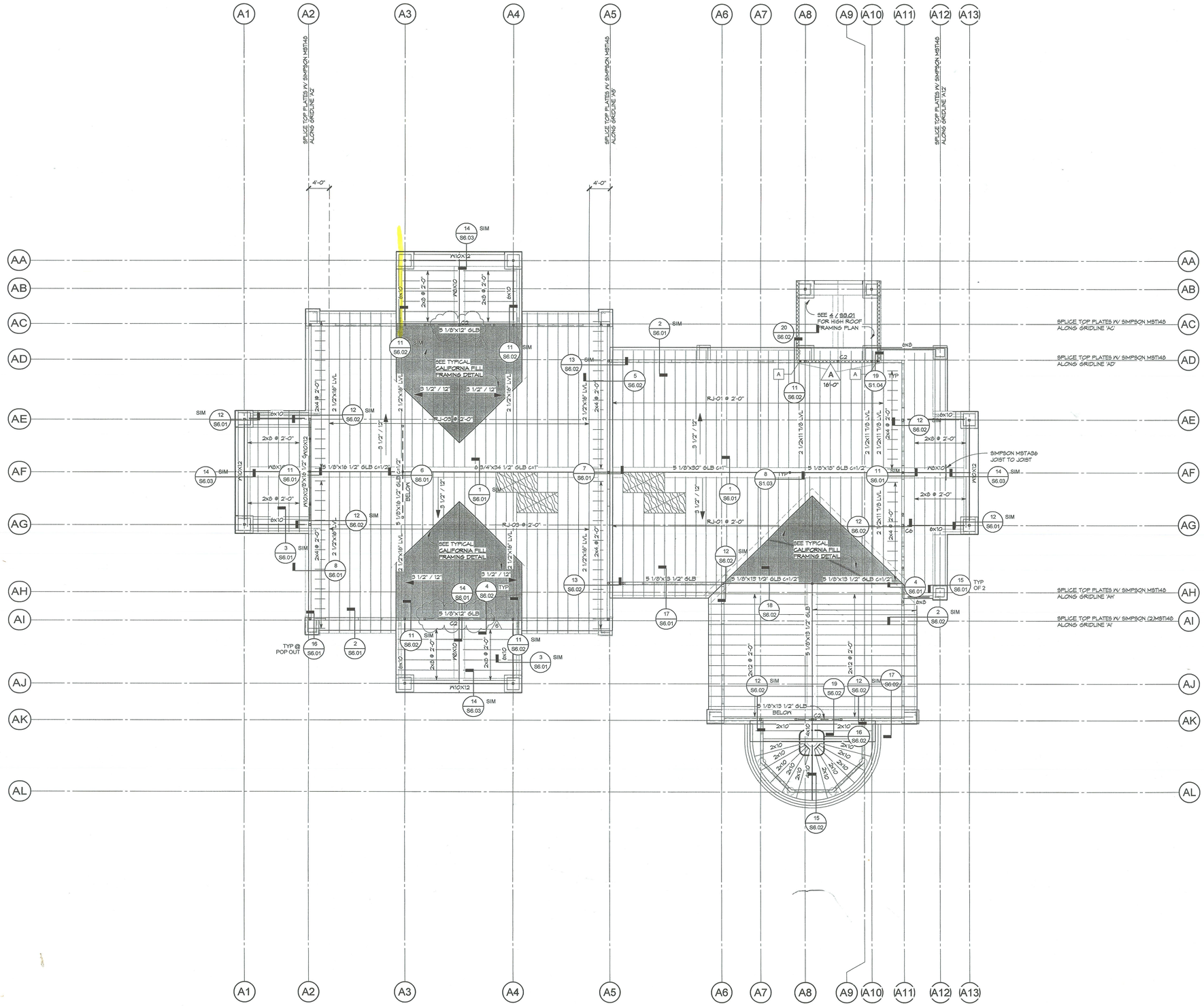
REGISTERED PROFESSIONAL ENGINEER GAETANO R. RANDAZZO No. 52385 EXPIRES 31-30-19 STRUCTURAL STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
Structural Engineers
Civil Engineers
7415 N. Palm Ave., Suite 100
Fresno, California 93711

Job No.:

Sheet No.: S3.01

Release:



1. SEE TYPICAL SHEETS FOR ALL GENERAL AND MATERIAL NOTES, AND ALL TYPICAL SCHEDULES AND DETAILS. THE INFORMATION ON THE TYPICAL SHEETS APPLY TO THE PROJECT AND ARE NOT SPECIFICALLY REFERENCED ON PLAN WORK, UNLESS NOTED OTHERWISE. IF TYPICAL DETAILS ARE SPECIFIED ON PLANS OR NOTES, THEY WILL BE REFERENCED WITH THE WORD "TYPICAL" FOLLOWED BY BOLD AND UNDERLINED TEXT STATING THE TITLE OF THE TYPICAL DETAIL OR NOTE.
2. ALL UNCLEAR AND/OR MISSING DETAILS SHALL BE BROUGHT TO THE STRUCTURAL ENGINEER'S ATTENTION BEFORE PROCEEDING WITH CONSTRUCTION.
3. ALL EXTERIOR WALLS AND EXTERIOR SOFFITS SHALL HAVE 15/32" SHEATHING WITH 10d @ 6"O.C. E.N. AND 10d @ 12"O.C. F.N., UNLESS NOTED OTHERWISE. SEE WOOD NOTES 2 / S1.04 FOR SHEATHING MATERIAL.
4. ALL TOP OF FRAMING ABOVE FINISH SLAB VARIES, SEE PLAN FOR INFORMATION.
5. VERIFY ROOF SLOPE(S) WITH ARCHITECTURAL PLANS.
6. AT OPENINGS IN DIAPHRAGM OR WALL SHEATHING SEE TYPICAL BLOCKING AROUND OPENING 6 / S1.05 DETAIL FOR INFORMATION, UNLESS NOTED OTHERWISE.
7. TOP PLATE SPLICES SHALL BE NAILED PER THE TYPICAL TOP PLATE SPLICE 3 / S1.05 DETAIL.
8. WHERE FRAMING MEMBERS ARE BUILT UP OF MULTIPLE MEMBERS, SEE TYPICAL MULTIPLE-MEMBER CONNECTION 7 / S1.05 DETAIL FOR FASTENER INFORMATION.
9. ALL FRAMING SHALL BE D.F. NO. 1 OR BETTER, UNLESS OTHERWISE NOTED. SEE TYPICAL WOOD NOTES 2 / S1.04 ADDITIONAL INFORMATION.
10. COORDINATE WITH ARCHITECTURAL IF ADDITIONAL FRAMING BLOCKING IS REQUIRED FOR SUSPENDED CEILING, SOFFITS AND FINISHES.
11. DIAPHRAGM SHEATHING SHALL BE BOUNDARY NAILED TO ALL BLOCKING, BEAM AND RAFTERS THAT ARE IN LINE WITH SHEAR WALLS.
12. WHERE HEADERS ARE NOT SPECIFICALLY NOTED ON PLAN, SEE TYPICAL WALL FRAMING AT OPENINGS 13 / S1.04 DETAIL FOR HEADER INFORMATION.
13. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1 WOOD FRAMING NOTES — S3.01 — 1/2" = 1'-0"

WALL BELOW. SEE FRAMING PLAN AT LEVEL BELOW.

"XX" AS INDICATED ON PLAN. SEE TYPICAL SCHEDULES AS FOLLOWS:
C# - COLUMN SCHEDULE
P# - PILASTER SCHEDULE
W# - WALL SCHEDULE
F# - FOUNDATION SCHEDULE

F.F. +X'-X" — FINISHED FLOOR (F.F.) OR TOP OF FRAMING (T.O.F.). +X'-X" IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

HEADER. WHERE "SIZE" IS NOT INDICATED, SEE TYPICAL WALL FRAMING AT OPENINGS FOR WOOD FRAMING DETAIL 13 / S1.04

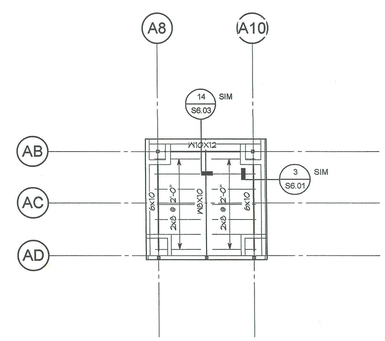
ROOF SHEATHING. PROVIDE 5/8" THICK SHEATHING WITH 8d NAILS @ 6@12, U.N.O. SEE TYPICAL ROOF SHEATHING - UNBLOCKED DETAIL 14 / S1.05
- 6" — FIELD
- 4" — PANEL EDGE
- 6" — CONTINUOUS PANEL EDGE & BOUNDARY

2 WOOD LEGEND — S3.01 — 1/4" = 1'-0"

HANGER SCHEDULE	
MEMBER SIZE	SIMPSON HANGER
2x4	HUS24
2x8	HUS26
2x12	HU212
2 1/2x11 7/8 LVL	U310
2 1/2x14 LVL	U314
2 1/2x16 LVL	U314
2 1/2x18 LVL	U314
RJ-01	U310
RJ-02	U314
RJ-03	U314
RJ-04	U314

NOTES:
1. USE 10d x 1 1/2" NAILS TYP. U.N.O.
2. USE SKEWED/SLOPED HANGERS AS INDICATED & REQUIRED.
3. HANGERS BASED ON SIMPSON C2017-18 CATALOG.
4. SEE SECTIONS FOR ALTERNATE HANGER REQUIREMENTS.
5. FILL ALL NAIL HOLES.

3 HANGER SCHEDULE — S3.01 — NOT TO SCALE



4 BUILDING A HIGH RIDGE — S4.01 — S3.01 — 1/8" = 1'-0"

20 BUILDING A - ROOF FRAMING PLAN — AD6-S3 — S3.01 — 1/8" = 1'-0"

Contractor: Lundgren Reference: S3.01 (4),A3.11 (2) Tower at (AD) & (AB),A6.10, E2.10 Question: A3.11(2) Colum line (AD) & (AB), shows a stucco to the bottom of the rafters. A6.10 at that same location shows a 10' ceiling height. Additionally: there is no access door shown in the 10' ceiling shows no access door to the light above the ceiling to below rafter location (E2.10). Can I get clarification?

Closed

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6311 N. Fresno, Suite 130 - Fresno, California 93710
Phone (559) 436-0681 Fax (559) 436-0688 Email: design@somam.com
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Revision:	Revision Description:	Rev. Date:
1	ADDENDUM #2	09/18/18
2	ADDENDUM #4	09/19/18
3	ADDENDUM #6	

Sheet Title: A ROOF FRAMING PLAN

Project Name & Address: NEW E.S. INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 9-07-18
Date:
Designer: Designer
DR:
PC:

Agency Approval Stamp:

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DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC SC FLS SH SS TN
DATE 12/20/18

TRACKING #: 63321-300

Stamp(s)

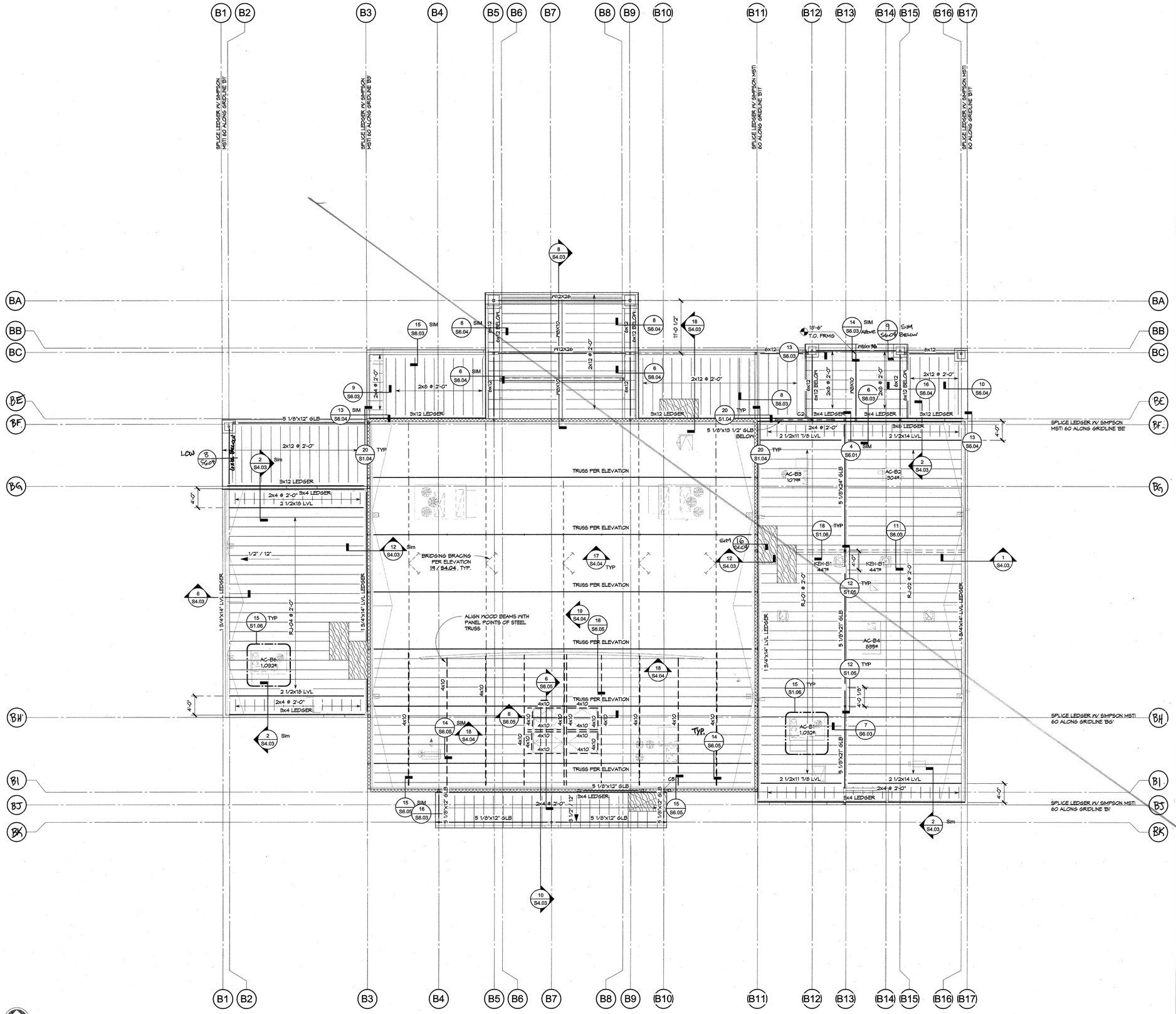
REGISTERED PROFESSIONAL ENGINEER — CASTLEROD R. RANSOM — No.52385 EXPIRES 12-31-20 — CIVIL — STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
Structural Engineers
Civil Engineers
7415 N. Palm Ave., Suite 100
Fresno, California 93711

Job No.:

Sheet No.: S3.01

Release: ADDENDUM NO. 6



1. SEE TYPICAL SHEETS FOR ALL GENERAL AND MATERIAL NOTES, AND ALL TYPICAL SCHEDULES AND DETAILS. THE INFORMATION ON THE TYPICAL SHEETS APPLY TO THE PROJECT AND ARE NOT SPECIFICALLY REFERENCED ON PLAN WORK, UNLESS NOTED OTHERWISE. IF TYPICAL DETAILS ARE SPECIFIED ON PLANS OR NOTES, THEY WILL BE REFERENCED WITH THE WORD "TYPICAL" FOLLOWED BY BOLD AND UNDERLINED TEXT STATING THE TITLE OF THE TYPICAL DETAIL OR NOTE.
2. ALL UNCLEAR AND/OR MISSING DETAILS SHALL BE BROUGHT TO THE STRUCTURAL ENGINEER'S ATTENTION BEFORE PROCEEDING WITH CONSTRUCTION.
3. ALL EXTERIOR WALLS AND EXTERIOR SOFFITS SHALL HAVE 15/32" SHEATHING WITH 10d @ 6"O.C. E.N. AND 10d @ 12"O.C. F.N., UNLESS NOTED OTHERWISE. SEE WOOD NOTES 3 / S1.04 FOR SHEATHING MATERIAL.
4. ALL TOP OF FRAMING ABOVE FINISH SLAB VARIES, SEE PLAN FOR INFORMATION.
5. VERIFY ROOF SLOPE(S) WITH ARCHITECTURAL PLANS.
6. AT OPENINGS IN DIAPHRAGM OR WALL SHEATHING SEE TYPICAL BLOCKING AROUND OPENING 6 / S1.05 DETAIL FOR INFORMATION, UNLESS NOTED OTHERWISE.
7. TOP PLATE SPLICES SHALL BE NAILED PER THE TYPICAL TOP PLATE SPLICE DETAIL. 3 / S1.05
8. WHERE FRAMING MEMBERS ARE BUILT UP OF MULTIPLE MEMBERS, SEE TYPICAL MULTIPLE-MEMBER CONNECTION 7 / S1.05 DETAIL FOR FASTENER INFORMATION.
9. ALL FRAMING SHALL BE D.F. NO. 1 OR BETTER, UNLESS OTHERWISE NOTED. SEE TYPICAL WOOD NOTES 3 / S1.04 ADDITIONAL INFORMATION.
10. COORDINATE WITH ARCHITECTURAL. IF ADDITIONAL FRAMING BLOCKING IS REQUIRED FOR SUSPENDED CEILING, SOFFITS AND FINISHES.
11. DIAPHRAGM SHEATHING SHALL BE BOUNDARY NAILED TO ALL BLOCKING, BEAM AND RAFTERS THAT ARE IN LINE WITH SHEAR WALLS.
12. WHERE HEADERS ARE NOT SPECIFICALLY NOTED ON PLAN, SEE TYPICAL WALL FRAMING AT OPENINGS 13 / S1.04 DETAIL FOR HEADER INFORMATION.
13. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1 WOOD FRAMING NOTES
S3.02 | NOT TO SCALE

BEARING WALL. ALL BEARING, EXTERIOR AND SHEAR WALLS SHALL BE 2x6 STUDS @ 16"O.C., U.N.O. SEE TYPICAL WALL FRAMING AT OPENINGS FOR WOOD FRAMING DETAIL 13 / S1.04

WALL BELOW. SEE FRAMING PLAN AT LEVEL BELOW.

"XX" AS INDICATED ON PLAN. SEE TYPICAL SCHEDULES AS FOLLOWS:
- CF - COLUMN SCHEDULE
- PS - PILASTER SCHEDULE
- WF - WALL SCHEDULE
- FS - FOUNDATION SCHEDULE

T.O.S. +X'-X" — TOP OF STEEL (T.O.S.) OR BOTTOM OF STEEL (B.O.S.). +X'-X" IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

SIZE (STUDS) / CAMBER — STEEL BEAM.

BEAM BOTTOM FLANGE BRACING. ARROW POINTS TOWARDS BOTTOM FLANGE OF BEAM BEING BRACED. SEE TYPICAL BEAM BOTTOM FLANGE BRACING DETAIL 13 / S1.03

COLUMN DOES NOT EXTEND BEYOND FRAMING LEVEL BEING DEPICTED.

ROOF SHEATHING. PROVIDE 5/8" THICK SHEATHING WITH 8d NAILS @ 6-6-12, U.N.O. SEE TYPICAL ROOF SHEATHING - UNBLOCKED DETAIL 14 / S1.03
- FIELD
- PANEL EDGE
- CONTINUOUS PANEL EDGE & BOUNDARY

3 STEEL LEGEND
S3.02 | NOT TO SCALE

HANGER SCHEDULE	
MEMBER SIZE	SIMPSON HANGER
2x4	HUS24
2x6	HUS26
2x12	HUS212
2 1/2x11 7/8 LVL	U310
2 1/2x14 LVL	U314
2 1/2x16 LVL	U314
2 1/2x18 LVL	U314
RJ-01	U310
RJ-02	U314
RJ-03	U314
RJ-04	U314

NOTES:
1. USE 10d x 1 1/2" NAILS TYP, U.N.O.
2. USE SKEWED/SLOPED HANGERS AS INDICATED & REQUIRED.
3. HANGERS BASED ON 'SIMPSON' C2013 CATALOG.
4. SEE SECTIONS FOR ALTERNATE HANGER REQUIREMENTS.
5. FILL ALL NAIL HOLES.

4 HANGER SCHEDULE
S3.02 | NOT TO SCALE

20 BUILDING B - LOW ROOF FRAMING PLAN
S4.03 | S3.02 | 1/8" = 1'-0"

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www.integrateddesigns.com

Revision:	Revision Description:	Rev. Date:

Sheet Title: B LOW ROOF FRAMING PLAN

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18 | Date: | Designer: | Designer | DR: | PC:

Agency Approval Stamp:

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DATE 08-22-18
TRACKING #: 63321-300

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REGISTERED PROFESSIONAL ENGINEER — GAYLORD R. HANSEN — No. 34844 — RENEWAL 3-31-20 — STRUCTURAL — STATE OF CALIFORNIA

BROOKS-RANDOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7425 N. First Ave., Suite 120
Fresno, California 93711
17000

Job No.:

Sheet No.: S3.02

Release:

Queries Regarding AB Drawings Approval Return

0024

Contractor: Kern Steel (06) 1. Please reference Page 1 & 2 of an attached sketch at the clouded area of structural drawings. We have placed column at the center of GL B/12 and B/13 at the ridge which makes 2'- 1 1/2" away from GL B/13 but in approval return (Pg 3) it is noted as 2'- 1 1/4" away from GL B/13. Please verify and confirm that our interpretation to proceed with 2- 1 1/2" from GL B/13 is correct.2. Please reference page 2 thru 4 of an attached sketch, we are not providing any columns at a clouded location even though structural drawing graphically shows column at these locations (but have not provided sufficient details). Please confirm.3. Please reference page 5 & 6 of an attached sketch, we are not providing any columns at a clouded location even though Architectural drawing graphically shows column at these locations (but have not provided sufficient details). Please confirm.4. Please reference page 7 of an attached sketch, regarding anchor bolt detail. We are proceeding with a plate and double nuts. Confirm.

Use 2'-1 1/2" off gridline B/13. Confirmed, columns shown were incorrectly shown on architectural base drawings. Confirmed, columns shown were incorrectly shown on architectural base drawings. Ok to proceed with plate and double nut.

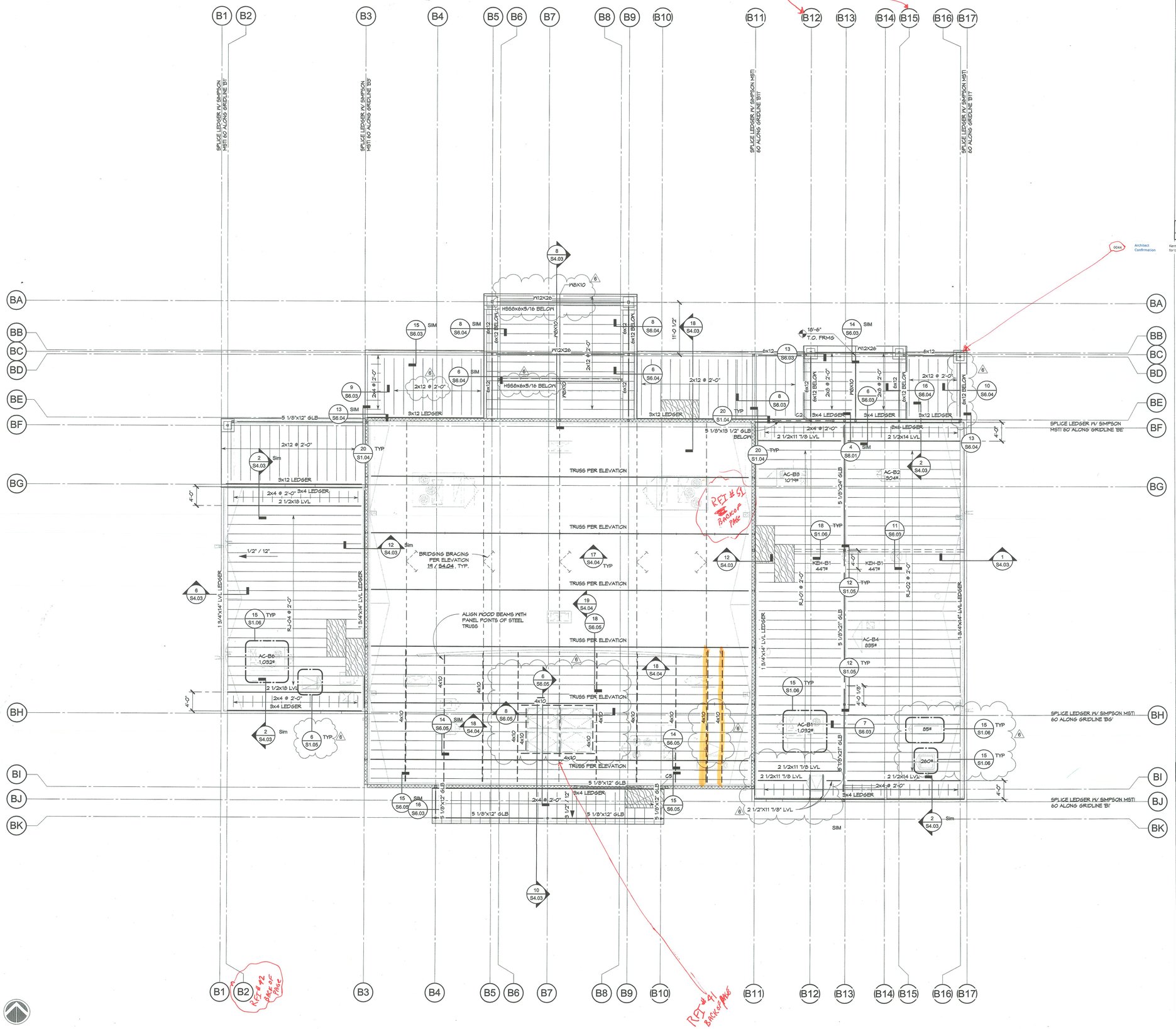
Closed

0044

Architect Confirmation

Kern Steel (17) 1. Please reference page 1 & 2 of the attached sketch. Approver commented as "Ok per structural, architect to confirm" for the clouded top of column elevation. Please verify and confirm the clouded column elevation.

The clouded column elevation is okay.



1 WOOD FRAMING NOTES

S3.02 | NOT TO SCALE

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3. ALL EXTERIOR WALLS AND EXTERIOR SOFFITS SHALL HAVE 15/32" SHEATHING WITH 10d @ 6"O.C. E.N. AND 10d @ 12"O.C. F.N., UNLESS NOTED OTHERWISE. SEE MOOD NOTES 3 / S1.04 FOR SHEATHING MATERIAL.
4. ALL TOP OF FRAMING ABOVE FINISH SLAB VARIES, SEE PLAN FOR INFORMATION.
5. VERIFY ROOF SLOPE(S) WITH ARCHITECTURAL PLANS.
6. AT OPENINGS IN DIAPHRAGM OR WALL SHEATHING SEE TYPICAL BLOCKING AROUND OPENING 6 / S1.05 DETAIL FOR INFORMATION, UNLESS NOTED OTHERWISE.
7. TOP PLATE SPLICES SHALL BE NAILED PER THE TYPICAL TOP PLATE SPLICE DETAIL 3 / S1.05
8. WHERE FRAMING MEMBERS ARE BUILT UP OF MULTIPLE MEMBERS, SEE TYPICAL MULTIPLE-MEMBER CONNECTION 1 / S1.05 DETAIL FOR FASTENER INFORMATION.
9. ALL FRAMING SHALL BE D.F. NO. 1 OR BETTER, UNLESS OTHERWISE NOTED. SEE TYPICAL MOOD NOTES 3 / S1.04 ADDITIONAL INFORMATION.
10. COORDINATE WITH ARCHITECTURAL IF ADDITIONAL FRAMING BLOCKING IS REQUIRED FOR SUSPENDED CEILING, SOFFITS AND FINISHES.
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BEARING WALL. ALL BEARING, EXTERIOR AND SHEAR WALLS SHALL BE 2x6 STUDS @ 16"O.C., U.N.O. SEE TYPICAL WALL FRAMING AT OPENINGS FOR MOOD FRAMING DETAIL 19 / S1.04

WALL BELOW. SEE FRAMING PLAN AT LEVEL BELOW.

"XX" AS INDICATED ON PLAN. SEE TYPICAL SCHEDULES AS FOLLOWS:
C# - COLUMN SCHEDULE
P# - PILASTER SCHEDULE
W# - WALL SCHEDULE
F# - FOUNDATION SCHEDULE

T.O.S. +X'-X" — TOP OF STEEL (T.O.S.) OR BOTTOM OF STEEL (B.O.S.). +X'-X" IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

SIZE (STUDS) CAMBER — STEEL BEAM.

BEAM BOTTOM FLANGE BRACING. ARROW POINTS TOWARDS BOTTOM FLANGE OF BEAM BEING BRACED. SEE TYPICAL BEAM BOTTOM FLANGE BRACING DETAIL 13 / S1.03

COLUMN DOES NOT EXTEND BEYOND FRAMING LEVEL BEING DEPICTED.

ROOF SHEATHING. PROVIDE 5/8" THICK SHEATHING WITH 8d NAILS @ 6-6-12, U.N.O. SEE TYPICAL ROOF SHEATHING - UNBLOCKED DETAIL 14 / S1.05
- FIELD
- PANEL EDGE
- CONTINUOUS PANEL EDGE & BOUNDARY

3 STEEL LEGEND

S3.02 | NOT TO SCALE

HANGER SCHEDULE

MEMBER SIZE	SIMPSON HANGER
2x4	HU524
2x6	HU526
2x12	HU5212
2 1/2x11 7/8 LVL	U310
2 1/2x14 LVL	U314
2 1/2x16 LVL	U314
2 1/2x18 LVL	U314
RJ-01	U310
RJ-02	U314
RJ-03	U314
RJ-04	U314

NOTES:
1. USE 10d x 1 1/2" NAILS TYP. U.N.O.
2. USE SKEWED/SLOPED HANGERS AS INDICATED & REQUIRED.
3. HANGERS BASED ON SIMPSON C2013 CATALOG.
4. SEE SECTIONS FOR ALTERNATE HANGER REQUIREMENTS.
5. FILL ALL NAIL HOLES.

4 HANGER SCHEDULE

S3.02 | NOT TO SCALE

20 BUILDING B - LOW ROOF FRAMING PLAN

S4.03 S3.02 | 1/8" = 1'-0"

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Phone: (559) 436-0881 Fax: (559) 436-0887 E-Mail: design @somam.com
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Revision:	Revision Description:	Rev. Date:
	ADDENDUM #2	08/16/18
	ADDENDUM #4	
	ADDENDUM #6	09/12/18

Sheet Title: B LOW ROOF FRAMING PLAN

Project Name & Address: NEW E.S. INCREMENT 2, BAKERSFIELD CITY SCHOOL DISTRICT, @ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 9-07-18 | Date: | Designer: Designer | DR: | PC:

Agency Approval Stamp:

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AC SC FLS SIT SS TH
DATE 12/20/18
TRACKING #: 63321-300

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REGISTERED PROFESSIONAL ENGINEER — CHRISTOPHER R. BROOKS — No. 52386 — EXPIRES 3-31-19 — STRUCTURAL — STATE OF CALIFORNIA

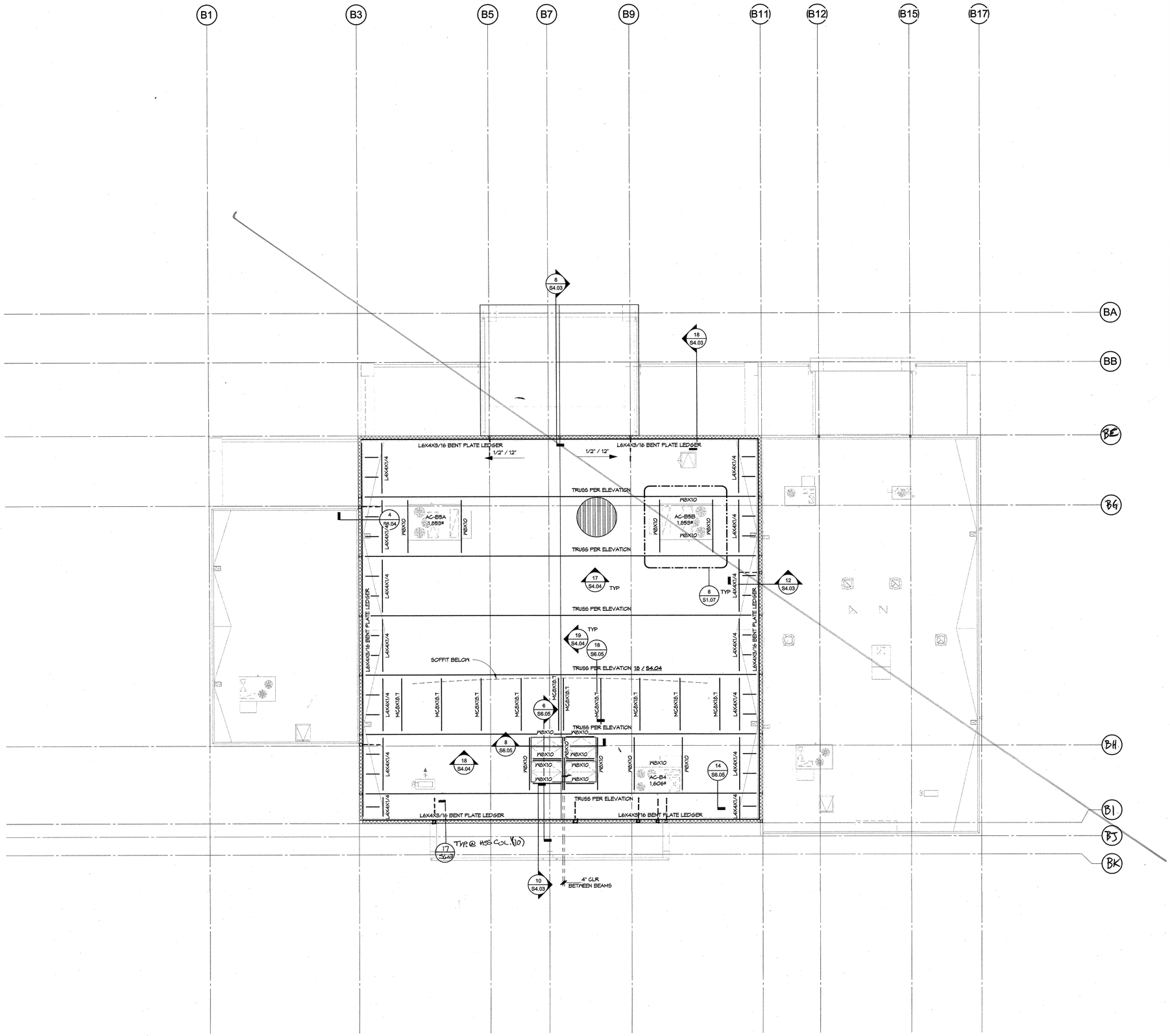
BROOKS-RANDOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711
17001

Job No.:

Sheet No.: S3.02

Release: ADDENDUM NO. 6

0041	Building B Mechanical Units	Item Steel-(14) 1. Approver comments at "Or per structural. Mechanical to verify" for cranked mechanical unit locations and hanger support. Please verify and comments hanging mechanical unit locating dimension and advise if any additional support is needed. 2. Refer page-2 and 3 of the attached sketch. Approver comments to verify. Pre-cranked duct size, opening size and operating location. Please verify and confirm the opening size and location of the mechanical unit by corordinating with the mechanical unit	Coordinate all dimensions with the prime contractors that Uunsen the sunear vent, roof hatch, mechanical units	Closed	
0042	Bldg B Connection Arrangements	Item Steel-(15) 1. Refer page-1 of the attached sketch. The cranked column is free-standing without any lateral support. Please advise if any lateral support is needed and also furnish the top of column elevation. 2. Refer page-2 & 3 of the attached sketch. Approver has commented to fix beam at column centre. Since columns at beams end are at offset (refer page-3), we have fixed beam at the center of one column. Please verify and confirm the cranked connection arrangement at offset and 3. Refer page-4 of the attached sketch. We are providing 4X10 wood per cranked approver comment and connecting to the bottom chord of the truss. For other locations, we have provided 4X10 wood at panel point of a truss. So please furnish bearing dimension for 4X10 wood arch support. Please advise if any lateral support is needed and also furnish the top of column elevation. 2. Refer page-2 & 3 of the attached sketch. We have fixed beam at the center of one column. Please verify and confirm the cranked connection arrangement at offset and 3. Refer page-4 of the attached sketch. We are providing 4X10 wood per cranked approver comments and connecting to the bottom chord of the truss. For other locations, we have provided 4X10 wood at panel point of a truss. So please furnish bearing dimension for 4X10 wood arch support. Please advise if any lateral support is needed and also furnish the top of column elevation. 2. Refer page-2 & 3 of the attached sketch. Approver has commented to fix beam at column centre. Since columns at beams end are at offset (refer page-3), we



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3. ALL TOP OF FRAMING ABOVE FINISH SLAB VARIES, SEE PLAN FOR INFORMATION.
4. VERIFY ROOF SLOPE(S) WITH ARCHITECTURAL PLANS.
5. SEE TYPICAL **MECHANICAL UNIT ATTACHMENT** 8 / S1.07 DETAIL FOR ROOF MOUNTED EQUIPMENT. CONTRACTOR SHALL VERIFY AND COORDINATE WEIGHTS AND LOCATIONS OF ALL ROOF SUPPORTED MECHANICAL AND ELECTRICAL UNITS, AND NOTIFY THE STRUCTURAL ENGINEER OF RECORD IF ANY DISCREPANCIES ARE DETERMINED.
6. AT OPENINGS IN DECKING SEE TYPICAL **OPENINGS IN STEEL DECK** 5 / S1.07 DETAILS FOR INFORMATION, UNLESS NOTED OTHERWISE. VERIFY OPENING SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
7. FOR FRAMING AT DRAINS, SEE TYPICAL **LARGE OPENINGS IN STEEL DECK** 6 / S1.07 DETAIL AND INFORMATION ON PLANS.
8. FOR ALL STEEL TO STEEL CONNECTIONS SEE TYPICAL **BEAM-TO-COLUMN CONNECTION** 18 / S1.03 AND **BEAM-TO-GIRDER CONNECTION** SCHEDULES. 7 / S1.03
9. THE FIRST SHEET OF STEEL DECKING ADJACENT AND PARALLEL TO CHORDS, COLLECTORS AND BRACED FRAME LINES (BOTH SIDES IF APPLICABLE) SHALL BE A FULL WIDTH SHEET. PROVIDE 2 ROWS OF PUDDLE WELDS AT ALL INTERIOR LATERAL RESISTING FRAME LINES.
10. ALL CHORD & COLLECTOR BEAMS TO BE INSTALLED PRIOR TO WELDING OF METAL DECKING.
11. BEAMS ARE EQUALLY SPACED BETWEEN MAIN BEAMS OR COLUMNS, U.N.O.
12. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1 STEEL FRAMING NOTES
S3.03 NOT TO SCALE

WALL BELOW. SEE FRAMING PLAN AT LEVEL BELOW.

"XX" AS INDICATED ON PLAN. SEE TYPICAL **SCHEDULES** AS FOLLOWS:
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P# - PILASTER SCHEDULE
W# - WALL SCHEDULE
F# - FOUNDATION SCHEDULE

T.O.S. +X'-X" — TOP OF STEEL (T.O.S.) OR BOTTOM OF STEEL (B.O.S.). +X'-X" IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

SIZE (STUDS) / CAMBER — STEEL BEAM.

BEAM BOTTOM FLANGE BRACING. ARROW POINTS TOWARDS BOTTOM FLANGE OF BEAM BEING BRACED. SEE TYPICAL **BEAM BOTTOM FLANGE BRACING** DETAIL 13 / S1.03

STEEL OR COMPOSITE DECK OPENING. SEE TYPICAL **SMALL/LARGE OPENINGS IN STEEL DECK FOR FLOOR OR ROOF DECK** 5 / S1.07 & / S1.07 DETAIL, U.N.O. VERIFY ALL OPENINGS WITH ARCHITECTURAL AND MECHANICAL PLANS.

STEEL DECK. INDICATES DIRECTION OF DECKING. SEE TYPICAL **STEEL ROOF DECK NOTES** 2 / S1.07 AND TYPICAL **DECK ATTACHMENT LAYOUT** 9 / S1.07 DETAILS.

3 STEEL LEGEND
S3.03 1/4" = 1'-0"

20 BUILDING B - HIGH ROOF FRAMING PLAN
S4.04 S3.03 1/8" = 1'-0"

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Revision:	Revision Description:	Rev. Date:	Revision:	Rev. Date:	Revision Description:	Rev. Date:

Sheet Title: B HIGH ROOF FRAMING PLAN
Issue Date: 01-31-18
Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT
Designer: Designer
DR: PC:

Agency Approval Stamp:

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AC. FLR SS
DATE 06-22-18
TRACKING #: 63321-300

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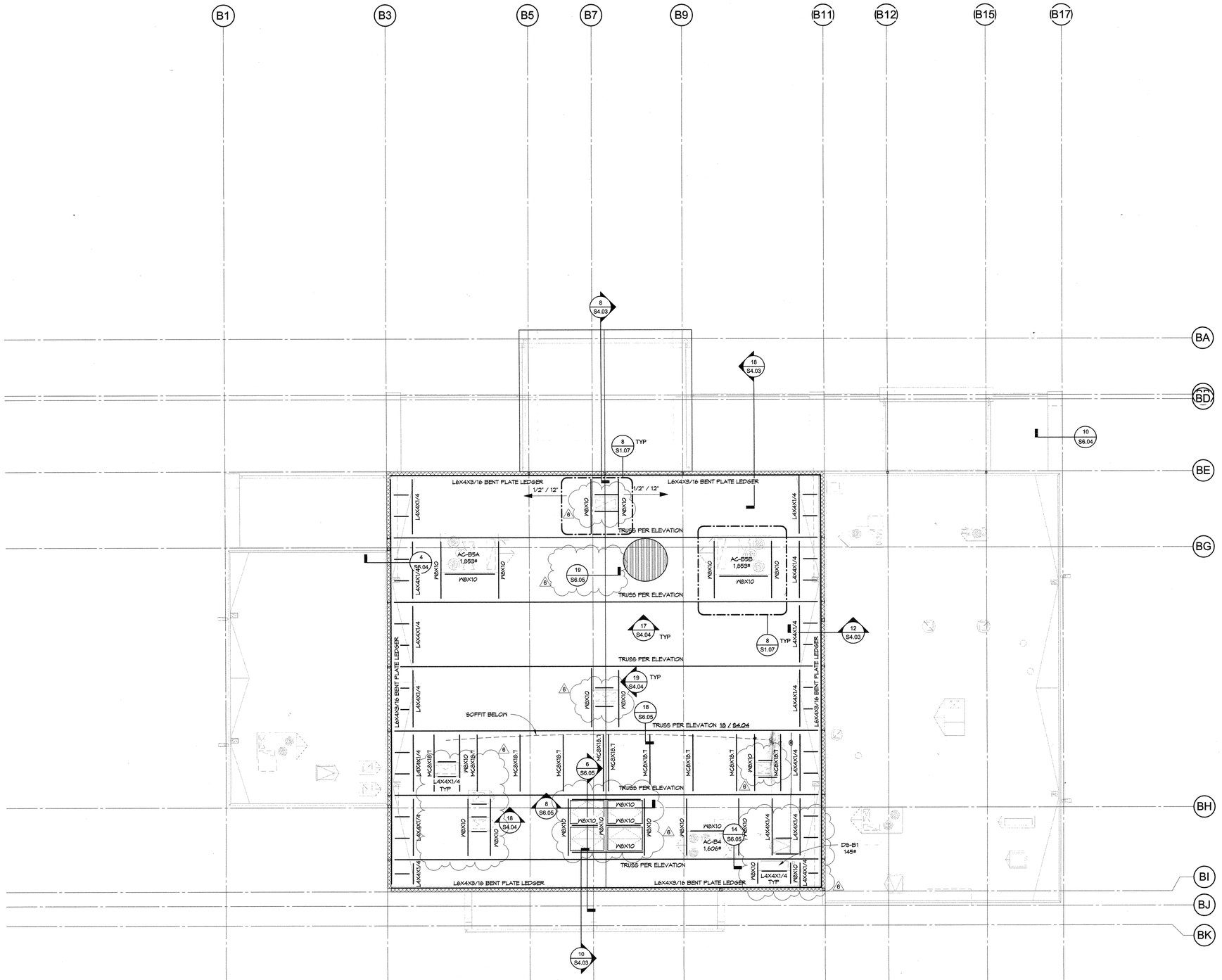
REGISTERED PROFESSIONAL ENGINEER — OAKLAND R. RANSOM — No. 52338 — EXPIRES 6-30-20 — STRUCTURAL — STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711

Job No.:

Sheet No.: S3.03

Release:



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6. AT OPENINGS IN DECKING SEE TYPICAL **OPENINGS IN STEEL DECK** 5 / S1.07 DETAILS FOR INFORMATION, UNLESS NOTED OTHERWISE. VERIFY OPENING SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
7. FOR FRAMING AT DRAINS, SEE TYPICAL **LARGE OPENINGS IN STEEL DECK** 6 / S1.07 DETAIL AND INFORMATION ON PLANS.
8. FOR ALL STEEL TO STEEL CONNECTIONS SEE TYPICAL **BEAM-TO-COLUMN CONNECTION** 18 / S1.03 AND **BEAM-TO-GIRDER CONNECTION** SCHEDULES. 7 / S1.03
9. THE FIRST SHEET OF STEEL DECKING ADJACENT AND PARALLEL TO CHORDS, COLLECTORS AND BRACED FRAME LINES (BOTH SIDES IF APPLICABLE) SHALL BE A FULL WIDTH SHEET. PROVIDE 2 ROWS OF PUDDLE WELDS AT ALL INTERIOR LATERAL RESISTING FRAME LINES.
10. ALL CHORD & COLLECTOR BEAMS TO BE INSTALLED PRIOR TO WELDING OF METAL DECKING.
11. BEAMS ARE EQUALLY SPACED BETWEEN MAIN BEAMS OR COLUMNS, U.N.O.
12. ALL ITEMS ARE NEW UNLESS NOTED OTHERWISE.

1 STEEL FRAMING NOTES
S3.03 NOT TO SCALE

WALL BELOW. SEE FRAMING PLAN AT LEVEL BELOW.

"XX" AS INDICATED ON PLAN. SEE TYPICAL **SCHEDULES** AS FOLLOWS:
C# - COLUMN SCHEDULE
P# - PILASTER SCHEDULE
W# - WALL SCHEDULE
F# - FOUNDATION SCHEDULE

T.O.S +X'-X" — TOP OF STEEL (T.O.S.) OR BOTTOM OF STEEL (B.O.S.). +X'-X" IS THE DISTANCE FROM FINISHED FIRST FLOOR, UNLESS NOTED OTHERWISE.

SIZE (STUDS) / CAMBER — STEEL BEAM.

BEAM BOTTOM FLANGE BRACING. ARROW POINTS TOWARDS BOTTOM FLANGE OF BEAM BEING BRACED. SEE TYPICAL **BEAM BOTTOM FLANGE BRACING** DETAIL 19 / S1.03

STEEL OR COMPOSITE DECK OPENING. SEE TYPICAL **SMALL/LARGE OPENINGS IN STEEL DECK FOR FLOOR OR ROOF DECK** 5 / S1.07 6 / S1.07 DETAIL, U.N.O. VERIFY ALL OPENINGS WITH ARCHITECTURAL AND MECHANICAL PLANS.

STEEL DECK. INDICATES DIRECTION OF DECKING. SEE TYPICAL **STEEL ROOF DECK NOTES** 2 / S1.07 AND TYPICAL **DECK ATTACHMENT LAYOUT** 3 / S1.07 DETAILS.

3 STEEL LEGEND
S3.03 1/4" = 1'-0"

20 BUILDING B - HIGH ROOF FRAMING PLAN
S4.04 S3.03 1/8" = 1'-0"

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Revision:	Revision Description:	Rev. Date:
1	ADDENDUM #2	08/16/18
2	ADDENDUM #4	
3	ADDENDUM #6	09/12/18

Rev. Date: Revision: Revision Description:

Sheet Title: B HIGH ROOF FRAMING PLAN

Project Name & Address: NEW E.S. INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 9-07-18
Date:
Designer:
Designer
DR:
PC:

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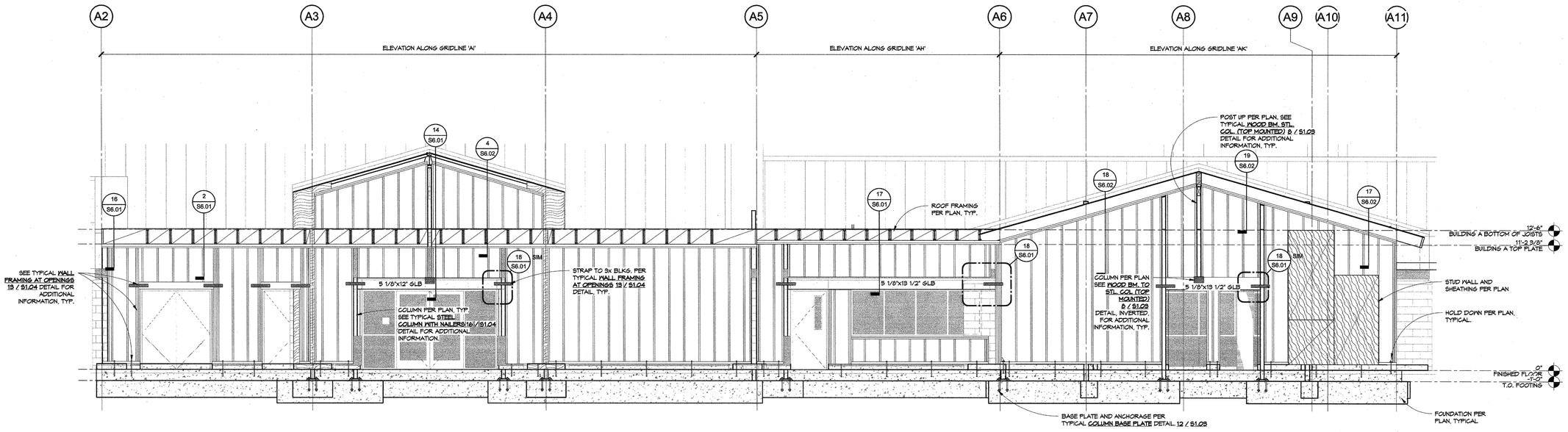
STRUCTURAL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711
17100

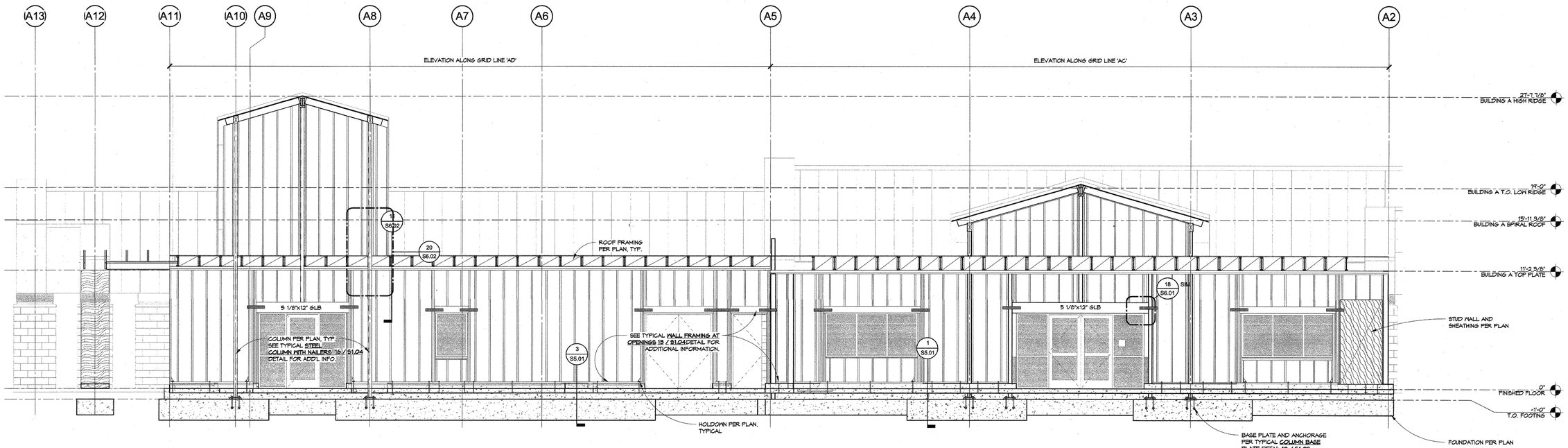
Job No.:

Sheet No.:
S3.03

Release: ADDENDUM NO. 6



18	ELEVATION
S2.01 | S4.01	1/4" = 1'-0"



20	ELEVATION
S2.01 | S4.01	1/4" = 1'-0"

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Revision:	Revision Description:	Rev. Date:	Revision:	Revision Description:	Rev. Date:

Sheet Title: A SHEARWALL ELEVATIONS

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
Date:
Designer: Designer
DR:
PC:

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RAYLAND R. RANSOM
No. S5355
EXP. 12-31-19
STRUCTURAL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711

Job No.:

Sheet No.: S4.01

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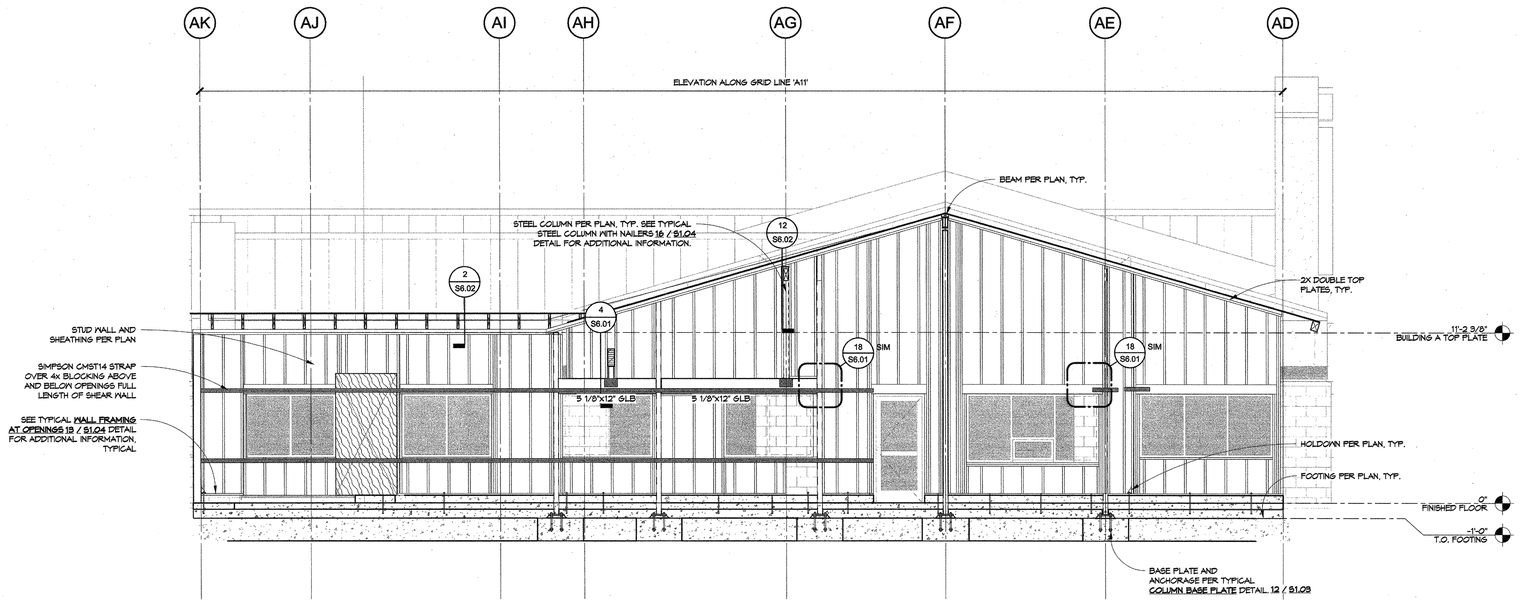
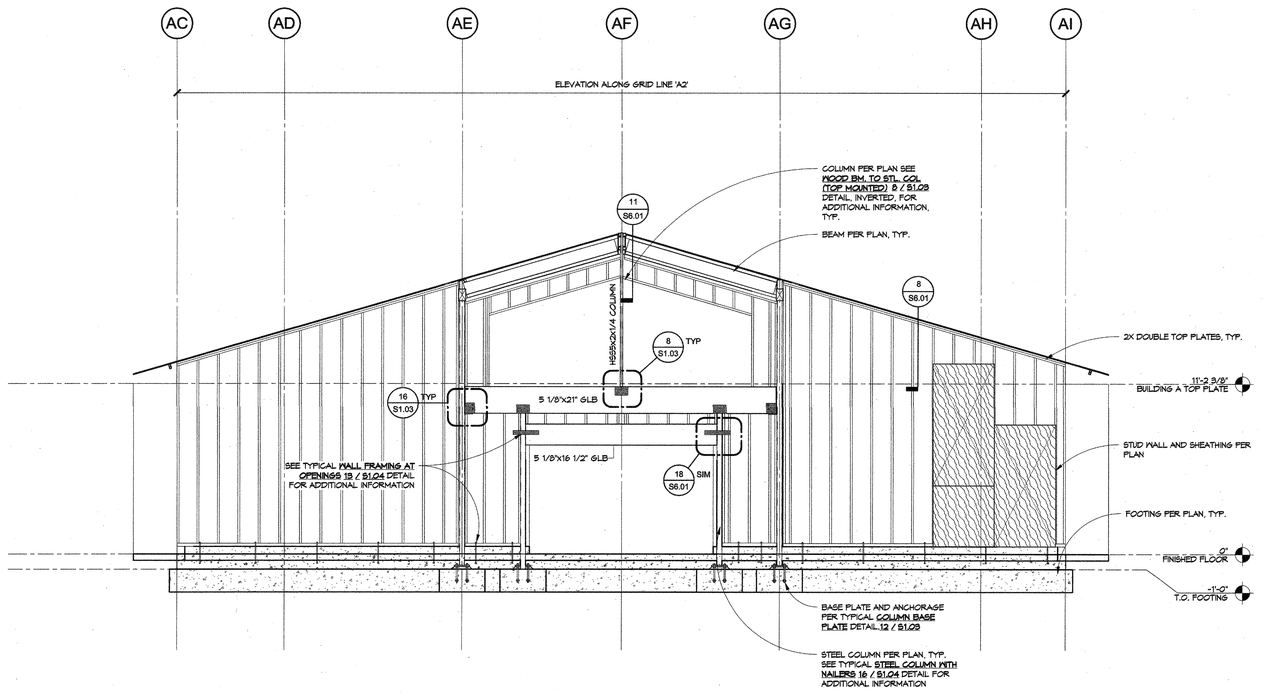


18 | ELEVATION

S2.01 | S4.02 | 1/4" = 1'-0"



12 | ELEVATION

S2.01 | S4.02 | 1/4" = 1'-0"

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Revision:	Revision Description:	Rev. Date:

Sheet Title: A SHEARWALL ELEVATIONS

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

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BROOKS-RANSOM ASSOCIATES
Structural Engineers
Civil Engineers
7415 N. Palm Ave., Suite 100
Fresno, California 93711

17800.

Job No.:

Sheet No.: S4.02

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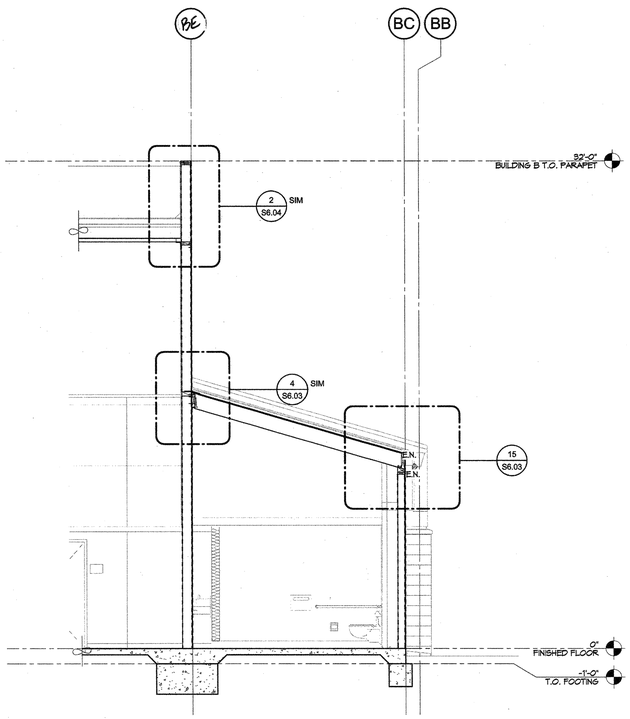

18	WALL SECTION		
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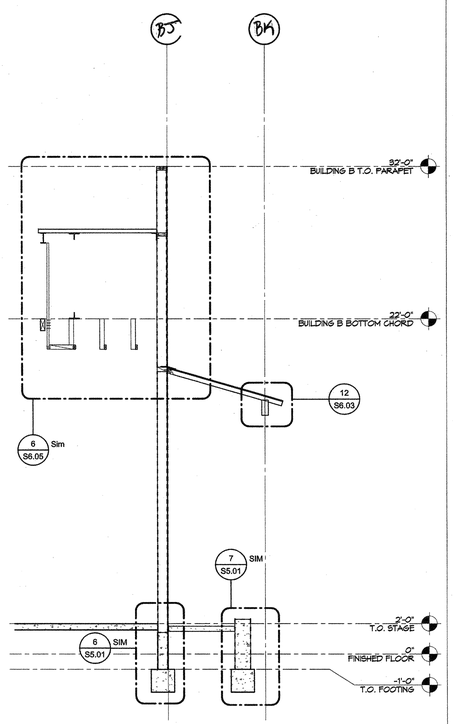

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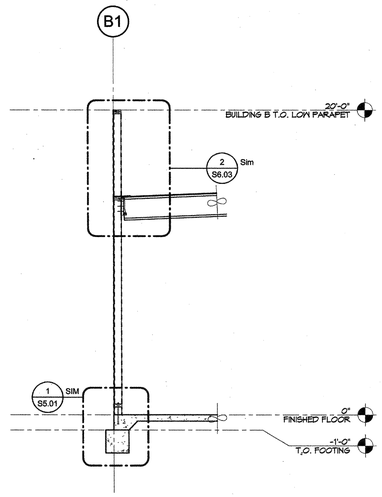
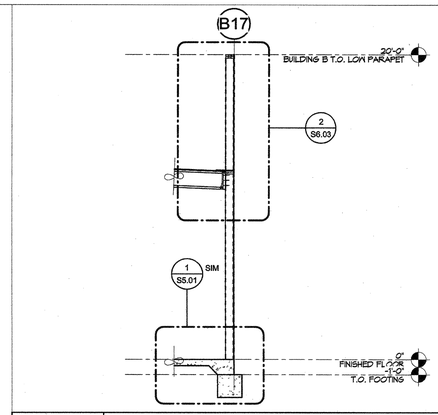
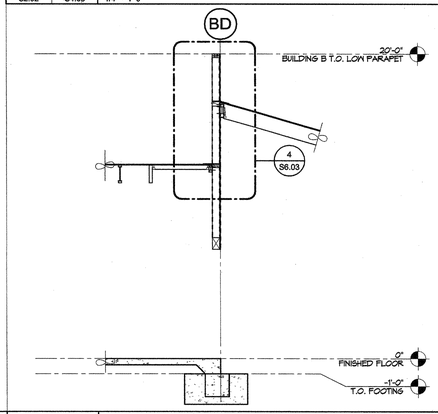
S2.02	S4.03	1/4" = 1'-0"	



6	WALL SECTION		
S2.02	S4.03	1/4" = 1'-0"	



1	WALL SECTION		
S2.02	S4.03	1/4" = 1'-0"	



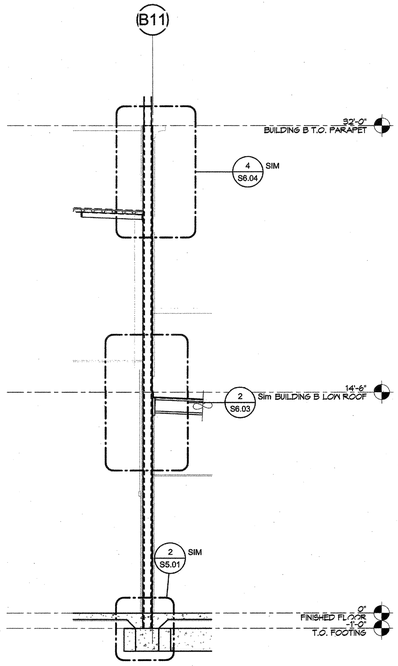
2	WALL SECTION		
S2.02	S4.03	1/4" = 1'-0"	

19	NOT USED
	NOT TO SCALE

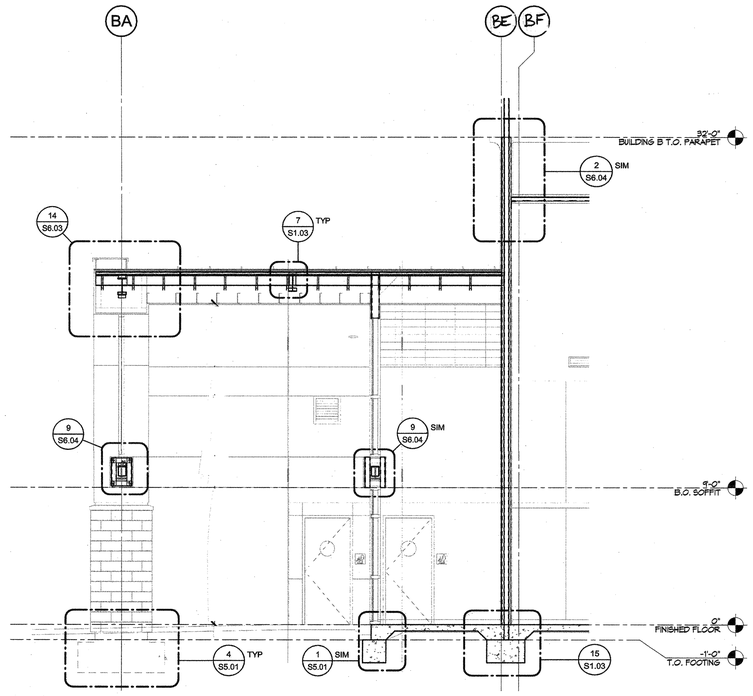
15	NOT USED
	NOT TO SCALE

20	NOT USED
	NOT TO SCALE

16	NOT USED
	NOT TO SCALE



12	WALL SECTION		
S2.02	S4.03	1/4" = 1'-0"	



8	WALL SECTION		
S2.02	S4.03	1/4" = 1'-0"	

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Sheet Title: B WALL SECTIONS

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
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Designer: Designer
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REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RANSOM
No. 52186
EXPIRES 12-31-20
STRUCTURAL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711

Job No.:

Sheet No.:
S4.03

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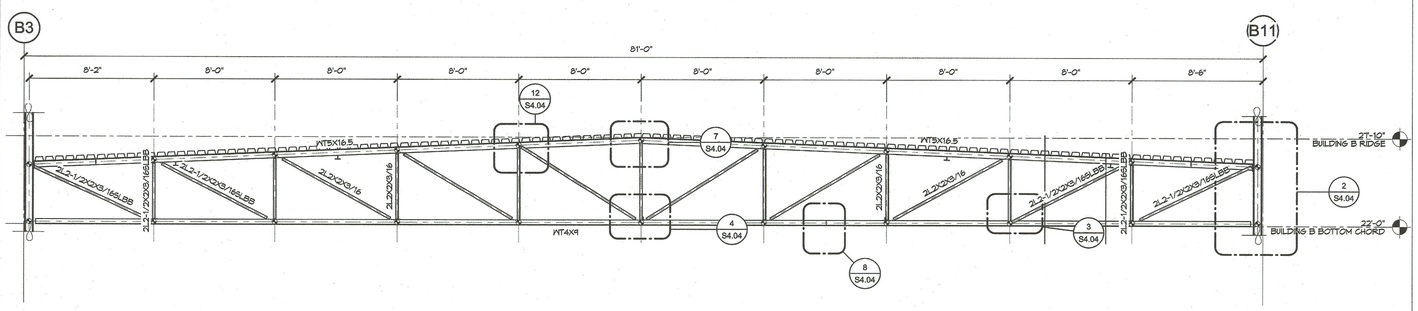


17 TYPICAL TRUSS ELEVATION

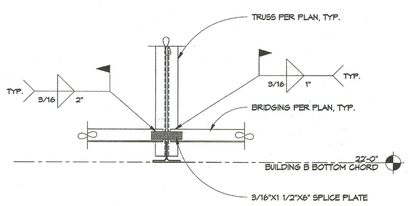
S3.02 | S4.04 | 1/4" = 1'-0"

009 Truss Type

Contractor: Kern Steel (1)Question:Please review attached.We are providing truss type per elevation 17/S4.04 at locations highlighted in pink and 18/S4.04 at locations highlighted in blue. Please confirm this is correct.

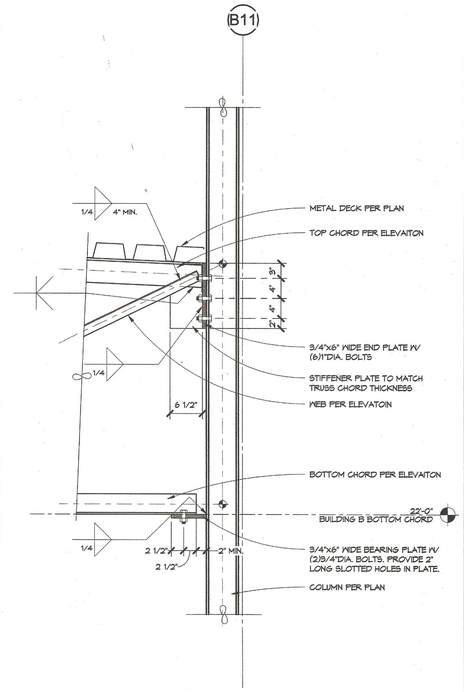
The above stated and highlighted attachment is correct.

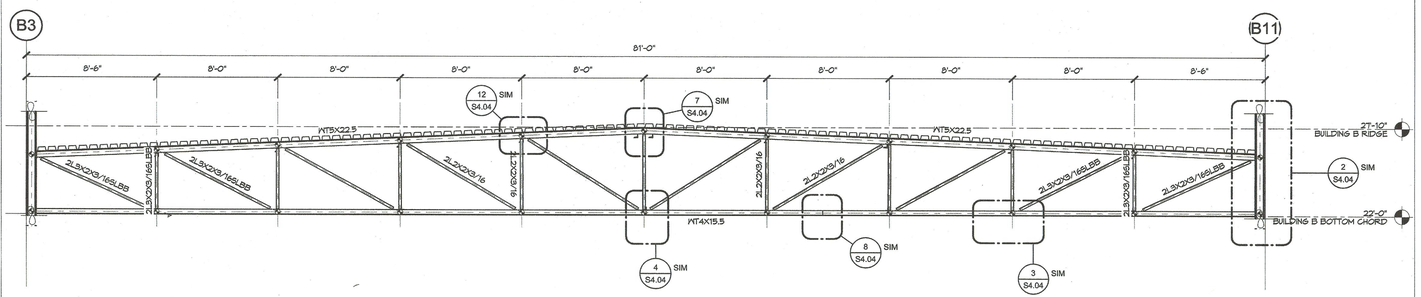
Closed



5 DETAIL

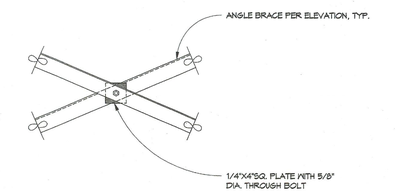
S4.04 | S4.04 | 1" = 1'-0"





18 TRUSS ELEVATION OVER STAGE

S3.02 | S4.04 | 1/4" = 1'-0"

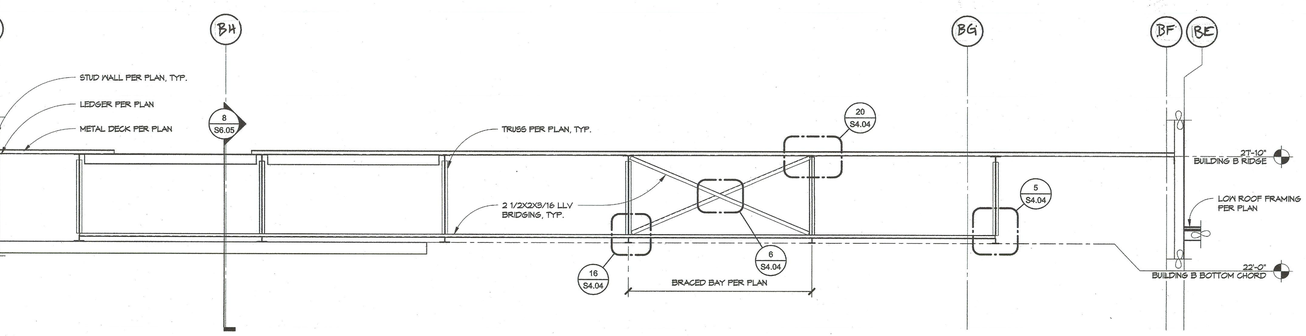


6 DETAIL

S4.04 | S4.04 | 1" = 1'-0"

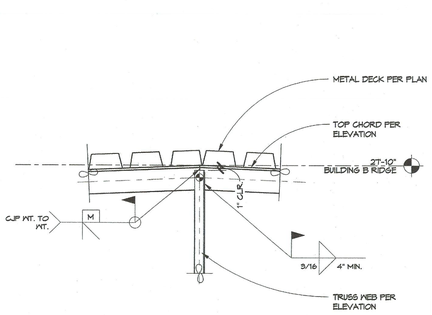
2 DETAIL

S4.04 | S4.04 | 1" = 1'-0"



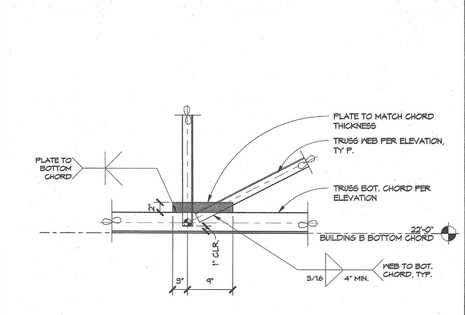
19 TRUSS BRIDGING ELEVATION

S3.02 | S4.04 | 1/4" = 1'-0"



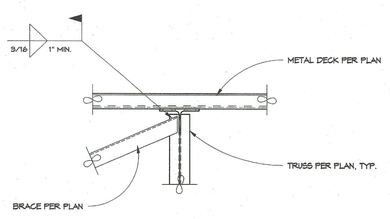
7 DETAIL

S4.03 | S4.04 | 1" = 1'-0"



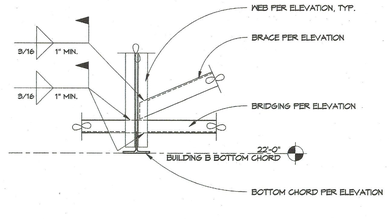
3 DETAIL

S4.04 | S4.04 | 1" = 1'-0"



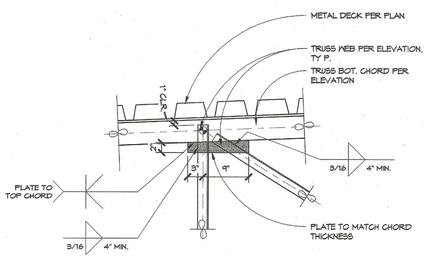
20 DETAIL

S4.04 | S4.04 | 1" = 1'-0"



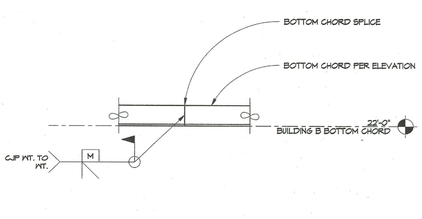
16 DETAIL

S4.04 | S4.04 | 1" = 1'-0"



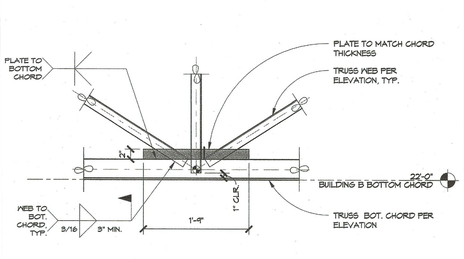
12 DETAIL

S4.04 | S4.04 | 1" = 1'-0"



8 DETAIL

S4.04 | S4.04 | 1" = 1'-0"



4 DETAIL

S4.04 | S4.04 | 1" = 1'-0"

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Revision: | Revision Description: | Rev. Date:
Revision: | Revision Description: | Rev. Date:

Sheet Title: B TRUSS ELEVATION AND DETAILS

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18 | Date: | Designer: Designer | DR: | PIC:

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GAYLORD R. RANSOM
No. S3886
CIVIL
STRUCTURAL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
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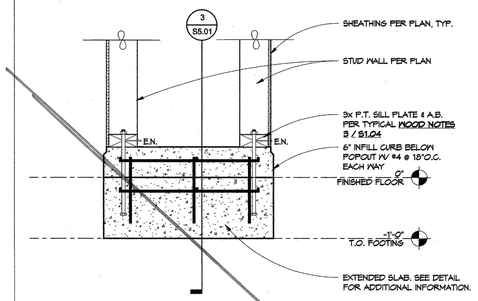
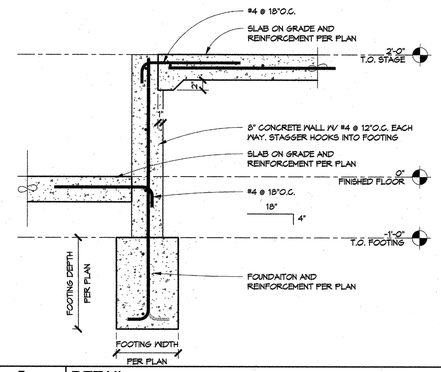
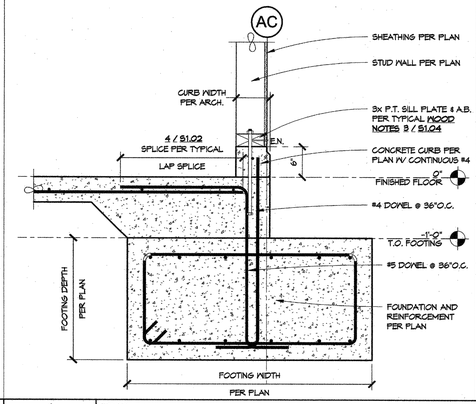
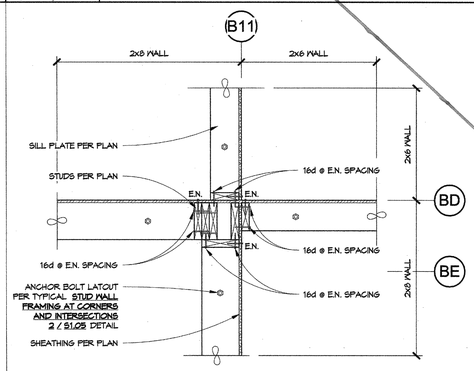
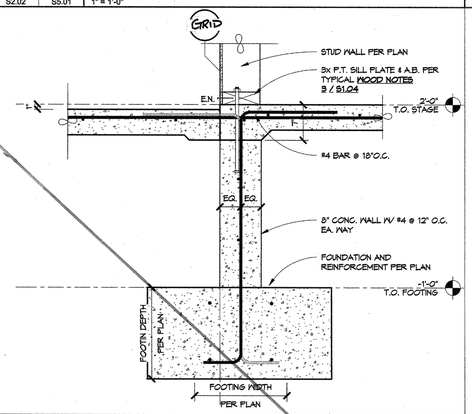
17	NOT USED NOT TO SCALE	13	NOT USED NOT TO SCALE	9	DETAIL S5.01 S5.01 1" = 1'-0"	5	DETAIL S2.02 S5.01 1" = 1'-0"	1	DETAIL S2.01 S5.01 1" = 1'-0"
18	NOT USED NOT TO SCALE	14	NOT USED NOT TO SCALE	10	DETAIL S2.02 S5.01 1" = 1'-0"	6	DETAIL S2.02 S5.01 1" = 1'-0"	2	DETAIL S2.01 S5.01 1" = 1'-0"
19	NOT USED NOT TO SCALE	15	NOT USED NOT TO SCALE	11	DETAIL NTS	7	DETAIL S2.02 S5.01 1" = 1'-0"	3	DETAIL S2.01 S5.01 1" = 1'-0"
20	NOT USED NOT TO SCALE	16	NOT USED NOT TO SCALE			8	DETAIL S2.01 S5.01 1" = 1'-0"	4	DETAIL S2.01 S5.01 1" = 1'-0"

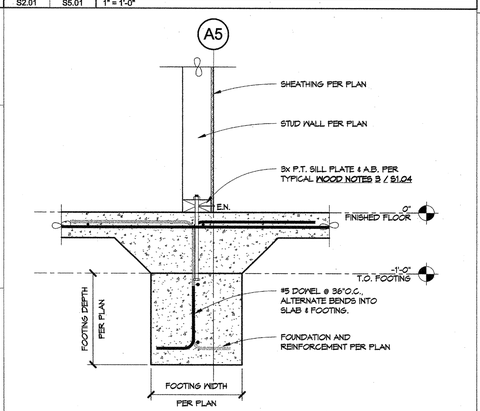


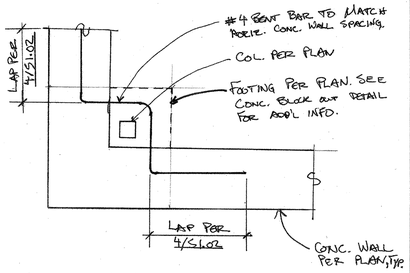


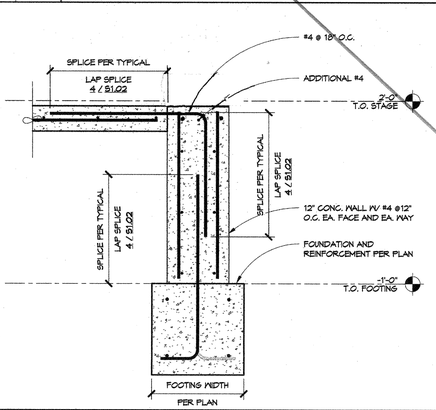


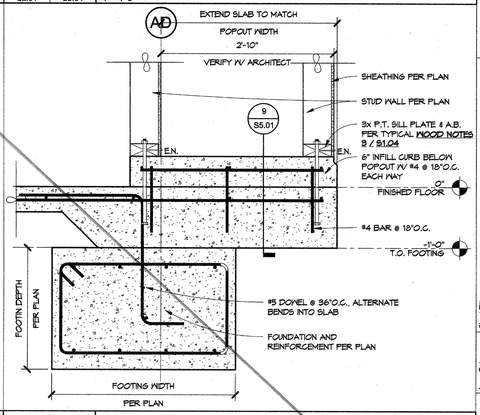


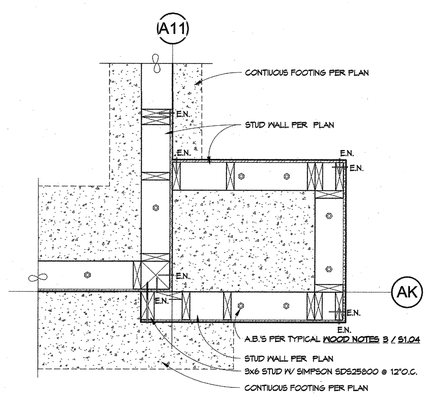


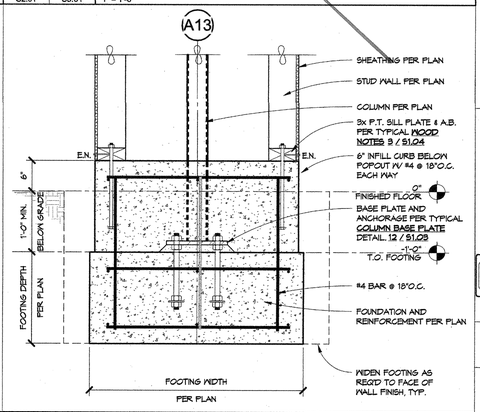












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2727 Fresno St., Fresno, CA 93721
Phone (559) 435-0851 Fax (559) 435-0855
E-Mail: design @somam.com
www.integrateddesigns.com

Revision:	Revision Description:	Rev. Date:

Sheet Title: FOUNDATION DETAILS

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
Date:
Designer: Designer
DR:
PC:

Agency Approval Stamp:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC. ✓ FLS. ✓ SS. KH
DATE 06-22-18

TRACKING #: 63321-300

Stamp(s)

REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RANSOM
No. 52788
CIVIL
STRUCTURAL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 150
FRESNO, CALIFORNIA 93711
17600

Job No.:

Sheet No.: S5.01

Release:

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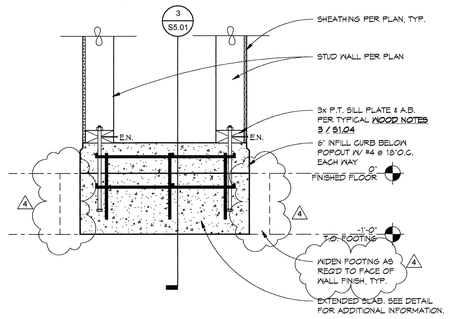
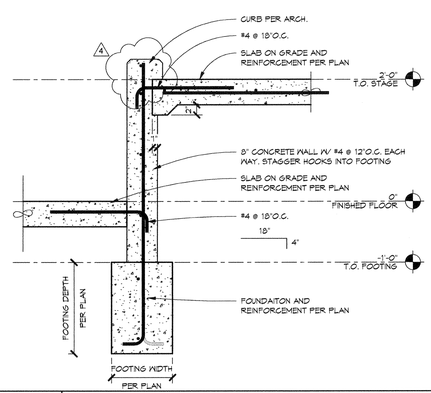
NOT TO SCALE

13 | NOT USED
NOT TO SCALE



9 | DETAIL
S5.01 | S5.01 | 1" = 1'-0"



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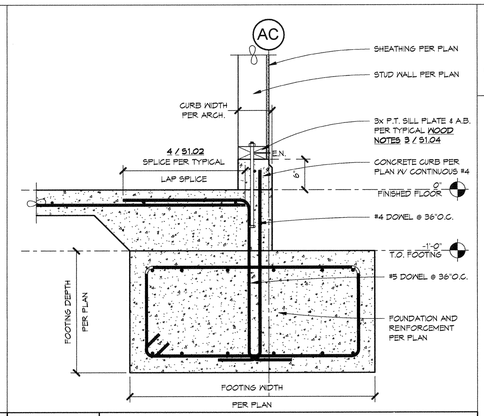

1 | DETAIL
S2.01 | S5.01 | 1" = 1'-0"

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NOT TO SCALE

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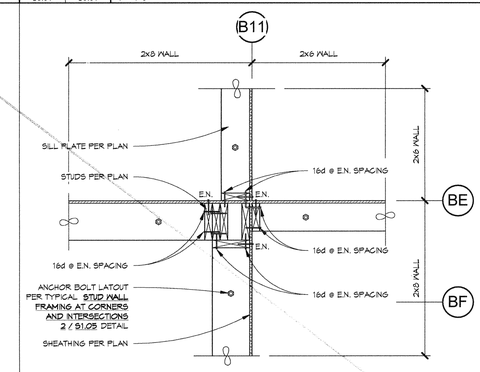
NOT TO SCALE



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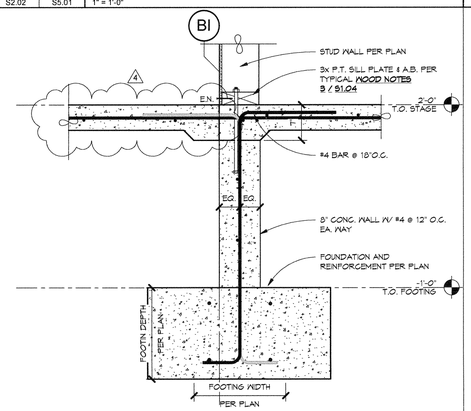

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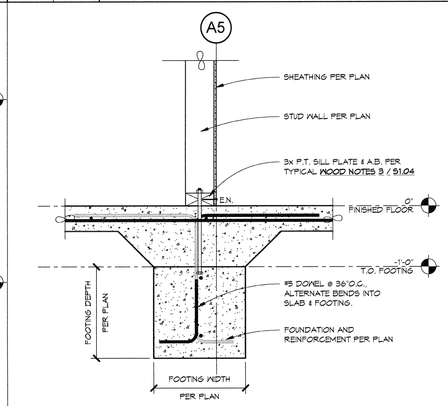

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S2.01 | S5.01 | 1" = 1'-0"

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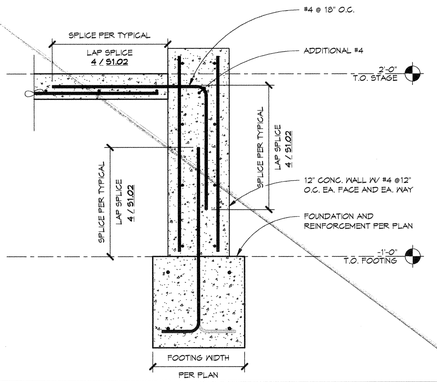
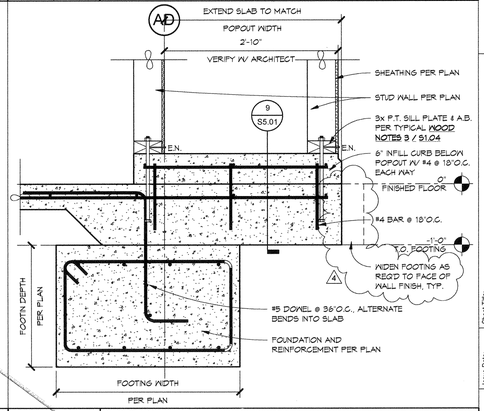
NOT TO SCALE



7 | DETAIL
S2.02 | S5.01 | 1" = 1'-0"



3 | DETAIL
S2.01 | S5.01 | 1" = 1'-0"

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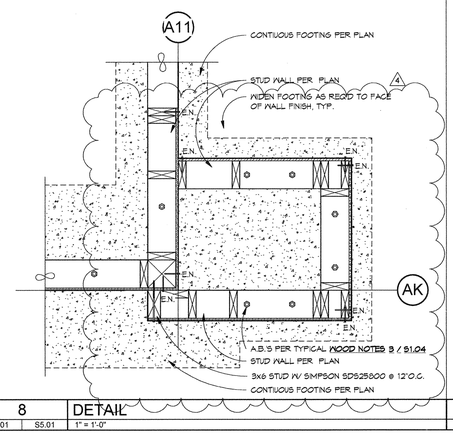
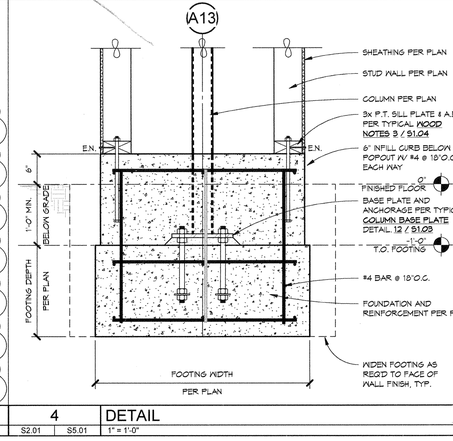
NOT TO SCALE

16 | NOT USED
NOT TO SCALE



8 | DETAIL
S2.01 | S5.01 | 1" = 1'-0"



4 | DETAIL
S2.01 | S5.01 | 1" = 1'-0"

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6011 N. Fresno, Suite 130 - Fresno, California 93710
Phone (559) 439-0881 Fax (559) 439-0888 E-Mail: design @somam.com
www.integrateddesign.com

Revision:	Revision Description:	Rev. Date:
1	ADDENDUM #2	08/16/18
2	ADDENDUM #4	09-07-18

Sheet Title: FOUNDATION DETAILS

Project Name & Address: NEW E.S. INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 9-07-18
Date:
Designer: Designer
DR:
PC:

Agency Approval Stamp:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC_____FLS_____SS_____
DATE_____

TRACKING #: 63321-300

Stamp(s)

REGISTERED PROFESSIONAL ENGINEER
CLIFFORD R. RANSOM
No. 52386
EXPIRES
3-31-19
CIVIL

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711

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Sheet No.: S5.01

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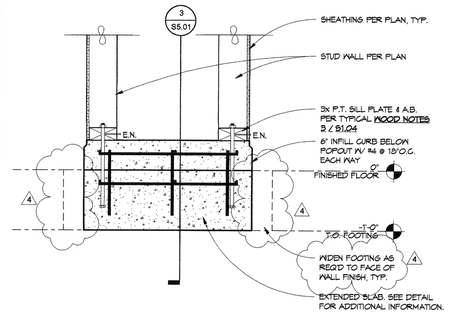

17	NOT USED	13	NOT USED	9	DETAIL
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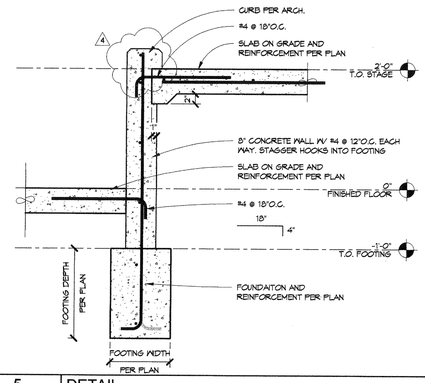

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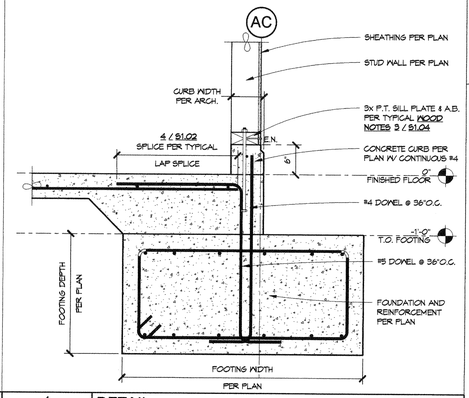
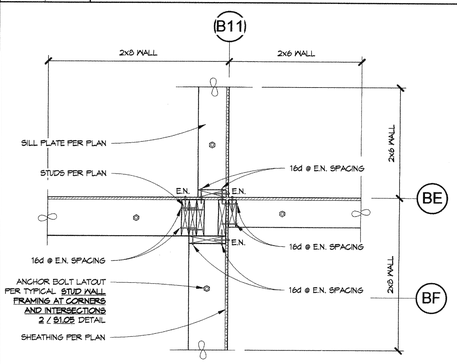


1	DETAIL
S2.01 S5.01	1" = 1'-0"



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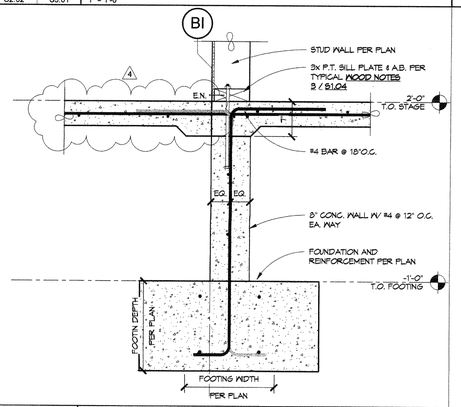
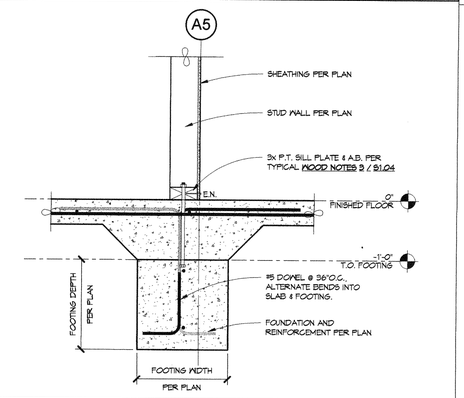


6	DETAIL
S2.02 S5.01	1" = 1'-0"



2	DETAIL
S2.01 S5.01	1" = 1'-0"

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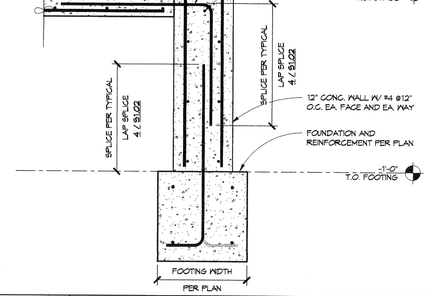
	NOT TO SCALE		NOT TO SCALE



7	DETAIL
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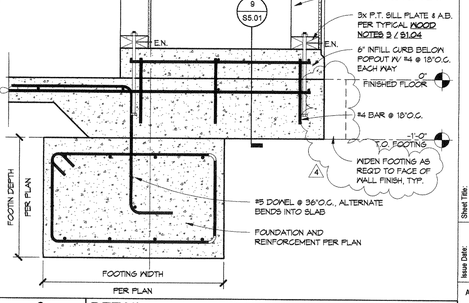

3	DETAIL
S2.01 S5.01	1" = 1'-0"

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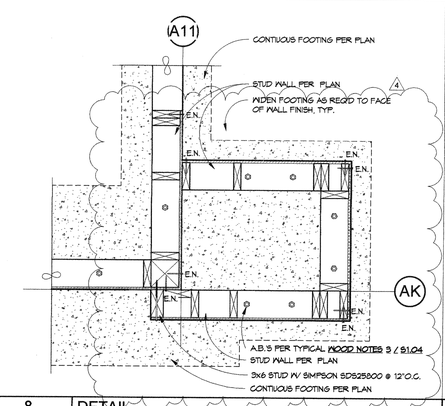
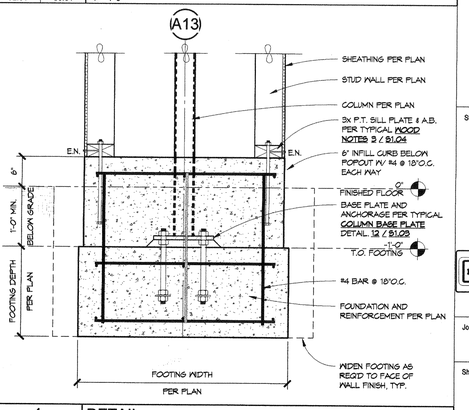


8	DETAIL
S2.01 S5.01	1" = 1'-0"



4	DETAIL
S2.01 S5.01	1" = 1'-0"

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Phone: (559) 436-0201 Fax: (559) 436-0207 E-Mail: design@somam.com
www.integrateddesigns.com

Revision:	Revision Description:	Rev. Date:
1	ADDENDUM #2	08/16/18
2	ADDENDUM #4	09-07-18

Sheet Title: FOUNDATION DETAILS

Project Name & Address:
NEW E.S. INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 9-07-18
Date:
Designer: Designer
DR:
PC:

Agency Approval Stamp:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC SC FLS SK ss TN
DATE 5/16/19

TRACKING #: 63321-300
ADDENDUM #4

Stamp(s)

REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RANDOLPH
No. 52386
EXPIRES 3-31-20
CIVIL

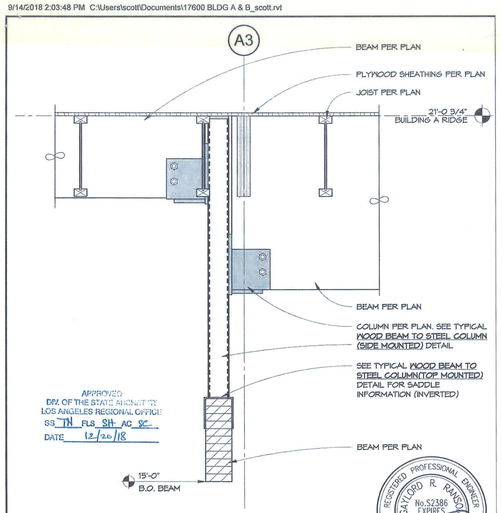
BROOKS-RANDOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 105
FRESNO, CALIFORNIA 93711
T 7000

Job No.:

Sheet No.:
S5.01

Release: ADDENDUM NO. 4

9/14/2018 2:03:48 PM C:\Users\scott\Documents\17600 BLDG A & B_scott.rvt



APPROVED
DIV. OF THE STATE ARCHITECT
LOS ANGELES REGIONAL OFFICE
SS TN FLS SH AC SC
DATE 12/20/18

REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RAMSEY
No. 52386
EXPIRES 3-31-20
CIVIL
STATE OF CALIFORNIA

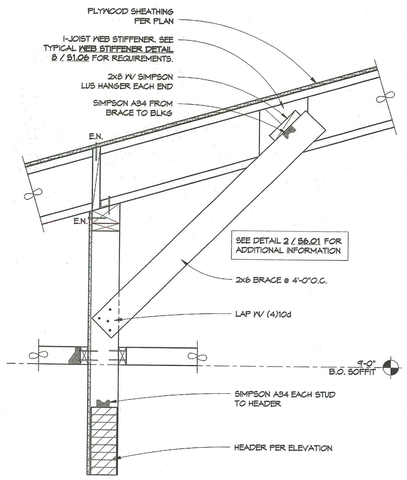
6	DETAIL
S6.01 AD6-S2	1" = 1'-0"

BROCKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS · CIVIL ENGINEERS
7415 N. PALM AVENUE, SUITE 100
FRESNO, CALIFORNIA 93711
B:(559)449-9444 · F:(559)449-9404

NEW E.S. INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

DRAWN BY: Author
DATE: 9-07-18
PROJ.#: 17600

SHEET: AD6-S2



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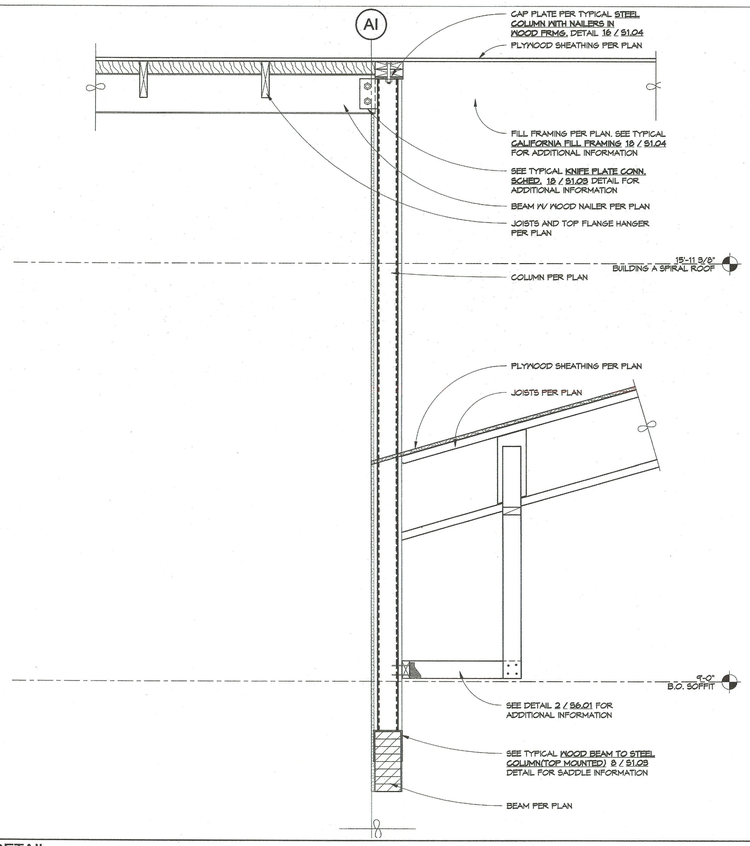

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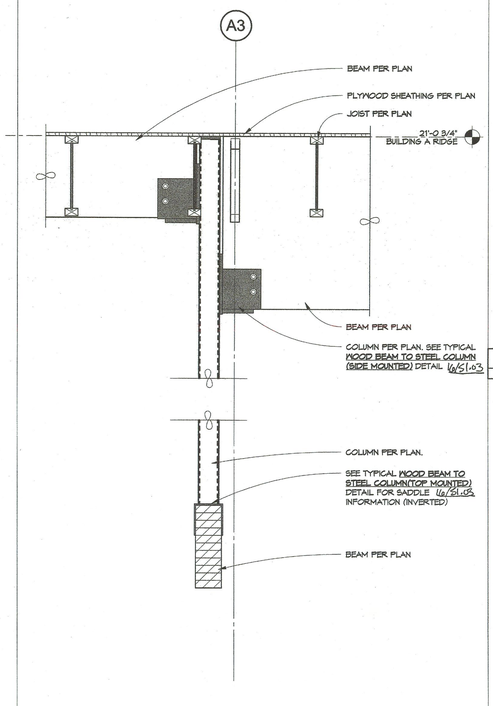

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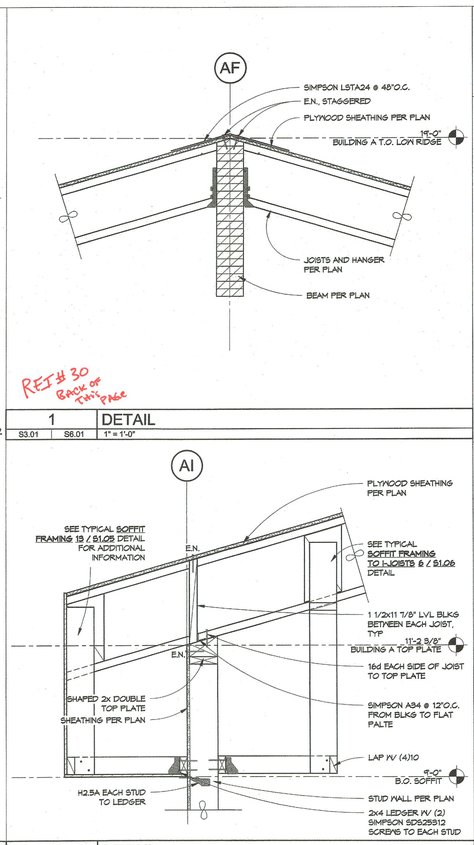
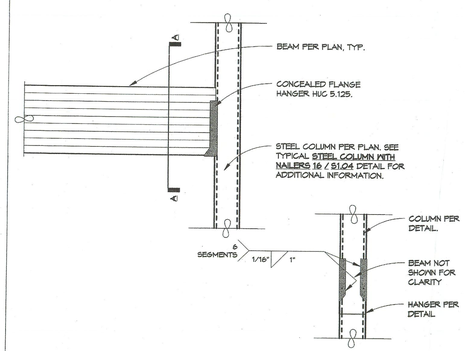


1 | DETAIL
S3.01 | S6.01 | 1" = 1'-0"

2 | DETAIL
S3.01 | S6.01 | 1" = 1'-0"



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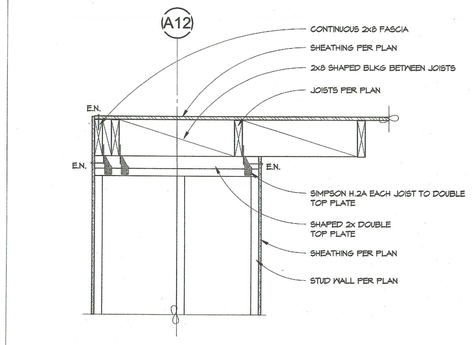

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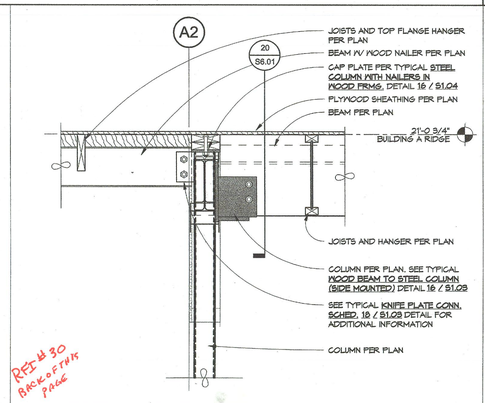

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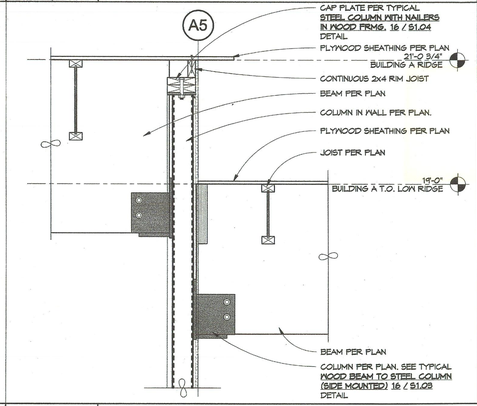

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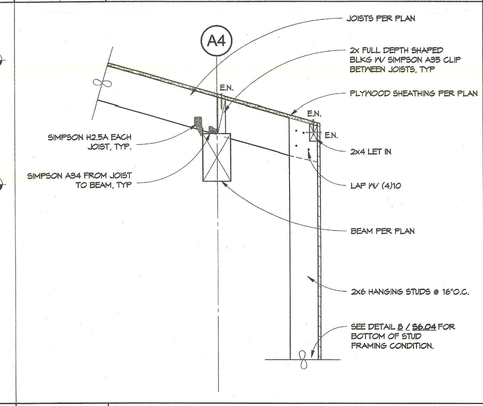
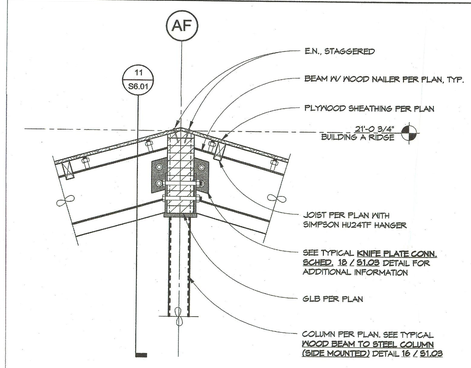
S3.01 | S6.01 | 1" = 1'-0"



3 | DETAIL
S3.01 | S6.01 | 1" = 1'-0"



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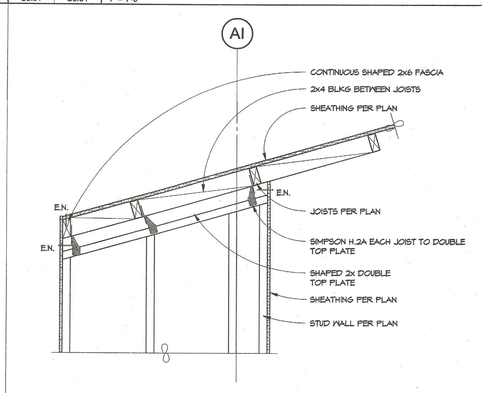

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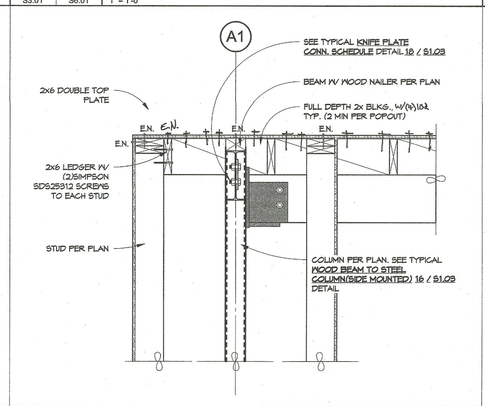

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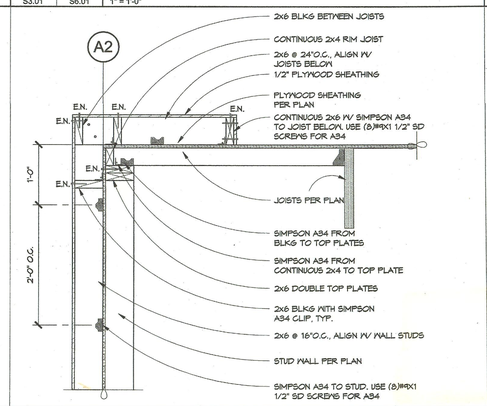

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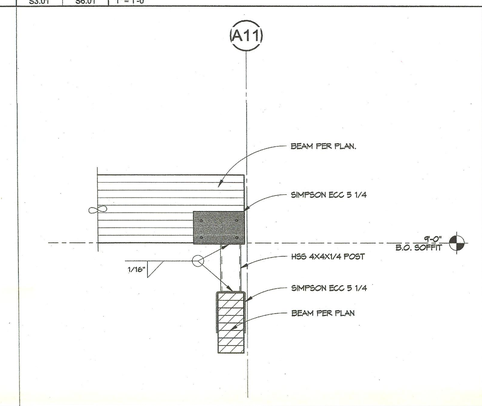

4 | DETAIL
S3.01 | S6.01 | 1" = 1'-0"

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Phone: (559) 439-0881 Fax: (559) 436-0887 E-Mail: design@somam.com
www.integrateddesigns.com

Revision: | Revision Description: | Rev. Date:
Revision: | Revision Description: | Rev. Date:

Sheet Title: A FRAMING DETAILS

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
Date:
Designer: Designer
DR:
PC:

Agency Approval Stamp:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC. / FLS. / SS
DATE 06-22-18

TRACKING #: 63321-300

Stamp(s)

REGISTERED PROFESSIONAL ENGINEER GALORD R. RANSOM No. S3855 PROFESSIONAL STRUCTURAL STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711

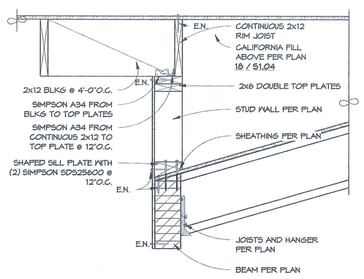
Job No.:

Sheet No.: S6.01

Release:

0030	Top Plate Elevation	Kern Steel (11) Please reference 1 thru 3. By following the bottom of sheathing elevation per structural section 1,11/S6.01 and 3-1/2" per foot slope, we arrive top plate elevationas shown in page 2 which is below ridge beam along GL AC and AL and AD and AH. If top plate elevation needs to be fixed at 11'-2 1/2" per approvercomments please advise which one of below options needs to be followed.1. bottom of sheathing per 1,11/S6.01.Or please advise if we shall fix top plate by following the bottom of sheathing elevation and roof slope per design drawing as submitted for approval.	Disregard approvers clouded elevations. Original elevations provided by KSF are correct. Approver took elevations to incorrect plate.	Closed

9/14/2018 11:14:39 AM C:\Users\scott\Documents\17600 BLDG A & B_scott.rvt



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DIV. OF THE STATE ARCHITECT
LOS ANGELES REGIONAL OFFICE
BY: TN FLS SH AC BT
DATE: 12/24/18

REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. RANSOM
No. S2386
EXPIRES 3-31-20
STATE OF CALIFORNIA

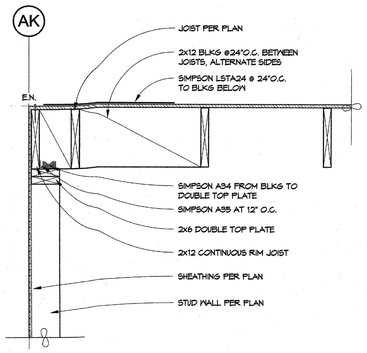
18	DETAIL
S6.02	AD6-S1
	1" = 1'-0"

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS · CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711
B/559/449-8444 · F/559/449-8404

NEW E.S. INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

DRAWN BY: Author
DATE: 9-07-18
PROJ. #: 17600

SHEET: AD6-S1



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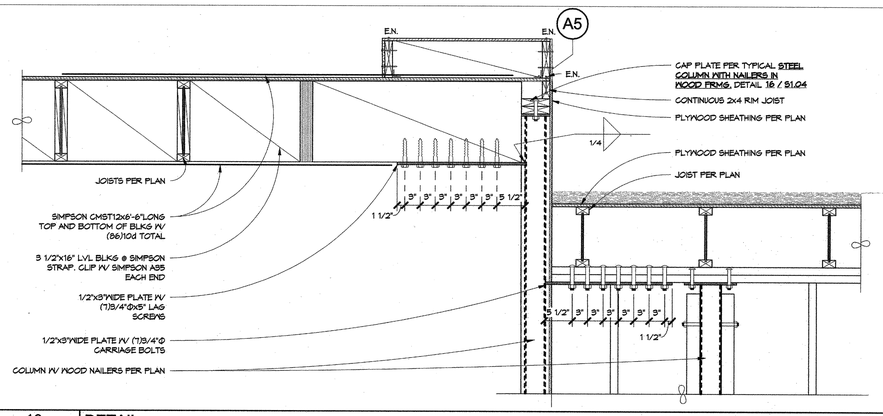

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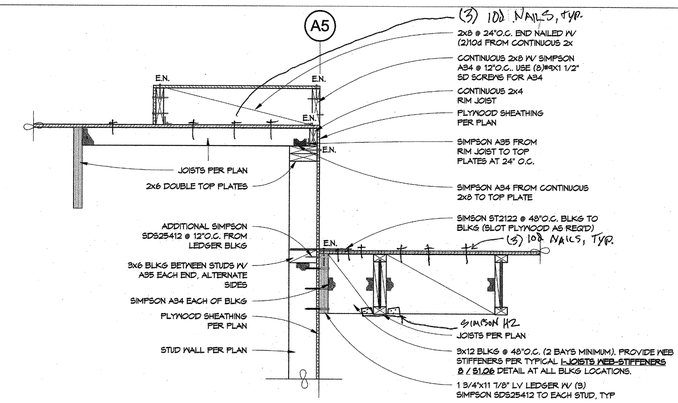

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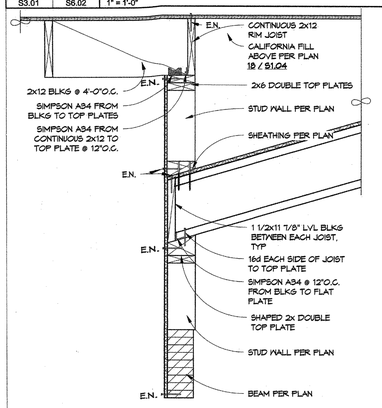

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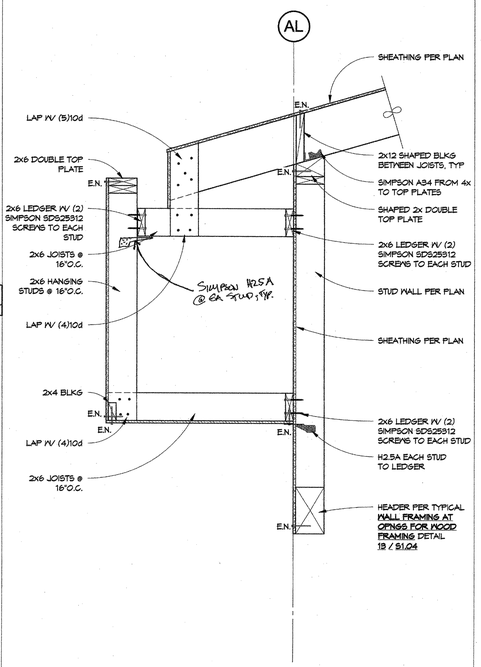

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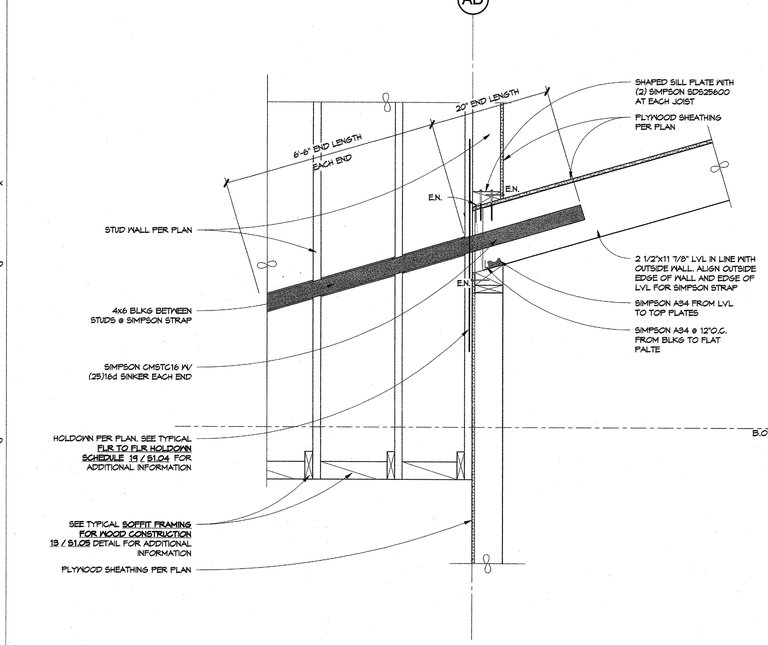

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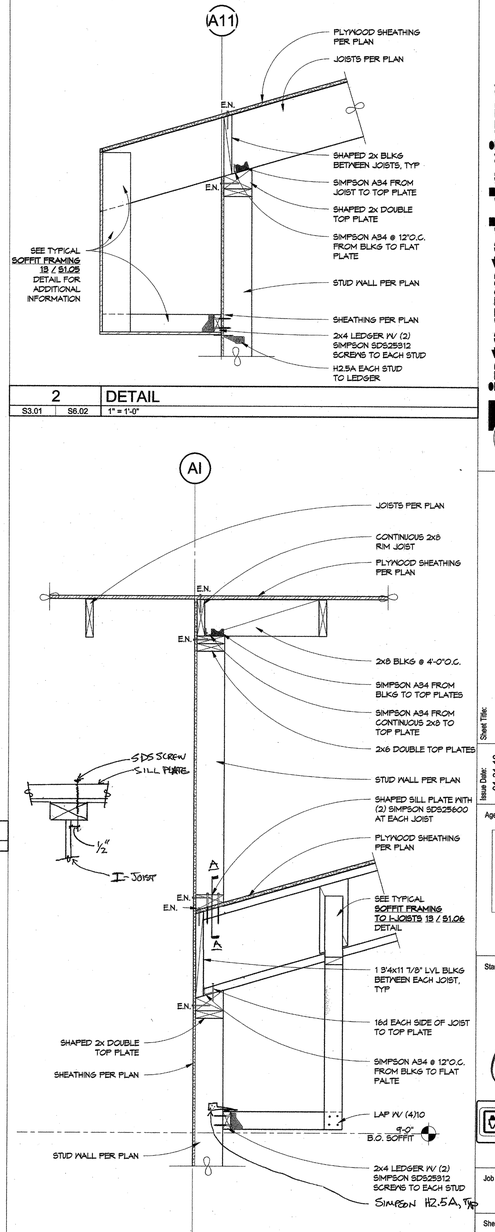

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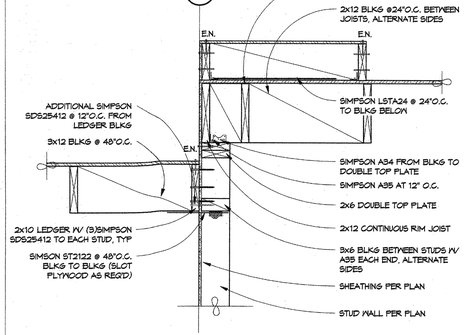

19	DETAIL	
S3.01	S6.02	1" = 1'-0"

16	DETAIL	
S3.01	S6.02	1" = 1'-0"

12	DETAIL	
S3.01	S6.02	1" = 1'-0"

4	DETAIL	
S3.01	S6.02	1" = 1'-0"

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Phone: (559) 490-0601 Fax: (559) 490-0602 Mobile: design@somam.com
www.integrateddesign.com

Revision:	Revision Description:	Rev. Date:

Sheet Title: A FRAMING DETAILS

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
Date:
Designer: Designer
DR:
PC:

Agency Approval Stamp:

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AC. FLS. SS
DATE 08-22-18
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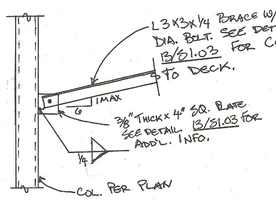
REGISTERED PROFESSIONAL ENGINEER
GALVORD R. RANSOM
No. 52386
EXPIRES 12-31-20
STRUCTURAL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711

Job No.:

Sheet No.: S6.02

Release:



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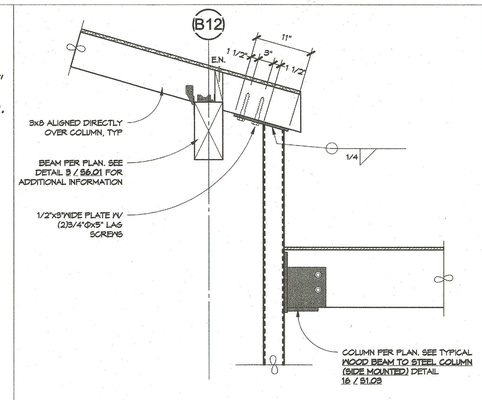

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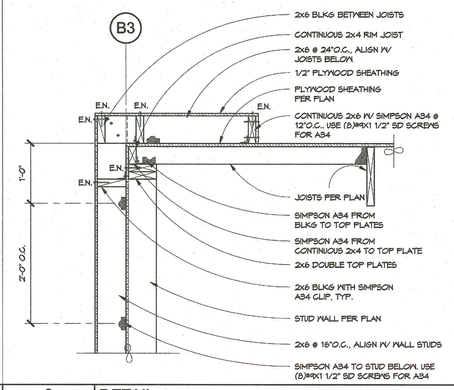

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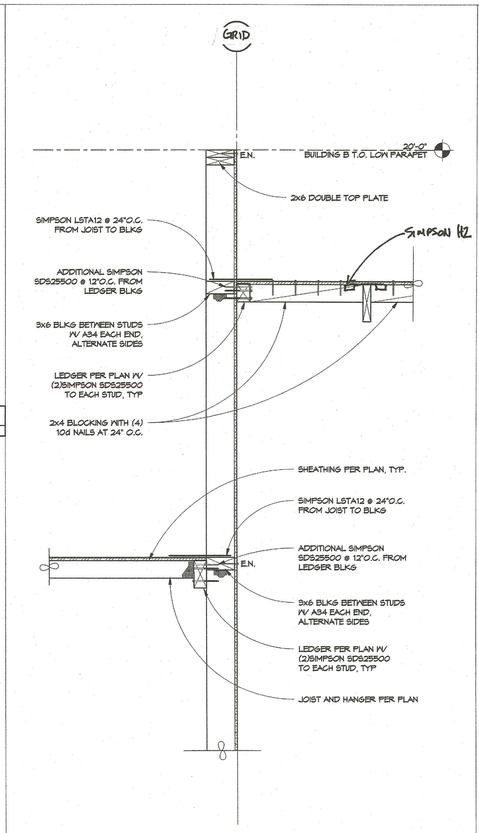

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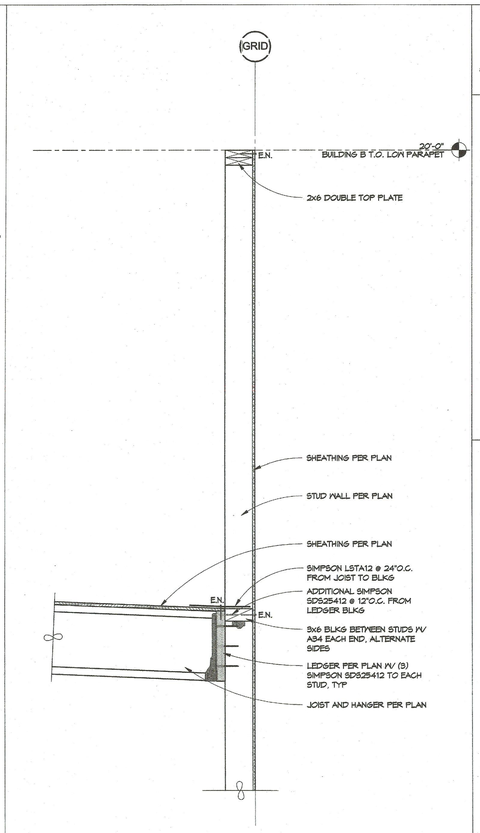

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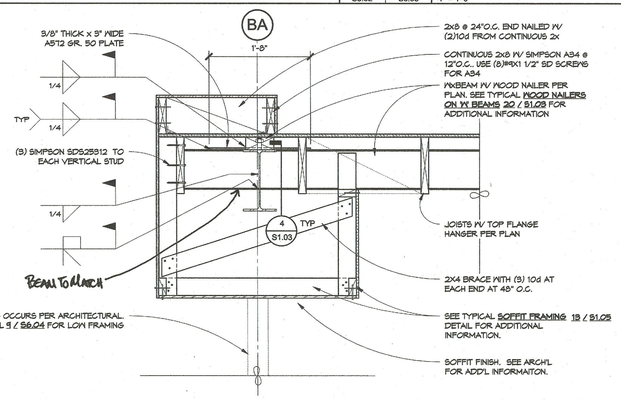

RFI # 28

14 DETAIL
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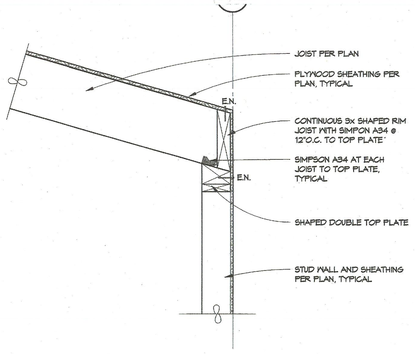

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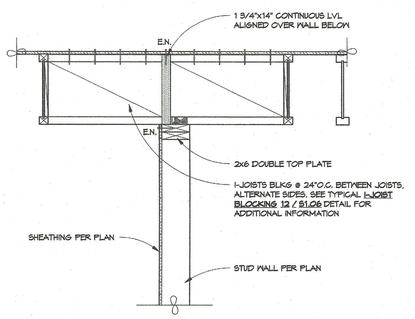

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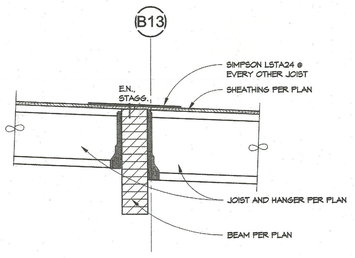

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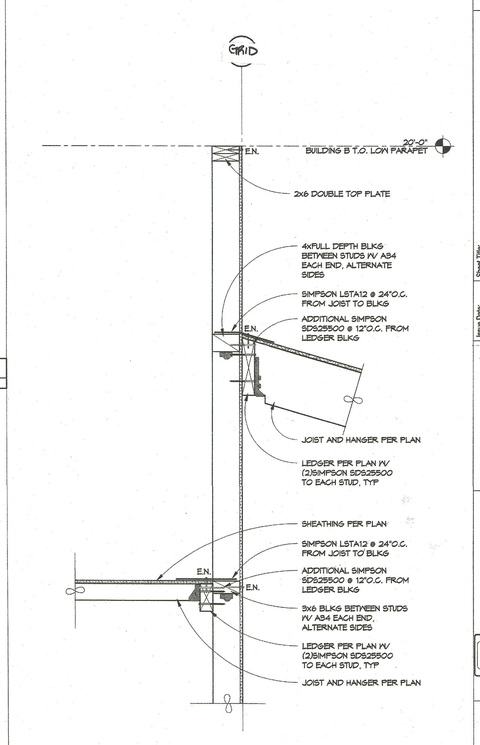

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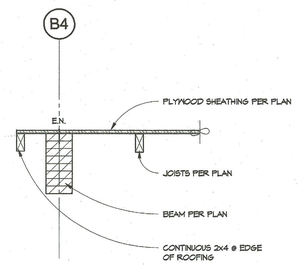
NOT TO SCALE



16 DETAIL
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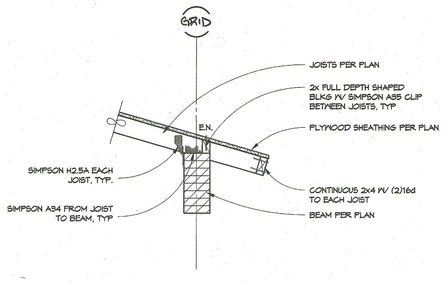

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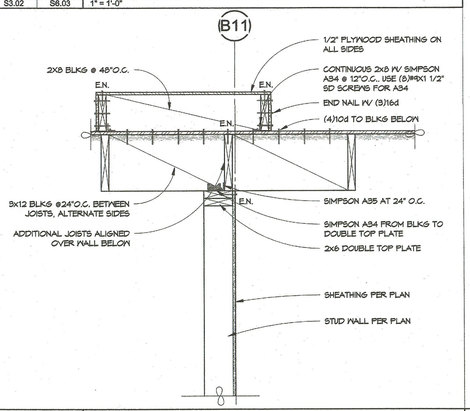

8 DETAIL
S3.02 S6.03 1" = 1'-0"

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Revision:	Revision Description:	Rev. Date:

Issue Date: 01-31-18
Date:
Designer: DR:
Designer PIC:

Sheet Title: B FRAMING DETAILS

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Agency Approval Stamp:
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OFFICE OF REGULATION SERVICES
03-118394
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DATE 08-22-18
TRACKING #: 63321-300

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GAYLORD R. RANSOM
No. S2136
EXPIRES 12-31-19
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STATE OF CALIFORNIA

BRODKIN-RANSOM ASSOCIATES
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CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711
17005

Job No.:

Sheet No.: S6.03

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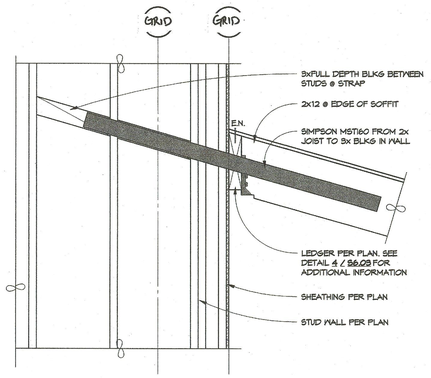

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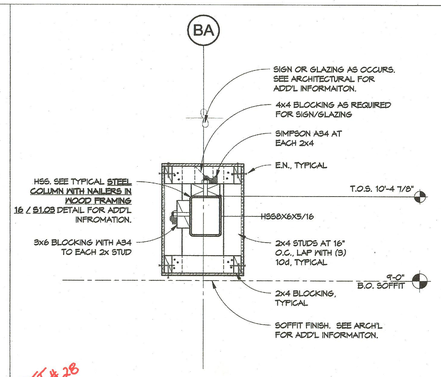

RFI # 28

13	DETAIL		
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9	DETAIL		
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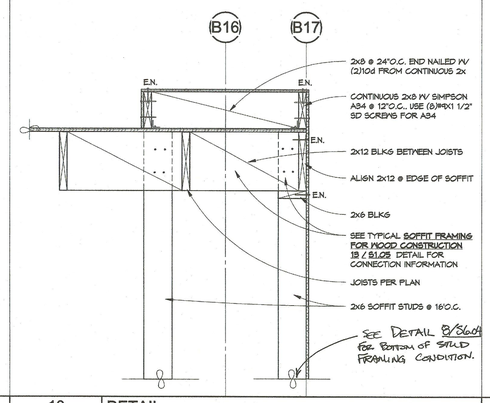

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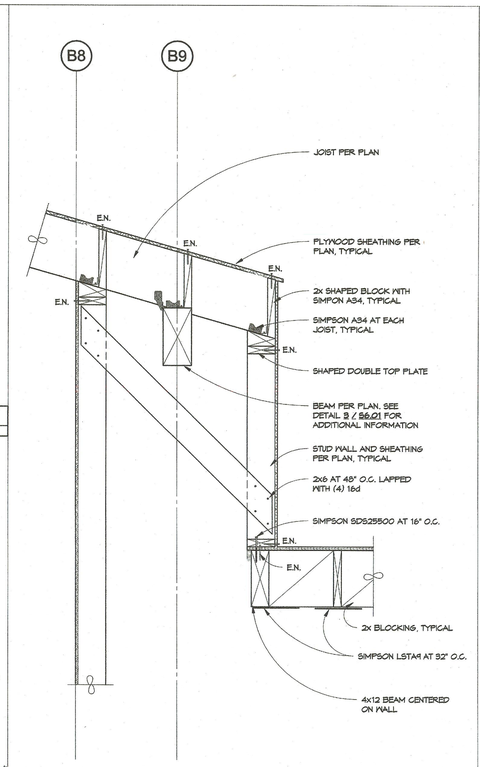

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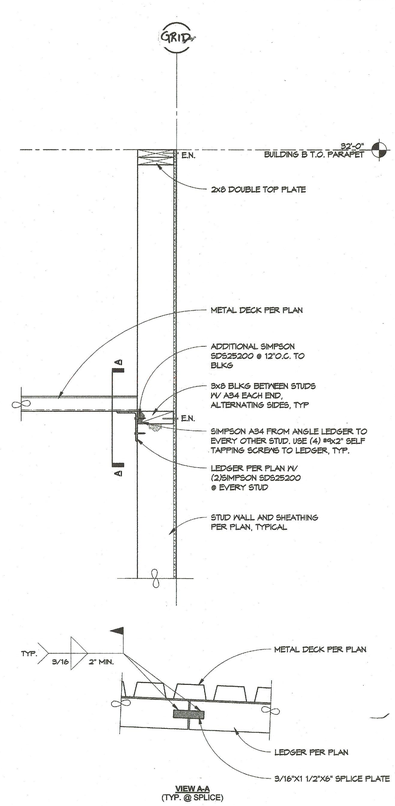

VIEW A-A
(TYP. @ SPLICE)

2	DETAIL		
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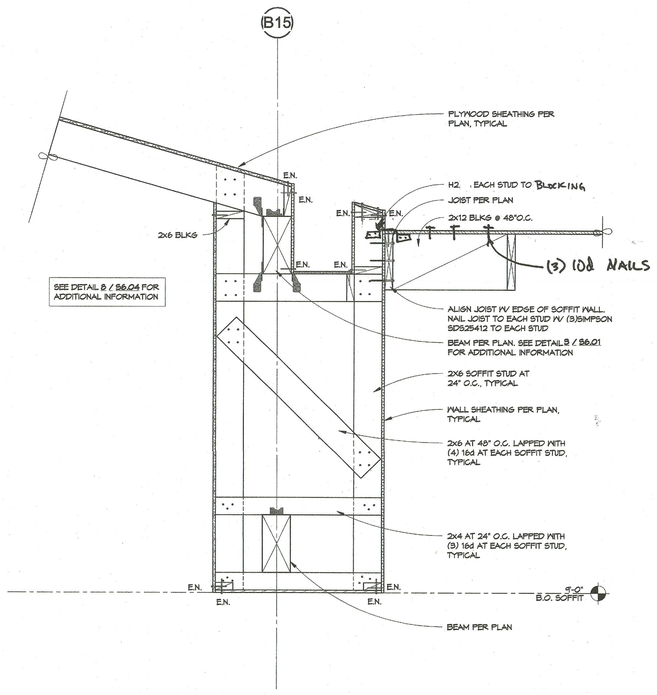

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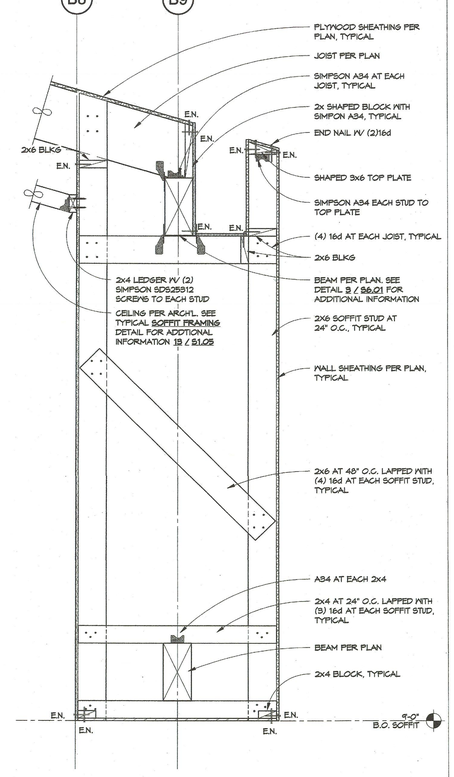

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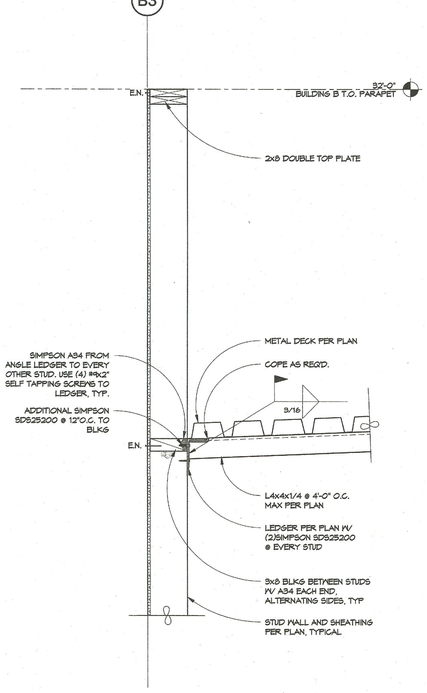

4	DETAIL		
S3.03	S6.04	1" = 1'-0"	

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Phone: (559) 436-0881 Fax: (559) 436-0887 E-Mail: design @somam.com
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Revision:	Revision Description:	Rev. Date:	Revision:	Revision Description:	Rev. Date:

Sheet Title: B FRAMING DETAILS

Project No: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

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01-31-18					

Agency Approval Stamp:

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AC ___ FLS ___ SS VH
DATE 06-22-18

TRACKING #: 63321-300

Stamp(s)

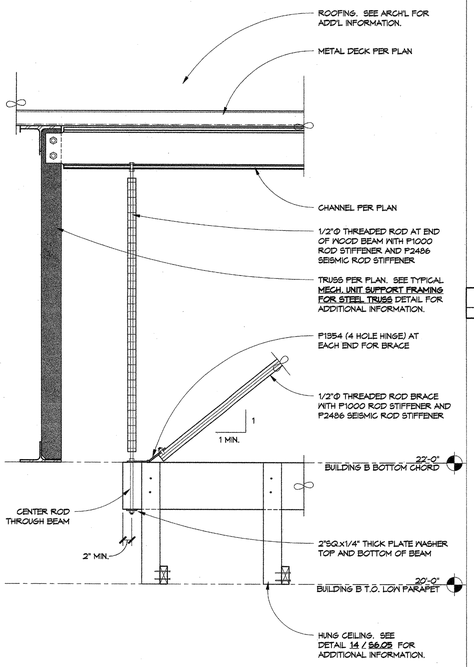
REGISTERED PROFESSIONAL ENGINEER
GAILLORD R. RAMOS
No. S23581
EXPIRES 06-30-20
STRUCTURAL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
Structural Engineers
Civil Engineers
7415 N. Palm Ave., Suite 100
Fresno, California 93711
17003

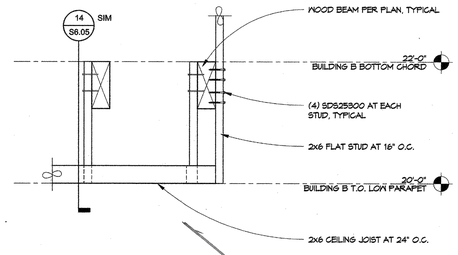
Job No.:

Sheet No.: S6.04

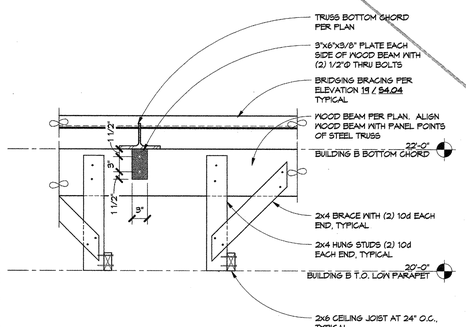
Release:



18	DETAIL		
S3.02	S6.05	1" = 1'-0"	



13	DETAIL		
S6.05	S6.05	1" = 1'-0"	



14	DETAIL		
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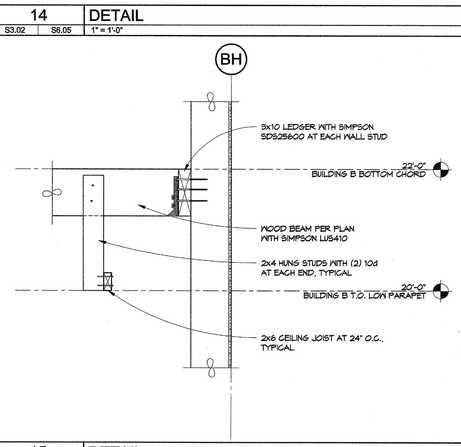

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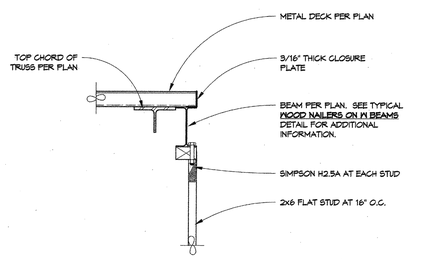

16	DETAIL		
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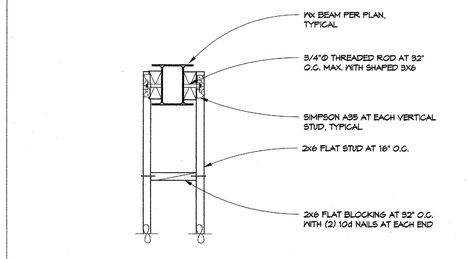

9	DETAIL		
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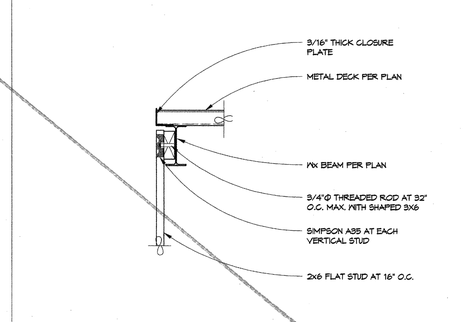

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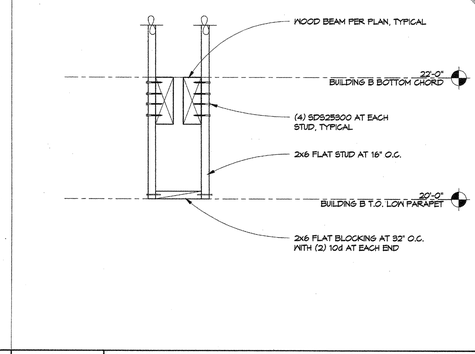

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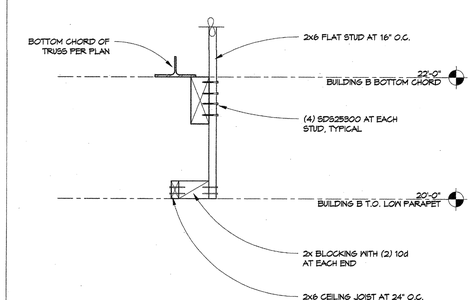

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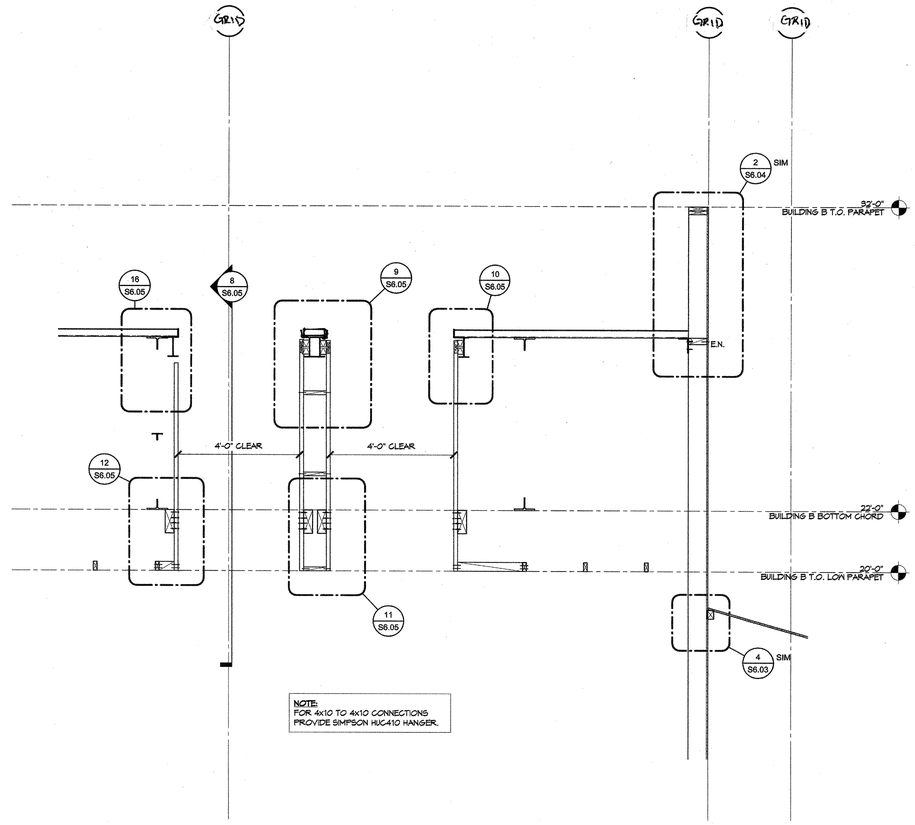
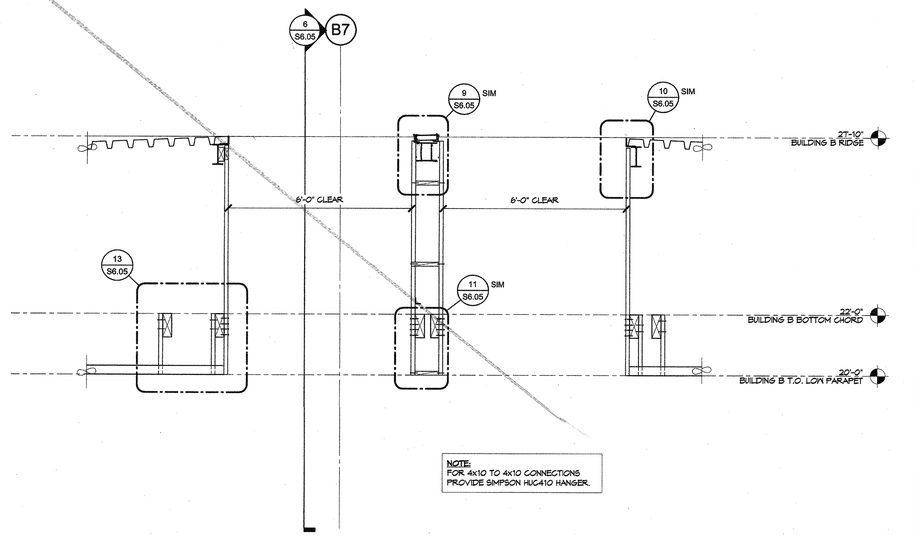


6	SECTION		
S3.02	S6.05	1/2" = 1'-0"	



8	SECTION		
S3.02	S6.05	1/2" = 1'-0"	

19	NOT USED
	NOT TO SCALE

20	NOT USED
	NOT TO SCALE

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6311 N. Fresno, Suite 120 - Fresno, California 93710
Phone (559) 436-0881 Fax (559) 436-0887 E-Mail: design@somam.com
www.integrateddesigns.com

Revision:	Revision Description:	Rev. Date:	Revision:	Revision Description:	Rev. Date:

Sheet Title: B FRAMING DETAILS

Project Name: NEW ELEMENTARY SCHOOL INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 01-31-18
Date:
Designer: Designer
DR:
PC:

Agency Approval Stamp:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
OFFICE OF REGULATION SERVICES
03-118394
AC. FLS SS XM
DATE 06-22-18

TRACKING #: 63321-300

Stamp(s)

REGISTERED PROFESSIONAL ENGINEER
CAYFORD R. RANSOM
No. S7356
EXPIRES 03-31-20
STRUCTURAL
STATE OF CALIFORNIA

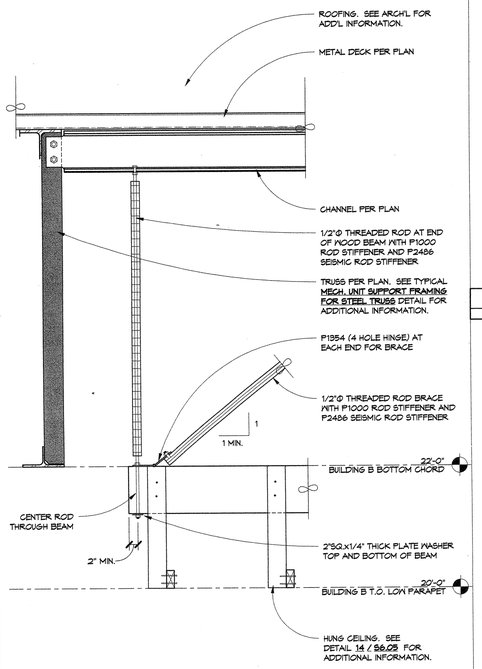
BROOKS-RANSOM ASSOCIATES
STRUCTURAL ENGINEERS
CIVIL ENGINEERS
7415 N. PALM AVE., SUITE 100
FRESNO, CALIFORNIA 93711

17001

Job No.:

Sheet No.: S6.05

Release:



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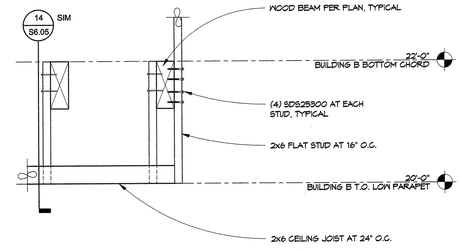

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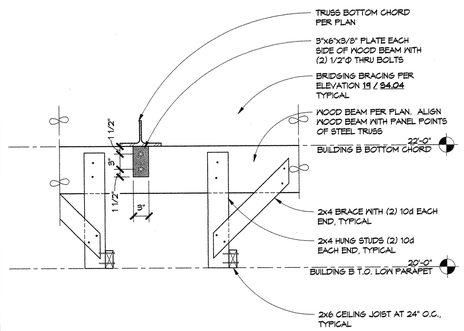

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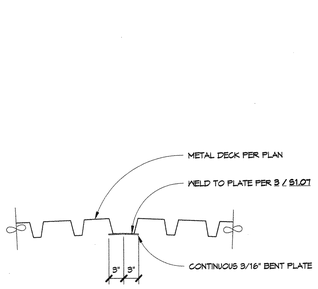

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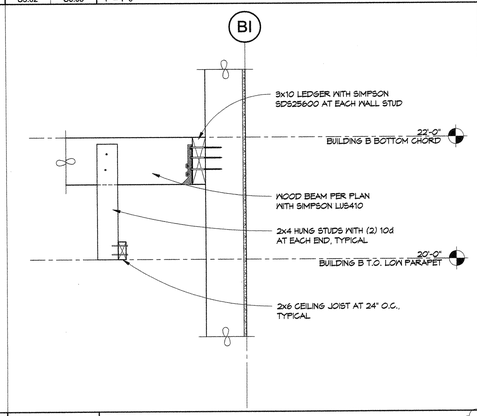

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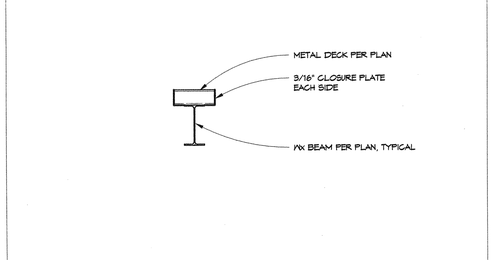

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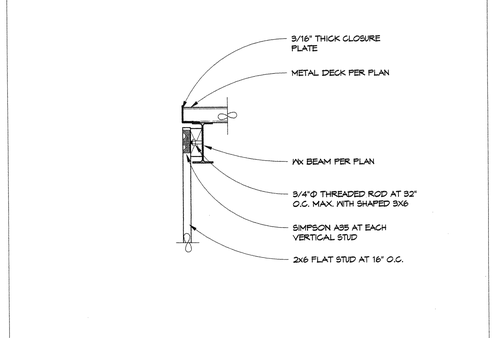

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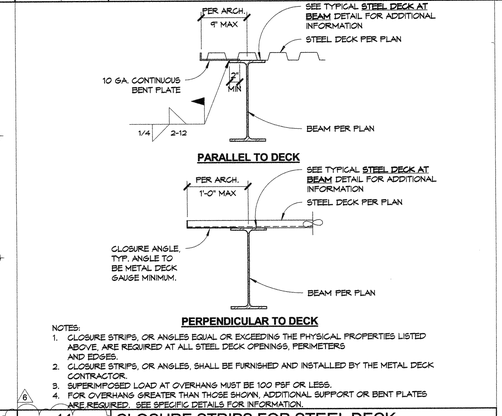

11	CLOSURE STRIPS FOR STEEL DECK		
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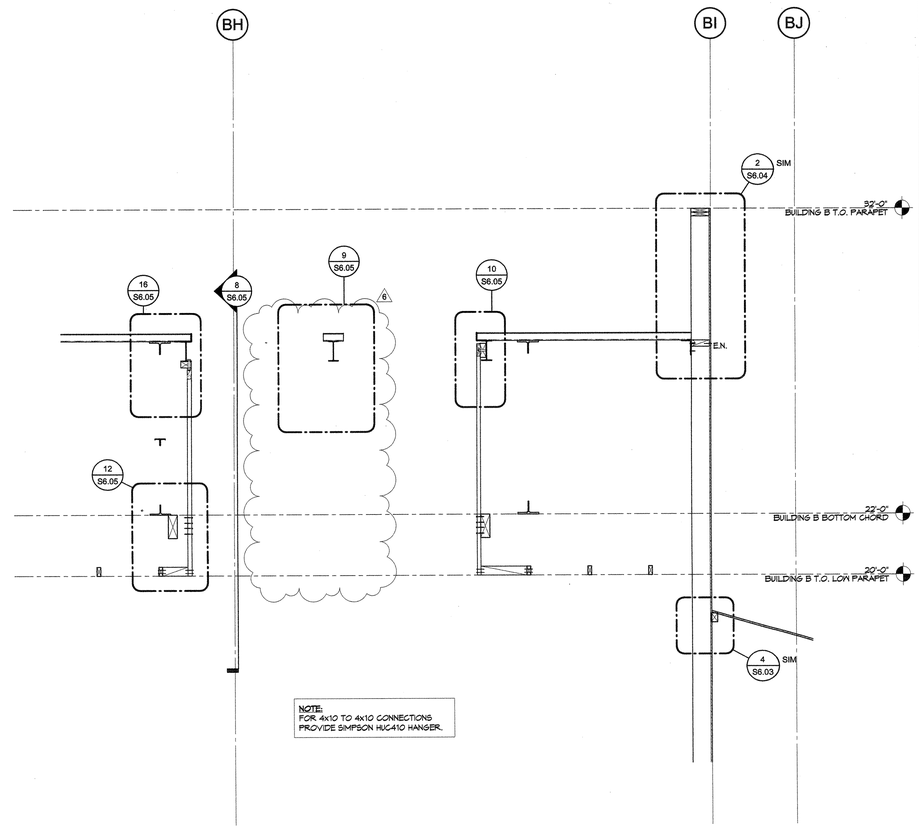
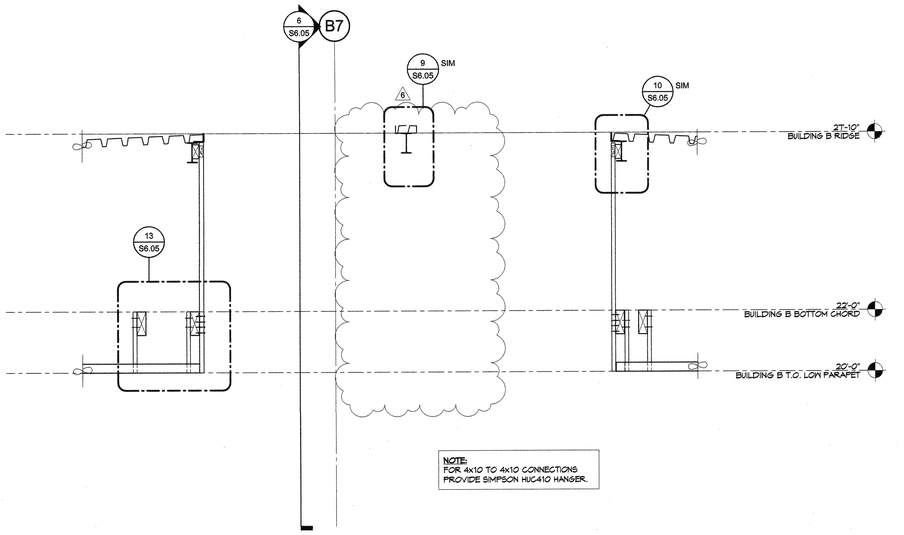


6	SECTION		
S3.02	S6.05	1/2" = 1'-0"	



8	SECTION		
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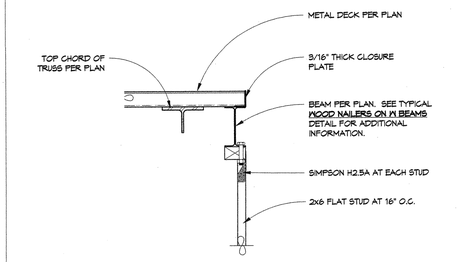

16	DETAIL		
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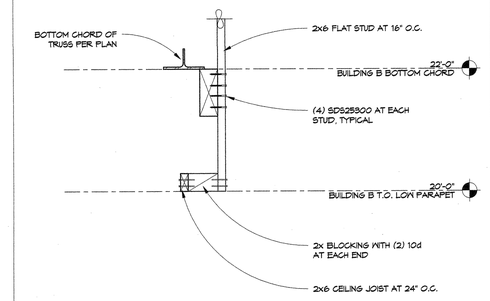

12	DETAIL		
S6.05	S6.05	1" = 1'-0"	

20	NOT USED
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Revisions:	Revision Description:	Rev. Date:	Revision:	Rev. Date:	Revision Description:	Rev. Date:
2	ADDENDUM #2	08/16/18				
4	ADDENDUM #4	09/12/18				
6	ADDENDUM #6					

Sheet Title: B FRAMING DETAILS

Project Name & Address:
NEW E.S. INCREMENT 2
BAKERSFIELD CITY SCHOOL DISTRICT
@ CITADEL ROAD & MARDI GRAS COURT

Issue Date: 9-07-18
Date:
Designer: Designer
DR:
PIC:

Agency Approval Stamp:

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
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AC SC FLS SS SS TN
DATE 12/20/18

TRACKING #: 63321-300

Stamp(s)

REGISTERED PROFESSIONAL ENGINEER
GAYLORD R. BROOKS
No. S-5191
STRUCTURAL
STATE OF CALIFORNIA

BROOKS-RANSOM ASSOCIATES
Structural Engineers
Civil Engineers
7415 N. Palm Ave., Suite 100
Fresno, California 93711

Job No.:

Sheet No.:
S6.05

Release: ADDENDUM NO. 6